

# AGRICULTURAL WATER MANAGEMENT PLAN

## 2015 Update

FOR THE

### MODESTO IRRIGATION DISTRICT



December 2015

Completed In Accordance With the  
WATER CONSERVATION ACT OF 2009  
(SBx7-7)



DATE SIGNED: 12-19-2015

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## Table of Contents

Section I. Plan Preparation and Adoption.....	1
A. Description of Previous Water Management Activities .....	4
B. Coordination Activities.....	8
C. AWMP Adoption and Submittal.....	9
D. AWMP Implementation.....	10
Section II. Description of the Modesto Irrigation District and Service Area.....	13
A. Physical Characteristics.....	13
B. Operational Characteristics.....	21
Section III. Description of Quantity of the Water Uses of the Agricultural Water Supplier .....	32
A. Agricultural Water Use.....	32
B. Environmental Water Use.....	34
C. Recreational Water Use.....	35
D. Municipal and Industrial Water Use .....	36
E. Groundwater Recharge Use .....	37
F. Transfer and Exchange Use.....	39
G. Other Water Use .....	39
H. Projected Water Use .....	39
Section IV. Description of Quantity and Quality of the Water Resources of the Agriculture Water Supplier .....	41
A. Water Supply Quantity .....	41
B. Water Supply Quality.....	46
C. Water Quality Monitoring Practices .....	48
Section V. Water Accounting and Water Supply Reliability.....	50
A. Quantifying the Water Supplier's Water Supplies .....	50
B. Quantification of Water Uses.....	52
C. Overall Water Budget .....	53
D. Water Supplies and Demands for 2013, 2014 and 2015.....	56
E. Water Supply Reliability.....	56
F. Future Water Supply.....	57
Section VI. Analysis of Effect of Climate Change.....	59
A. Effects of Climate Change on Water Supply.....	59
B. Effects of Climate Change on Agriculture's Water Demand .....	60
C. MID Response to Effects of Climate Change.....	61
Section VII. Water Use Efficiency Information.....	62
A. EWMP Implementation and Reporting.....	62
B. Documentation for Non-Implemented EWMPs.....	71
Section VIII. Agricultural Water Measurement Regulation Documentation .....	73
A. Introduction .....	73
B. Existing Facilities and Measurement Practices .....	74
C. Legal Certification and Apportionment Required for Water Measurement – Lack of Legal Access to the Farm-gate.....	76
D. Engineer Certification and Apportionment Required for Water Measurement – Technically Infeasible .....	77
E. Description of Water Measurement Best Professional Practices.....	78
F. Documentation of Water Measurement Conversion to Volume .....	80
G. Device Corrective Action Plan Required for Water Measurement .....	81
Section IX. References .....	93

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List of Figures

---

Figure 1 - Location Map of MID and Stanislaus County.....	2
Figure 2 - MID Irrigation Service Area.....	15
Figure 3- Annual Tuolumne River Computed Natural Flow and River Releases by MID Diversions and New Don Pedro Storage .	29
Figure 4 - Modesto Groundwater Basin, Spring 1994 Groundwater Elevations, Unconfined Aquifer.....	37
Figure 5 - Modesto Groundwater Basin, Spring 2010 Groundwater Elevations, Unconfined Aquifer.....	37
Figure 6 - Critically Overdrafted Groundwater Basins.....	58
Figure 7 – Pilot Program Site Locations.....	84
Figure 8 – Preliminary Pilot Program Test Results.....	87

List of Tables

---

Table 1 - Summary of Coordination, Adoption and Submittal Activities.....	10
Table 2 - Water Supplier History and Size.....	13
Table 3 - Expected Changes to Irrigation Service Area.....	14
Table 4 - Water Conveyance and Delivery System.....	16
Table 5 - Water Supplier Reservoirs.....	17
Table 6 - Tailwater/Operational Outflow Recovery System.....	18
Table 7 - Landscape Characteristics.....	20
Table 8 - Summary Climate Characteristics.....	20
Table 9 - Detailed Climate Characteristics.....	21
Table 10 - Supplier Delivery System.....	22
Table 11 - Water Allocation Policy.....	23
Table 12 - Actual Lead Times.....	23
Table 13 - Water Delivery Measurements.....	25
Table 14 - Water Rate Basis.....	25
Table 15 - Rate Structure.....	26
Table 16 – 2015 Volumetric Pricing Structure.....	26
Table 17 - Frequency of Billing.....	27
Table 18 - Decreased Water Supplies Allocation.....	27
Table 19 - Enforcement Methods of Allocation Policies.....	28
Table 20 - Representative Year.....	28
Table 21 – Implementation of Drought Management Plan.....	31
Table 22 - Agricultural and Municipal Water Use for 2012.....	32
Table 23 - Agricultural Crop and Water Demand Data for 2012.....	33
Table 24 - Irrigated Acres for 2012.....	34
Table 25 - Multiple Crop Information for 2012.....	34
Table 26 - Environmental Water Uses for 2012.....	35
Table 27 - Recreational Water Uses for 2012.....	35
Table 28 - Municipal/Industrial Water Uses for 2012.....	36
Table 29 - Groundwater Recharge Water Uses for 2012.....	38
Table 30 - Transfers and Exchanges Water Uses for 2012.....	39
Table 31 - Other Water Uses for 2012.....	39
Table 32 - Surface Water Supplies– Agricultural and Municipal for 2012.....	41
Table 33 - Restrictions on Water Sources.....	41
Table 34 - Groundwater Basins.....	43
Table 35 - Groundwater Management Plan.....	43
Table 36 - Groundwater Supplies for 2012.....	45
Table 37 - Drainage Discharge for 2012.....	46
Table 38 - Modesto Reservoir Average Water Supply Quality - 2012.....	47
Table 39 - Drainage Reuse Effects.....	48
Table 40 - Water Quality Monitoring Practices.....	48
Table 41 - Water Quality Monitoring Programs for Surface/Sub-Surface Drainage.....	49
Table 42 - Surface and Other Water Supplies for 2012.....	50
Table 43 - Groundwater Supplies Summary for 2012.....	51
Table 44 - Effective Precipitation Summary for 2012.....	52
Table 45 - Applied Water for 2012.....	52
Table 46 - Quantify Water Leaving the District for 2012.....	53

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## Modesto Irrigation District – 2015 Agricultural Water Management Plan Update

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Table 47 - Irrecoverable Water Losses for 2012.....	53
Table 48 Overall Water Balance for 2012.....	54
Table 49 - Water Balance Parameters – Source of Information .....	55
Table 50 - Report of EWMPs .....	63
Table 51 - Report of EWMPs Efficiency Improvements.....	68
Table 52 - Schedule to Implement EWMPs .....	71
Table 53 - Non-Implemented EWMP Documentation .....	72
Table 54 - Preliminary Turnout Inventory .....	76
Table 55 - Pilot Program site characteristics .....	83
Table 56 – Pilot Program costs per site.....	85
Table 57 – Measurement Devices tested.....	86
Table 58 - Preliminary* Flow Measurement Implementation Plan.....	89
Table 59 - Proposed Implementation Timeline .....	91
Table 60 - Implementation Cost Forecast for Preliminary Measurement Implementation Plan .....	92

### List of Appendices

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Appendix A. Public Notice of Plan Preparation
Appendix B. Resolution of Plan Adoption
Appendix C. Rules and Regulations Governing the Distribution of Irrigation Water Within the Modesto Irrigation District
Appendix D. Irrigation Rate Structure
Appendix E. Drought Management Plan
Appendix F. Groundwater Management Plan: Executive Summary
Appendix G. Conservation Program Guidelines
Appendix H. ITRC Water Measurement Reports
Appendix I. Comments on Draft Agricultural Water Management Plan

**List of Acronyms**

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AB	Assembly Bill
AF	acre-feet
ASO	Airborne Snow Observatory
AWMP	Agricultural Water Management Plan
CCR	California Code of Regulations
CCSF	City and County of San Francisco
cfs	cubic feet per second
CIMIS	California Irrigation Management Information System
CNF	Computed natural flow
CVRWQCB	Central Valley Regional Water Quality Control Board
CWRMP	Comprehensive Water Resources Management Plan
DPRA	Don Pedro Recreation Agency
DSS	Decision Support System
DWR	Department of Water Resources
ET	Evapotranspiration
ET <sub>c</sub>	Crop evapotranspiration
ET <sub>o</sub>	Reference evapotranspiration
EWMP	Efficient Water Management Practice
FERC	Federal Energy Regulatory Commission
JPL	Jet Propulsion Laboratory
GIS	Geographic Information System
GSA	Groundwater Sustainability Agency
GSP	Groundwater Sustainability Plan
ID	Improvement District
ILRP	Irrigated Lands Regulatory Program
IRGWMP	Integrated Regional Groundwater Management Plan
ITRC	Irrigation Training and Research Center
M&I	Municipal and Industrial
METRIC	Mapping of ET with Internal Calibration
MID	Modesto Irrigation District
MRWTP	Modesto Regional Water Treatment Plant
NPDES	National Pollution Discharge Elimination System
O&M	Operation and Maintenance
OID	Oakdale Irrigation District
PEIR	Programmatic Environmental Impact Report
PRMS	Precipitation Runoff Modeling System
QC/QA	Quality Control/Quality Assurance
SB	Senate Bill
SBx7-7	Water Conservation Act of 2009
SCADA	Supervisory Control and Data Acquisition
SFPUC	San Francisco Public Utility Commission
SGMA	Sustainable Groundwater Management Act
SSJID	South San Joaquin Irrigation District
STRGBA	Stanislaus and Tuolumne Rivers Groundwater Basin Association
SWRCB	State Water Resources Control Board
TDS	Total Dissolved Solids
TID	Turlock Irrigation District
USACE	U.S. Army Corps of Engineers
USGS	United States Geological Survey
UWMP	Urban Water Management Plan
VAMP	Vernalis Adaptive Management Plan
WSID	West Stanislaus Irrigation District

# AGRICULTURAL WATER MANAGEMENT PLAN

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## ***Section I. Plan Preparation and Adoption***

This Modesto Irrigation District (MID or District) Agricultural Water Management Plan (AWMP) is an update of the District's 2012 AWMP and has been prepared in accordance with the requirements of the Water Conservation Act of 2009 (SBx7-7), and conforms to the framework presented in *A Guidebook to Assist Agricultural Water Suppliers to Prepare a 2015 Agricultural Water Management Plan* (2015 Guidebook) that was issued by the California Department of Water Resources (DWR) in June 2015. The District is located in Stanislaus County, east of the San Joaquin River and between the Tuolumne and Stanislaus Rivers, as shown in Figure 1.

The 2015 Guidebook includes new requirements for AWMPs that were originally established by Executive Order B-29-15, issued by Governor Jerry Brown on April 1, 2015. These new requirements include a detailed drought management plan and water supply and demand data for 2013, 2014 and 2015, to the extent the data is available.

The requirements introduced by SBx7-7 are intended to encourage agricultural water suppliers to assess current efficient water management practices (EWMP), to evaluate additional practices that may conserve water, and to require accurate measurement of water delivered to customers. The AWMP process also presents an opportunity for water suppliers to demonstrate existing accomplishments in water use efficiency as well as anticipated water use efficiency measures.

Included in Section VII of this AWMP is an analysis of each of the EWMPs to be addressed as part of SBx7-7 and as outlined in the 2015 Guidebook prepared by DWR. The EWMPs are grouped into the following categories:

- Critical Efficient Water Management Practices
  1. Measure the volume of water delivered to customers with sufficient accuracy to comply with subdivision (a) of California Water Code Section 531.10 and to implement paragraph (2) of the legislation.
  2. Adopt a pricing structure for water customers based at least in part on quantity delivered.
- Conditional Efficient Water Management Practices
  1. Facilitation of alternative land use for lands with exceptionally high water duties or whose irrigation contributes to significant problems, including problem drainage.

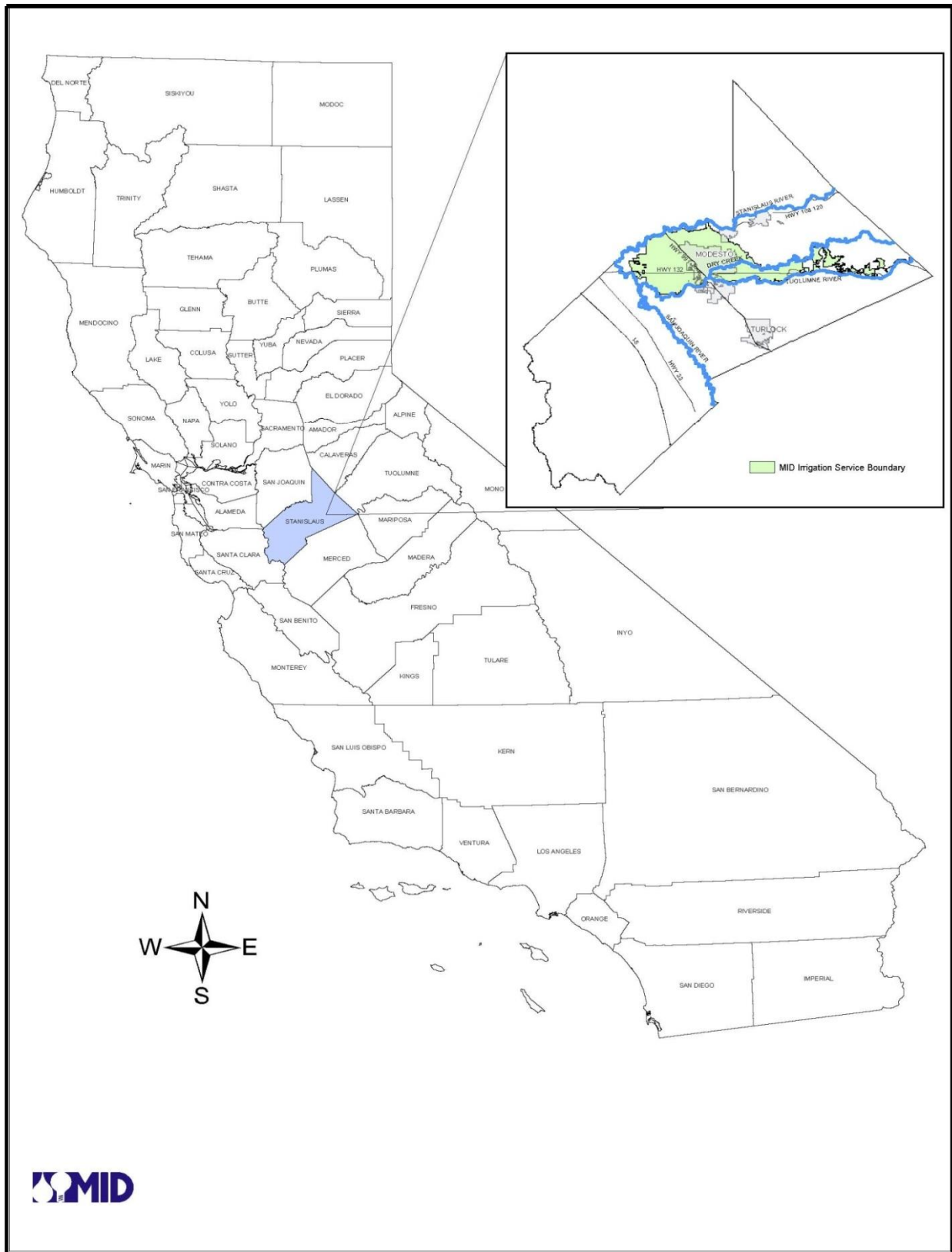


Figure 1 - Location Map of MID and Stanislaus County

2. Facilitation of use of available recycled water that otherwise would not be used beneficially, meets health and safety criteria, and does not harm crops or soils. The use of recycled urban wastewater can be an important element in overall water management.
3. Facilitate the financing of capital improvements for on-farm irrigation systems.
4. Implement an incentive pricing structure that promotes one or more of the following goals:
  - A. More efficient water use at the farm level such that it reduces waste;
  - B. Conjunctive use of groundwater;
  - C. Appropriate increase of groundwater recharge;
  - D. Reduction in problem drainage;
  - E. Improved management of environmental resources, and
  - F. Effective management of all water sources throughout the year by adjusting seasonal pricing structures based on current conditions.
5. Expand lined or piped distribution systems, construct regulatory reservoirs to increase distribution system flexibility and capacity, decrease maintenance, and reduce seepage.
6. Increase flexibility in water ordering by, and delivered to, water customers within operational limits.
7. Construct and operate supplier operational outflow and tailwater systems.
8. Increase planned conjunctive use of surface water and groundwater within the supplier service area.
9. Automate canal control devices.
10. Facilitate or promote customer pump testing and evaluation.
11. Designate a water conservation coordinator who will develop and implement the water management plan and prepare progress reports.
12. Provide for the availability of water management services to water users. These services may include, but are not limited to, all of the following:
  - A. On-farm irrigation and drainage system evaluations;
  - B. Normal year and real-time irrigation scheduling and crop evapotranspiration (ETc) information;
  - C. Surface water, groundwater, and drainage water quantity and quality data, and
  - D. Agricultural water management educational programs and materials for irrigators.



13. Evaluate the policies of agencies that provide the supplier with water to identify the potential for institutional change to allow more flexible water deliveries and storage.
14. Evaluate and improve the efficiencies of the suppliers' pumps.

### **A. Description of Previous Water Management Activities**

MID and its agricultural water users have implemented many of the EWMPs described in the District's 1999 and 2012 AWMP's. In addition, numerous water conservation measures beyond those identified in the 1999 and 2012 AWMP's have been implemented.

A central consideration in the District's determination of how best to implement a program of EWMPs is the District's goal of providing flexible, reliable service to its agricultural water users. Irrigators in MID are transitioning from producing field crops such as alfalfa and grains to permanent crops such as trees and vines. As irrigators transition from field crops to permanent crops and shift toward pressurized, low-volume drip and micro-sprinkler systems, the requirements of customer service are changing.

In addition, regardless of crop mix and on-farm irrigation practices, the District remains committed to maintaining a balance between surface water and groundwater as sources of supply and has pursued pricing policies and operational practices that support conjunctive management. The effort required to sustain groundwater levels and retain the ability to tap this resource during periods of prolonged drought has served the District well and, as discussed later in this AWMP, may serve as an effective mechanism for meeting requirements of the Sustainable Groundwater Management Act (SGMA) and responding to the effects of climate change.

For the reasons described above, when evaluating EWMPs, MID assesses the value of EWMPs as part of a comprehensive package of practices that assist the District in providing a high level of customer service and support conjunctive management. As a result, the District may implement individual EWMPs that are not cost-effective in a narrow sense. However, providing reliable, responsive customer service is essential for maintaining a stable customer base and meeting the changing needs of MID's agricultural water users.

The following section describes some of the practices implemented by the District that are consistent with the principles of the District's AWMP planning efforts:

- **Financial Grants:** MID has provided financial support to agricultural water users for the replacement of on-farm water supply ditches and concrete cast-in-place pipelines. The District recently developed the MID Conservation Program (Program), which provides partial funding to qualifying MID landowners for projects that conserve water and improve water management after the eligible project is completed. Projects must meet certain eligibility criteria and be pre-approved by MID. The Guidelines (**Appendix G**) provide information on eligible projects, applicant eligibility, available funding, the application process, project ranking criteria, contractual obligations, and the anticipated

annual schedule. The Program will occur annually, but is subject to funding and approval by the Board of Directors (Board) on an annual basis.

- **Water Measurement Pilot Program.** MID has been testing several water measurement devices to determine which are the most suitable and accurate for their irrigation system. The initial results of the Pilot Program are discussed herein and may be refined as additional testing is conducted during the 2016 irrigation season. This information will help the District to make an informed decision on appropriate measurement devices or methods necessary to comply with the measurement requirements of SBx7-7.
- **Financial Contributions:** MID has made financial contributions to a Mobile Irrigation Lab, operated by the East Stanislaus Resource Conservation District, to evaluate the performance and efficiency of grower’s on-farm irrigation systems (MID has paid up to 75 percent of the cost of the irrigation system evaluation).
- **In-lieu Groundwater Recharge:** Prior to 1995, the City of Modesto relied solely on groundwater to meet its municipal and industrial (M&I) needs. MID now delivers up to 36,600 acre-feet (AF) of treated Tuolumne River water per year to the City of Modesto for M&I uses. With completion of Phase II by the end of 2015, MID will have the capacity to deliver up to 70,000 AF of treated surface water to the City of Modesto or nearly 85% of the City of Modesto’s estimated 2015 demand as published in the City of Modesto/MID 2010 Joint Urban Water Management Plan (UWMP).
- **Automatic SCADA Controls:** Automatic Supervisory Control and Data Acquisition (SCADA) systems have been installed at most of the District’s water distribution diversion and operational outflow facilities. The automation of water distribution diversion and operational outflow facilities gives the District greater flexibility to manage the water distribution system and increases the reliability of on-farm water deliveries.
- **Crop Water Use Information:** MID makes data from the California Irrigation Management Information System (CIMIS) available to water users. CIMIS daily and seasonal crop water use information is available through telephone access or through the MID’s website at [www.mid.org](http://www.mid.org).
- **U.S Geological Survey (USGS) Groundwater Study:** MID through its involvement with the Stanislaus and Tuolumne Rivers Groundwater Basin Association (STRGBA) was instrumental in contracting with the USGS to conduct a basin groundwater study. The 2004 study, entitled "*Hydrogeologic Characterization of the Modesto Area, San Joaquin Valley, California*", provided the District and the other basin water users and suppliers with information regarding the hydrologic structure of the basin. The USGS completed an update to the study in 2015 entitled "*Hydrologic Model of the Modesto Region, California, 1960-2004*". The update includes a three-dimensional

groundwater model for the Modesto Groundwater Sub-basin called the MERSTAN model.

- **Tuolumne River Watershed Hydrologic Model:** MID and Turlock Irrigation District (TID) purchased a hydrologic model for the Tuolumne River for analysis of current and future watershed runoff potential. The model optimizes MID's and TID's management of Tuolumne River watershed runoff and New Don Pedro Reservoir water storage.
- **Airborne Snow Observatory:** Recent drought conditions and increased regulatory requirements have increased the need for water managers in the Tuolumne River watershed to make better and earlier predictions of inflow patterns. In an attempt to better understand the variability in upcountry snow storage, MID along with TID and the City and County of San Francisco (CCSF) have partnered with the Airborne Snow Observatory (ASO). The ASO is a project run by the Jet Propulsion Laboratory (JPL) and NASA wherein a LiDAR instrument is mounted on an airplane and flown over the Tuolumne River watershed to measure snow depth. The resulting snow depth maps are then coupled with snow density modeling to give snow water equivalent patterns over the Tuolumne River watershed. The ASO has been making weekly flights from April 1 through the end of runoff season for the last three years (2013 through 2015).
- **Precipitation Runoff Modeling System:** Watershed cooperators (MID, TID, CCSF) have partnered with the Department of Water Resources (DWR) and the United States Geological Survey (USGS) to develop a Precipitation Runoff Modeling System (PRMS) for the Tuolumne River watershed. PRMS offers an alternative approach for forecasting water supply based on the simulation of each component of the hydrologic cycle using physical and empirical methods. A key component of successful PRMS implementation is ensuring that the model is capable of producing accurate forecasts in a quick and efficient manner.
- **Water Quality Monitoring and Sampling:** MID has a water quality monitoring program and successfully complied with the statewide general NPDES permit for discharge of aquatic herbicides. MID also participates in a water monitoring and sampling program in compliance with the Irrigated Lands Regulatory Program (ILRP) as adopted by the Central Valley Regional Water Quality Control Board (CVRWQCB) as a member of the East San Joaquin Water Quality Coalition. In addition, MID performs annual monitoring of select District groundwater wells.
- **Rim Fire Water Quality Monitoring:** In August 2013 the Rim Fire (3<sup>rd</sup> largest in California history) burned approximately 400 square miles of the Tuolumne River watershed. MID and TID partnered with the United States Geological Survey (USGS) to establish a stream gage and extensive water quality monitoring at a location downstream from the fire extent and upstream from New Don Pedro Reservoir. Documenting the quantity and quality of water entering the New Don Pedro Reservoir, and modeling streamflow changes in response to the fire, gives water managers the tools to understand

the cumulative effects of the Rim Fire on future water supplies. Water quality monitoring began in late fall of 2013 and will continue into the future.

- **UC Davis Water Quality Study:** The MID Domestic Water Treatment Plant entered into an agreement (December 6, 2013) with the University of California Davis (UCD) Watershed Science Center to conduct water quality monitoring and perform laboratory treatability studies to identify the constituents and parameters of greatest concern for the efficacy of water treatment processes and the quality of treated water. This project is expected to continue through 2016.
- **Water Allocation and Pricing:** Consistent with MID's goals, the MID Board of Directors (Board) has been increasing the cost of the irrigation water service charge by, on average, approximately 10 percent per year to encourage efficient water use. The most recent change has included conversion to a pricing structure, based at least in part on the quantity delivered, to encourage water conservation.
- **2010 Urban Water Management Plan (UWMP):** MID and the City of Modesto jointly prepared and adopted the 2010 UWMP in compliance with the Urban Water Management Planning Act. MID will again partner with the City of Modesto for preparation of a 5-year update to the plan in 2015/2016.
- **Integrated Regional Groundwater Management Plan for the Modesto Groundwater Sub-basin (IRGWMP):** MID was the lead agency in the development of the IRGWMP by the Stanislaus and Tuolumne Rivers Groundwater Basin Association (STRGBA) in 2005. Current STRGBA members include MID, the Cities of Modesto, Riverbank, and Oakdale, Stanislaus County, and the Oakdale Irrigation District (OID). Revision to the existing Memorandum of Understanding (MOU) is ongoing and the City of Waterford is expected to be added as a signatory, thereby including all local agencies within the Modesto Sub-basin. It is envisioned that STRGBA will be the lead agency for complying with SGMA.
- **Recharge Characterization of the Modesto Groundwater Sub-basin.** This project assessed recharge areas within the Modesto Sub-basin and recharge mechanisms within those areas. The project identified where recharge is occurring currently, where it has occurred in the past, and where future recharge could occur.
- **Well Field Optimization Project:** This project by MID and OID, in cooperation with STRGBA, was developed using a DWR-funded grant. The project involved the design and implementation of a computer-aided Decision Support System (DSS) to operate irrigation wells. The DSS was developed as a management tool for implementing the District's conjunctive use program.
- **Comprehensive Water Resources Management Plan:** Beginning in 2007, MID embarked on a Comprehensive Water Resources Management Plan (CWRMP) consisting of a

variety of recommendations for policy and facility improvements to accommodate current and future water demands. The District's Comprehensive Water Resources Management Plan (CWRMP) is a multi-phase effort intended to incorporate elements of prior planning efforts, new information, and creative ideas into a comprehensive plan to guide future water management decisions. Decision makers, stakeholders, consultants, and staff benefit from a comprehensive picture of the issues and impacts related to water management in the district. The District's goals in developing the CWRMP are to:

1. Address discharge water quality and regulatory risks
2. Improve operations efficiency and customer service
3. Plan for aging system replacement
4. Adapt to technology change
5. Plan for Federal Energy Regulatory Commission (FERC) relicensing so that the District can provide evidence to support its renewal application and minimize any negative impacts that might result from relicensing decisions
6. Understand the options and opportunities available to the District for addressing current and future needs

MID is currently working on a Programmatic Environmental Impact Report (PEIR) for the CWRMP under a contract with CH2MHill. The PEIR is intended to provide a high level analysis of the potential CWRMP impacts and set the stage for focused individual project specific environmental review as projects warrant and as resources allow. MID anticipates completion of the PEIR in 2016. While implementation of the CWRMP is contingent upon funding, MID sees benefits in the CWRMP as an effort to identify better methods to manage the District's water resources. Potential funding mechanisms to implement the CWRMP will be identified in the PEIR.

## **B. Coordination Activities**

### **1. Notification of AWMP Preparation**

SBx7-7 requires that each city or county within which the supplier provides water supplies be notified that the AWMP is being prepared, but doesn't specifically identify how much advance time is required for notification of cities and counties of the AWMP preparation. SBx7-7 also doesn't require notification to any other agency(s) and doesn't require that comments from any city, county or other agency must be solicited and considered. The District however, did notify local agencies and the public that the AWMP was being updated as shown in **Table 1. Appendix A** includes documentation on the public noticing of the AWMP preparation and adoption.

### **2. Public Participation**

Public participation activities associated with preparation of the updated AWMP are presented in **Table 1.**

### **C. AWMP Adoption and Submittal**

The purposes of this updated AWMP are to assess MID's current water management operations, provide background with respect to actions taken since the 2012 AWMP, to respond to the provisions of SBx7-7 and to discuss future actions that may be taken within the next planning horizon. The AWMP adoption and submittal process follows that outlined in the 2015 Guidebook.

#### **1. AWMP Adoption**

This 2015 AWMP update has been adopted by the District Board of Directors. **Appendix B** of this document includes a Resolution of AWMP Adoption.

#### **2. AWMP Submittal**

The District followed the steps that are described in the 2015 Guidebook for submittal of the AWMP and the process that was followed is as outlined in Table 1.

#### **3. AWMP Availability**

In preparing this AWMP, MID solicited public input by holding a public hearing and inviting oral and written comments prior to adoption of the AWMP at a Board of Director's meeting on December 15, 2015. The public hearing was advertised in the Modesto Bee newspaper on November 15, December 1, and December 8, 2015. A copy of the newspaper notice is found in **Appendix A**. Table 1 shows the state and local interested parties who were notified about preparation of the updated AWMP. The public hearing was also advertised on the District website. Written comments provided on the AWMP are found in **Appendix I**.

Table 1 - Summary of Coordination, Adoption and Submittal Activities

Potential Interested Parties	Notified of AWMP Preparation	Notified of Public Meetings	Sent Copy of Adopted AWMP
Department of Water Resources			X
City of Modesto	X		X
City of Riverbank	X		X
City of Waterford	X		X
Turlock Irrigation District	X		X
Stanislaus County	X		X
Local Newspaper		Nov. 15, Dec. 1 and 8, 2015	
Stanislaus and Tuolumne Rivers Groundwater Basin Association	X		
LAFCO			X
City/County Library			X
State Library			X
MID Website	November 10, 2015	November 15, 2015	

**D. AWMP Implementation**

MID continues to implement EWMPs based upon the implementation plan presented in its original AWMP and refined in later AWMP updates. In addition to implementing EWMPs described in previous AWMP’s, the District has recently implemented the volumetric pricing EWMP and plans to implement the water measurement EWMP mandated by SBx7-7 as described later in this document.

Following are MID Capital Projects completed from 2012-2014 (since preparation of the District’s 2012 AWMP) that are consistent with the goals and EWMPs in this AWMP.

**2012 CAPITAL PROJECTS**

**1. MID Upper Main Canal Tunnel Gate Rehabilitation Project**

The MID Upper Main Tunnel is served by a 16’ diameter tunnel that diverts both agricultural water and domestic water to serve both Modesto Irrigation District and the City of Modesto respectively from a point of diversion upstream of the La Grange Dam on the Tuolumne River. The MID Upper Main Canal Tunnel Gate Rehabilitation Project (Project) was completed in an effort to repair and rehabilitate the MID Upper Main Tunnel Gate to conserve water, reduce seepage rates and to improve gate control, as the gate experienced difficulty closing at larger hydraulic pressures. The Project replaced all axles and gate rollers of the original Rodney Hunt

12' x 16' roller gate to allow for full closure of the gate, installed a new gate seal to reduce seepage rates, and replaced all controls to allow for a better calibrated gate measurement of flow in the MID Upper Main Canal. In addition to the above described work, the MID Upper Main Canal USGS gaging site was rehabilitated with local MID SCADA control to provide redundant flow measurement and accuracy of the MID Upper Main Canal Tunnel Gate. Both the MID Upper Main Canal Tunnel Gate and MID Upper Main Canal USGS gaging site were integrated into the new irrigation SCADA system to allow irrigation operations staff real-time control and data access via a laptop computer.

**2. Waterford Upper Main Canal Headworks Rehabilitation Project**

The Waterford Upper Main Canal Headworks Rehabilitation Project (Project) replaced the SCADA controls on the gate diverting water from the MID Upper Main Canal into the Waterford Upper Main Canal. The Project was completed in conjunction with the MID Upper Main Canal Tunnel Gate Rehabilitation Project with the intent to better manage agricultural delivery in the MID Upper Main Canal. The Project was integrated into the new Irrigation SCADA system to allow irrigation operations staff real-time control and data access via a laptop computer.

**2013 CAPITAL PROJECTS**

**1. Dichtender Remote SCADA Access Project**

The Dichtender Remote SCADA Access Project (Project) provided for all seven (7) ditchtender delivery areas in MID to have real time access to flow data from a newly developed irrigation SCADA system consisting of approximately thirty (30) water measurement sites via a laptop computer located in each ditchtender vehicle. The Project was completed to allow for improved water management and measurement by each ditchtender in their individual delivery areas, and has proven to be especially helpful during drought operations.

**2. Modesto Reservoir Outlet Automation Project**

The Modesto Reservoir Outlet SCADA Automation Project (Project) consisted of upgrading and replacing the Modesto Reservoir Outlet SCADA controls on the MID Lower Main Lateral and the Waterford Lower Main Lateral. The Project was completed to provide for improved flow control and measurement for both the MID Lower Main Lateral and Waterford Lower Main Lateral as they provide irrigation delivery to the majority of MID agricultural water users. The Project was integrated into the new irrigation SCADA system to allow Irrigation Operations staff real-time control and data access via a laptop computer located in each ditchtender vehicle.

**3. Lateral 5 Pump Automation Project**

The Lateral 5 Pump Automation Project (Project) upgraded the controls and automated a series of five (5) MID deep wells and a flow measurement site located along MID Lateral 5, west of the City of Modesto. The Project utilizes the automated deep wells to supplement agricultural water delivered to agricultural water users along the lower portion of the MID Lateral 5. Completion of the Project has resulted in increased water delivery flexibility and reduced operational outflows. The Project was integrated into the new Irrigation SCADA system to allow Irrigation Operations staff real-time control and data access via a laptop computer located in each ditchtender vehicle.

**4. Waterford Lower Main Lateral Lining Project**



The Waterford Lower Main Lateral Lining Project (Project) was completed to help satisfy part of a previously defined long term goal of concrete lining placement throughout the entire MID canal system. The MID Waterford Lower Main Lateral was previously selected as it is the largest of the few remaining unlined laterals in MID. This Project concrete lined an additional 940 linear feet of the Waterford Lower Main Lateral.

## **2014 CAPITAL PROJECTS**

### **1. MID Lower Main Canal Pump Automation Project**

The MID Lower Main Pump Automation Project (Project) upgraded the controls and automated a series of seven (7) irrigation deep wells and a flow measurement site located along MID Lower Main Canal located near the City of Riverbank. The Project utilizes the automated deep wells and a flow measurement site to supplement agricultural water delivered to agricultural customers along the MID Lower Main Canal and MID Lateral 8 resulting in increased water delivery flexibility and reduced terminal flows. The Project was integrated into the new Irrigation SCADA system to allow operators real time control and data access via a laptop computer located in each ditchtender's vehicle.

### **2. Langdon-Merle Rehabilitation Project**

The Langdon-Merle Rehabilitation Project (Project) provided for remote flow monitoring, measurement and control through a newly designed civil works with two (2) Rubicon FlumeGates. The Project is a critical control site located west of the City of Riverbank on the MID Lower Main Canal. Irrigation Operations staff selected this Project as a critical project due to age of the gates and SCADA control equipment. This Project was the last existing SCADA control site to be integrated into the new irrigation SCADA system allowing operators control of the site via a laptop computer for better response in operation and water management.

The water diverted by the structure can be any combination of MID irrigation water, Oakdale Irrigation District (OID) operation spills, OID stormwater and City of Riverbank stormwater. The OID and Riverbank waters are highly variable and not scheduled as they enter the MID Lower Main Lateral, making water control at this location difficult. Significant operational benefits were obtained from the project including upstream level control during the irrigation season, and downstream flow control during winter months when stormwater is a concern.

## **Section II. Description of the Modesto Irrigation District and Service Area**

### **A. Physical Characteristics**

The Modesto Irrigation District is a public agency which supplies irrigation and electrical service to agricultural, residential, and municipal customers, and treated municipal water to the City of Modesto. Irrigation water supplies include surface water from the Tuolumne River and groundwater from the Modesto Sub-basin. MID’s irrigation service area covers an area of approximately 162 square miles (103,733 acres) in the Tuolumne River watershed (the irrigation service area differs from MID’s electric service area). The 1,880 square mile watershed extends to the high Sierra Nevada Mountains and the Tuolumne River flows to its confluence with the San Joaquin River approximately ten miles west of the City of Modesto. Most of the water in the Tuolumne River comes from snowmelt with peak runoff flows occurring from April through July during which time over 60 percent of the annual flow takes place. The Tuolumne River’s annual median year runoff is approximately 1,900,000 acre-feet, varying between a low of 382,680 acre-feet in 1977 to a high of 4,632,000 acre-feet in 1983. 2014 was one of the lowest runoff years on record, and 2015 will also end up being exceptionally dry. This variability in runoff drives many of MID’s irrigation policies and practices. Figure 2 shows the irrigation service area of the District as well as the cities that are located within the irrigation service area.

#### **1. Size of the Irrigation Service Area**

MID was formed on July 9, 1887 as the second irrigation district to be established in California under the California Irrigation Districts Act (Wright Act). During its early years, MID acquired numerous water rights including pre-1914 rights and constructed facilities to deliver water to irrigate farmland and to generate electricity. As shown in Table 2, a total of 66,451 acres were irrigated within MID in 2012, with approximately 57,000 acres of land receiving surface water from MID.

Table 2 - Water Supplier History and Size

Date of Formation	1887
Source of Water	
Local Surface Water (Tuolumne River)	Yes
Local Groundwater	Yes
Gross Acreage - at Time of Formation	108,000
Gross Acreage - Current Irrigation Service Area (2012)	103,733
Current Irrigated Acreage (2012) <sup>1</sup>	66,451

1 – Cropping data taken from True Point Crop Summary Report for the selected Water Management Plan year (2012). Includes lands that receive MID water, and cropped land within MID that does not currently use MID water.

MID is governed by a five-member, locally elected Board of Directors (Board). Each board member represents a geographical area within MID known as a division. Board members must live within the division they represent and are elected by the registered voters living within that division.

Land use within MID’s irrigation service area is primarily agricultural. Prior to the construction of District irrigation conveyance facilities, dry land crops (primarily wheat and pasture) were grown in the irrigation service area.

The City of Modesto, with a population of over 200,000 people, divides the District into essentially two parts – east and west of the City of Modesto. In addition, the City of Waterford, with a population of over 10,000, is located on the District’s eastern end. Of the 103,733 acres within the District boundary, over 40,000 acres have been developed into residential, commercial and industrial centers.

The irrigated acreage within the District has varied over time principally due to minor boundary adjustments, plus a merger with the Waterford Irrigation District in 1978 and changes in land use driven by urbanization. The trend toward greater urbanization within the District’s boundaries is expected to continue at the rate of about 600 acres per year, based on the 2008 Modesto General Plan (this number hasn’t been updated recently). This conversion rate will be revised as updated information comes available through City and/or County planning efforts. The anticipated magnitude of this change in land use is shown in Table 3.

Table 3 - Expected Changes to Irrigation Service Area

<b>Change to Irrigation Service Area</b>	<b>Estimate of Magnitude</b>	<b>Cause of Change</b>	<b>Effect on Water Supplier</b>
Reduced Irrigation Service Area	None	NA	NA
Increased Irrigation Service Area	None	NA	NA
Reduction in Irrigated Area	600 acres per year	Urbanization	Change from agricultural to M&I supply

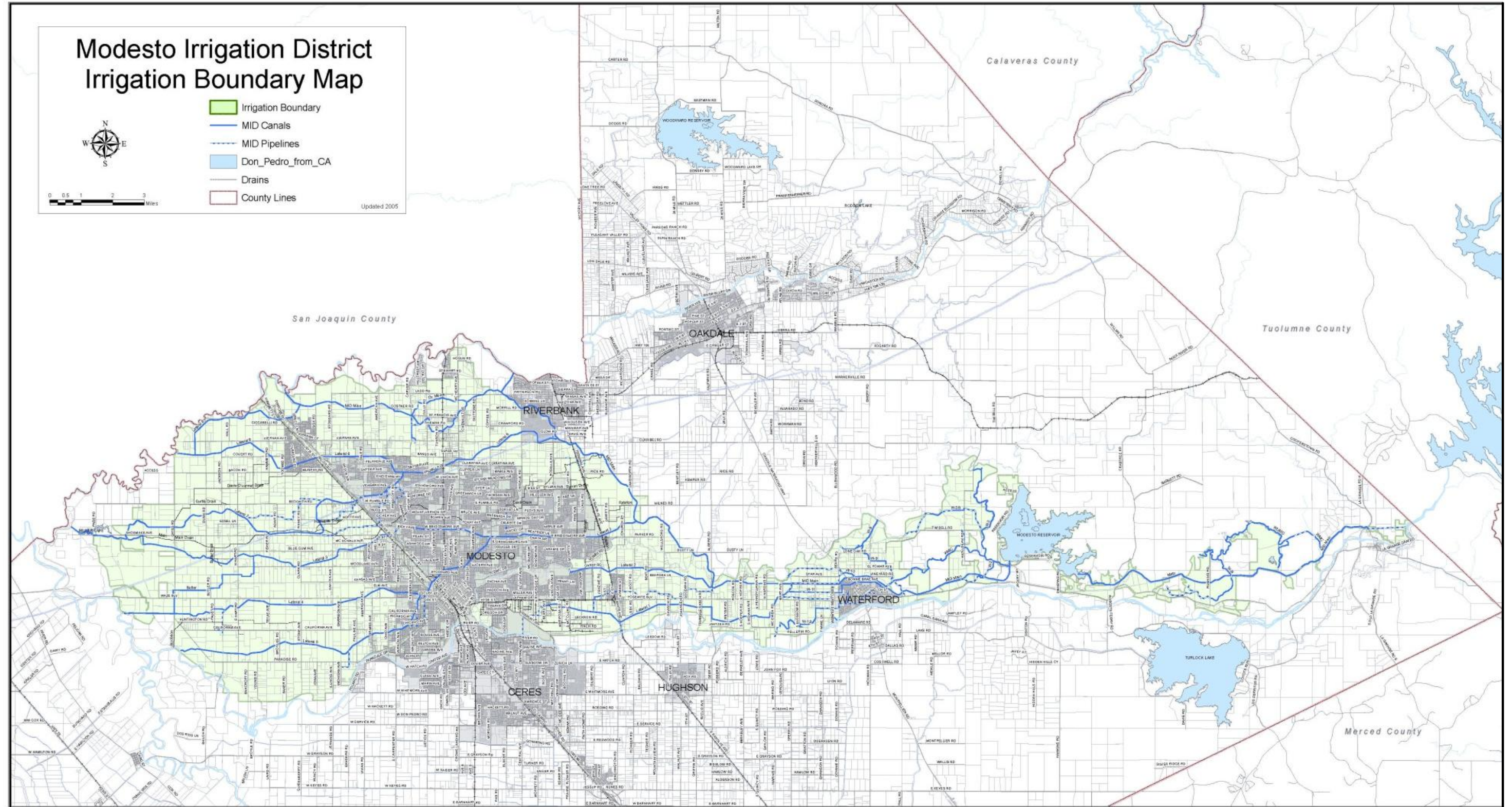


Figure 2 - MID Irrigation Service Area

## 2. Location of the Irrigation Service Area and Water Management Facilities

As shown on Figure 1, MID is located in northeastern Stanislaus County which lies in the northeastern part of the San Joaquin Valley. MID is bounded on the north by the Stanislaus River, on the south by the Tuolumne River, on the west by the San Joaquin River, and on the east by the Sierra Nevada foothills. Neighboring irrigation districts are Turlock Irrigation District (TID) to the south, Oakdale Irrigation District (OID) and South San Joaquin Irrigation District (SSJID) to the north, and West Stanislaus Irrigation District (WSID) and a few smaller water districts to the west. MID, TID, OID and SSJID all divert irrigation water from the Tuolumne (MID & TID) and Stanislaus (OID & SSJID) Rivers which provide high quality runoff from the Sierra Nevada Mountains.

Within the upper Tuolumne River watershed, the City and County of San Francisco (CCSF) operates three reservoirs with a total storage capacity of 656,000 acre-feet; in the lower part of the watershed, MID and TID (collectively the “Districts”) operate the New Don Pedro Reservoir with a maximum storage capacity of 2,030,000 acre-feet. The Districts are also responsible to maintain regulated fish flows in the Tuolumne River to comply with FERC licensing requirements. MID’s median annual diversion from the Tuolumne River is approximately 294,000 acre-ft of water (hydrologically average period from 2003 to 2012). Of that amount, approximately 32,900 acre-feet (average from 2003 to 2012) is delivered to the MRWTP for treatment and delivery to the City of Modesto.

MID distributes a combination of Tuolumne River water and groundwater via a network of storage facilities, canals, pipelines, pumps, drainage facilities and control structures. The District distribution system is shown in **Figure 2**. MID's first major project was the construction of La Grange Dam completed on December 13, 1893 in conjunction with TID, however its size precludes it from re-regulating water. This masonry dam is still used to divert water from the Tuolumne River into MID's Upper Main Canal. When La Grange Dam was built it was the highest overflow dam in the world. On June 27, 1903 irrigators along the newly completed main canal began receiving water, and by September of that year, water was moving through District laterals. Table 4 provides a summary of existing irrigation facilities in MID.

Table 4 - Water Conveyance and Delivery System

<b>System Used</b>	<b>Number of Miles</b>
Unlined Canals	15
Lined Canals	147
Pipelines	42
Drains	39

Storage and regulation of main canal deliveries began in 1911 with the completion of the 28,000 acre-foot Dallas-Warner Reservoir, now known as Modesto Reservoir. The capacity of this reservoir was too small to allow carryover water from year to year to protect permanent crops from extended droughts. Such storage wasn’t available until the completion of the Old Don Pedro Dam and Reservoir. When completed, in 1923, at a height of 284 feet, Old Don Pedro Dam was the

highest gravity dam in the world. Old Don Pedro Reservoir allowed MID and TID to store a maximum of 290,400 acre-feet of water for irrigation and recreation and to generate electrical power.

In 1970, MID again added to its water storage and power generation facilities with the completion of the 2,030,000 acre-foot New Don Pedro Dam and Reservoir. The New Don Pedro facilities are owned by MID and TID and operated by TID. The Districts also share pre-1914 water rights, water diversion facilities, and water right licenses.

New Don Pedro Reservoir is a multi-purpose water storage facility. In addition to storing water for irrigated agriculture and M&I use, water releases generate electricity, and the reservoir is used as a recreation and water sports facility. MID and TID also release water to increase instream flows which enhance the environment downstream of New Don Pedro Reservoir.

The City and County of San Francisco (CCSF) has an obligation to release specific flows from the Hetch Hetchy Project into New Don Pedro Reservoir depending on the time of year. In order to assist CCSF in managing available water while meeting the Districts' prior water rights, the Districts have agreed to allow CCSF to have a water bank of 570,000 acre-feet in New Don Pedro Reservoir. This water bank allows CCSF to pre-release water to the water bank when available, allowing CCSF to optimize their upstream operations while meeting with the District's senior water rights at all times. Whenever there is water in the water bank, CCSF is relieved of its obligation to meet District flow requirements.

There is also a U.S. Army Corps of Engineers (USACE) flood control storage requirement of 340,000 acre-feet of reservoir space that is maintained from October 7 to April 27 of each year. The minimum dead pool storage is 309,000 acre-feet leaving MID and TID with an average working capacity of 1,721,000 acre-feet of which MID's annual share is 31.54 percent or 542,803 acre-feet.

MID has a maximum annual carryover storage capacity of 570,803 acre-feet when storage in the 28,000 acre-foot Modesto Reservoir is included as shown in Table 5.

Table 5 - Water Supplier Reservoirs

<b>Reservoir</b>	<b>Capacity (AF)</b>	<b>MID's Storage Rights (AF)</b>
Modesto Reservoir	28,000	28,000
New Don Pedro Reservoir	2,030,000	542,803
Total Storage	2,058,000	570,803

The MID water conveyance and distribution system was designed to deliver water by gravity flow from La Grange Dam on the east to the San Joaquin River on the west. This gravity conveyance system is energy efficient, but occasionally creates operational outflows to downstream tributaries. While these operational outflows are of relatively high quality and generate no environmental impacts, they are a lost resource to MID. As part of the CWRMP, MID is in the process of evaluating facilities to capture and return operational outflows for reuse within the irrigation service

area. The District anticipates that it will be able to conserve thousands of acre-feet per year once middle and end of system regulating reservoirs are constructed to capture and re-circulate operational outflows, although there will be a significant cost to construct such facilities.

The need for on-farm surface drainage within the District is minimal, as the majority of the land within the irrigation service area is well drained. Much of the land is irrigated with the use of level basins allowing agricultural water users to retain all irrigation water applied on-farm within the parcels' boundaries. Table 6 summarizes the existence of tailwater/operational outflow recovery systems. Currently MID has no District-operated recovery system and tailwater returns to the District conveyance system are minimal. Some growers, especially at dairies, re-circulate their water on site.

Table 6 - Tailwater/Operational Outflow Recovery System

<b>System</b>	<b>Yes/No</b>
District Operated Operational Outflow Recovery	No
On-Farm Operated Tailwater/Operational Outflow Recovery	Yes

There have been substantial improvements to MID's main and secondary canals since they were built in the early part of the 20<sup>th</sup> century. These improvements have increased the effectiveness of water deliveries. In addition to the District facilities, irrigators constructed ditches and pipelines necessary to convey water from the District's canals to the irrigated fields. By the early 1920s, despite improvements to canals and other water service facilities, many private community ditches weren't being maintained. The lack of maintenance to these private ditches and lack of cooperation among the water users resulted in frequent water shortages and inadequate or inefficient water deliveries.

MID couldn't take on the financial burden of improving the private community ditches without raising taxes to all landowners within the District. As an alternative, the District initiated state legislation allowing for the establishment of local ditch and pipeline "Improvement Districts" (ID) within irrigation districts. The legislation to form "Improvement Districts" was sponsored by a local state senator and became state law in 1927.

Improvement Districts are small locally controlled districts within a larger irrigation district organized for the purpose of more equitably providing improvements to the land and water conveyance facilities serving that specific area's needs and are, in effect, legal subdivisions of the irrigation district. These Improvement Districts use the technical and financial expertise of the irrigation district, while leaving the basic decision of whether or not to make the improvements in the hands of those using the community facility. In general, the Improvement District landowners make facility improvement decisions that enhance the water delivery efficiency of the local system. Since the Water Code requires that two-thirds of the landowners within an Improvement District agree on the expenditures made to Improvement District facilities, conflicting interests can be a problem. However, Improvement Districts are valuable mechanisms for making improvements where most of the landowners have similar interests. Today, there are approximately 230 active Improvement Districts within MID.

### 3. Terrain and Soils

The terrain of the District is relatively flat and is composed primarily of alluvial fans sloping from east to west from the foothills to the San Joaquin River. Elevations range from over 200 feet above sea level on the east to less than 40 feet above sea level on the west. On the east, MID is intersected by Dry Creek which drains over 100 square miles of land from the foothills east of the City of Modesto and runs in a westerly direction before merging with the Tuolumne River near the City of Modesto.

Land within MID consists mainly of sediments that have formed the broad alluvial plains of the Stanislaus and Tuolumne Rivers, two perennial streams which flow in a southwesterly direction and discharge into the San Joaquin River. The topography on the eastern one-third of the District's service area consists mostly of hilly to rolling land sloping in a westerly direction. The western two-thirds of the service area are relatively flat with a mild westerly slope.

The predominant irrigation system in MID continues to be gravity-fed level basins. However, pressurized, low-volume drip or micro-sprinkler irrigation systems are now the system of choice for lands converting to permanent orchard and vineyard crops. For this reason, some land planted to permanent crops irrigated using level basins or impact sprinklers is being converted to low-volume irrigation systems. The current rate of conversion to low-volume micro-irrigation systems is estimated to be about 130 acres per year (Truepoint data base, 2011-2015 data). In some cases the flood systems are kept intact to provide occasional flood irrigation events for vermin control or leaching.

The soils of the District consist of a broad range of textures from sand to heavy adobe. The soils are distributed according to their position in six distinct physiographic areas: (1) alluvial flood plains; (2) basin lands; (3) young alluvial fans; (4) low alluvial terraces; (5) high alluvial terraces, partially eroded into rolling hills; and (6) uplands of the Sierra Nevada.

The eastern fringe of arable land occurs in the rolling hills of the upland range where the older granitic alluvium supports irrigated trees, mainly almonds. The western fringe consists of mixed alluvium of low relief with some occurrence of heavy adobe and clay containing alkali. Much of the alkali area has been reclaimed, and the soil supports pasture, row and other field crops and some permanent crops. The largest area of land within the basin rim consists of sand to sandy loam, which also supports a wide range of crops and growing conditions. Hardpan occurs mostly in the eastern and western edges of the District.

A portion of the MID irrigation service area is underlain by the Corcoran Clay, a formation originating from ancient lake deposits of clayey silt. This formation creates a low permeability boundary of 20 to 120 feet in thickness. Irrigation wells drilled in the areas where the Corcoran Clay is present penetrate aquifers both above and below the clay. However, some deeper wells are perforated exclusively below the Corcoran Clay as that is where the best quality water is found. Generally, wells screened mostly above the clay exhibit better production characteristics than those screened in zones below the clay. Although numerous silt and clay beds occur above and below the



Corcoran Clay, they are not correlated over large areas. Therefore, those beds are only of local importance to the confinement of groundwater.

Table 7 summarizes the topographic characteristics of the irrigated lands.

Table 7 - Landscape Characteristics

<b>Topography Characteristic</b>	<b>% of the District</b>	<b>Effect on Water Operations and Drainage</b>
Rolling Land	20% of irrigated land	Land is adaptable to sprinkler and micro-irrigation systems.
Flat Land	80% of irrigated land	Land is adaptable to flood and other types of irrigation systems.

#### 4. Climate

The major features of the climate are hot, dry summers and cool, wet winters. Temperature distribution is uniform throughout the area. Average annual rainfall increases from about 10 inches at the San Joaquin River to about 14 inches at the edge of the foothills with 12 inches in the City of Modesto area. Most of the precipitation occurs from December to March with little to none occurring during the summer months of June through August; the pattern for potential evapotranspiration (ET) and evaporation are just the reverse. Summer temperatures commonly are above 85°F and may exceed 100° F, but rarely exceed 105°F. Winter temperatures commonly fall below 32°F, but are rarely lower than 25°F. Table 8 summarizes climatic conditions for Modesto;

Table 9 presents more detailed information.

Table 8 - Summary Climate Characteristics

<b>Climate Characteristic</b>	<b>Annual Value</b>
Average Precipitation	12.15 inches
Precipitation (2012)	13.84 inches
Minimum Precipitation (1913)	4.30 inches
Maximum Precipitation (1983)	26.01 inches
Minimum Temperature (Avg. Winter)	39.6°F
Maximum Temperature (Avg. Summer)	91.3°F

Note: Data provided by Modesto Irrigation District 1888 to 2015

Table 9 - Detailed Climate Characteristics

Month/Time	Average Precipitation (inches) <sup>2</sup>	Average Reference Evapotranspiration (ET <sub>o</sub> ) (inches) <sup>3</sup>	Average Minimum Temperature, °F <sup>2</sup>	Average Maximum Temperature, °F <sup>2</sup>
January	2.33	1.1	38	54
February	2.06	1.88	42	61
March	1.90	3.57	44	67
April	0.95	5.23	47	73
May	0.49	6.98	52	81
June	0.10	7.87	57	88
July	0.02	7.95	60	94
August	0.03	6.89	59	92
September	0.21	5.1	57	88
October	0.62	3.4	51	78
November	1.33	1.7	43	64
December	2.12	1.05	39	55
Wet Season <sup>1</sup>	11.31			
Dry Season <sup>1</sup>	0.85			

Notes:

1 - Wet season typically October through April, Dry season is May through September

2 - Data provided by Modesto Irrigation District per MID Temperature Records since 01/01/1939

3 - ET<sub>o</sub> data from Modesto Station # 71 (1989-2011)

## **B. Operational Characteristics**

### **1. Operating rules and regulations**

*The Rules and Regulations Governing the Distribution of Irrigation Water Within the Modesto Irrigation District* (2015 revision) (Rules and Regs) is the guideline for the operation and delivery of irrigation water and is presented in **Appendix C**. The Rules and Regs cover the procedures followed to distribute irrigation water in an orderly, efficient, and equitable manner. The Rules and Regulations were updated in early 2015 with significant revisions to allow for improved water resources management. Major revisions were made on the following topics:

- Changes to irrigation scheduling procedures
- Fines for unauthorized water use (\$1,500 per infraction)
- Additional details on water measurement
- Irrigators must decide by May 1 if they will be irrigating
- Requirement for backflow prevention from lagoons and agricultural filter discharge stations
- Commitment to annually review the Rules and Regs

The MID on-farm water delivery system was originally designed to deliver irrigation water by gravity with very large flows, 10-20 cubic feet per second (cfs), to each field turnout on a predetermined rotation (typically every 10-20 days) basis. Water delivery on rotation can be an effective method to deliver water to flood irrigated level basins because the soil moisture holding

capacity of the crop root zone is utilized to store water for use by the crop until the delivery rotation comes back again. The time between irrigations is dependent on the water holding capacity of the soil and climatic conditions which drive the rate of evapotranspiration (ET), as well as the distribution system itself. However, as irrigators convert their on-farm application practices from flood to pressurized systems, the requests for irrigation water have shifted from rotation to arranged-demand as pressurized micro-irrigation systems need a smaller volume of water but irrigation must occur more frequently, often on a daily basis.

Most of the on-farm gravity water delivery systems were designed and built with cast-in-place pipelines and ditches capable of delivering large flows for flood irrigation on a rotation schedule. These pipelines and ditches typically hold water for only a few days as the rotation moves to other facilities downstream. On-farm arranged-demand delivery requires that water be available most of the time and be delivered at a constant low flow rate, a practice which creates an incompatibility between the delivery requirements of flood and low-volume on-farm systems.

Facing a deteriorating system of ditches and pipelines that wasn't capable of delivering water to the range of on-farm irrigation systems present within the District, the MID Board of Directors has approved funding to upgrade the District's water delivery system and to help landowners modernize their on-farm application systems. These upgrades and replacements enhanced water delivery flexibility and increased reliability. With District, improvement district, and private upgrades, MID is now capable of delivering irrigation water to a majority of its customers on a demand or an arranged demand schedule as summarized in Table 10.

Table 10 - Supplier Delivery System

Type	Check if Used	Percentage of System Supplied
On Demand	X	30
Arranged Demand	X	45
Rotation	X	25

MID operates a decentralized water ordering and delivery system. The ditchtenders take water orders from agricultural water users and coordinate deliveries based on demand and the flow capacity of the distribution system. As MID moves away from rotation to the more flexible arranged demand water delivery system, the ditchtenders' functions have become less routine and more customer-oriented.

Agricultural water users with flood irrigated lands may continue to irrigate on a fairly constant rotation while the water users with pressurized irrigation systems may request irrigation water on an arranged demand basis. Therefore, water order lead times vary depending on the time of year, system capacity, and where water is being routed. For example, an agricultural water user close to Modesto Reservoir with land near a large canal may have a greater probability of receiving water on short notice than an agricultural water user who is more distant from the reservoir and from delivery facilities. The District's goal is to supply water to the agricultural water user when the water is needed and to maintain that delivery for the duration necessary to refill the soil profile or to satisfy the crop water requirement.

Water Allocation Policy

Table 11 illustrates factors used to allocate water at MID on an annual basis. These factors are considered in setting the annual water allocation that is applied uniformly across the District (ag and urban) and which, in a normal year, is approximately 42 inches/year. The years 2012 and 2013 were drier than normal with allocations of 36 inches each year, while 2014 and 2015 were very dry years with allocations of 24 inches and 16 inches, respectively.

Table 11 - Water Allocation Policy

Basis of Water Allocation	(Check if applicable)			Allocation	
	Flow	Volume	Seasonal Allocations	Normal Year	Percent of Water Deliveries (%)
Land within the irrigation service area		X		42 in/year	100 %
Reservoir storage		X		42 in/year	100 %
Riparian rights					
Water Year Type		X		42 in/year	100 %
Amount of land owned					
Predicted runoff		X		42 in/year	100 %

The annual allocation is based on factors including the volume of water carried over in storage in New Don Pedro Reservoir and the projected runoff from the Tuolumne River watershed. The allocation generally isn't finalized on an annual basis until after the rainy season when runoff information has been made available by DWR.

Table 12 describes lead times for water orders and shut-offs now typical of MID operations. The lead time was recently increased from 3 days to 5 days, primarily to account for increased water management opportunities during prolonged droughts. While this is an upper bookend, orders are generally filled as soon as possible.

Table 12 - Actual Lead Times

Operations	Hours/Days
Water orders	0-120 hours
Water shut-off	0 hours

**Water Delivery Measurements or Calculations**

Following is a brief discussion on current water delivery measurements. The District is also performing an extensive delivery point water-measurement pilot-testing program as part of its efforts to comply with SBx7-7, and improve the accuracy of water measurement throughout the District. Refer to Section VIII for more information on delivery point measurement, the Pilot Program and SBx7-7 compliance.

MID uses a variety of devices and methods to measure water within its delivery system. Diversions from the Tuolumne River into the Upper Main Canal are measured continuously by the USGS gage number 11289000 (Modesto Canal near LaGrange). MID uses a Supervisory Control and Data

Acquisition (SCADA) system to monitor and control diversions from Modesto Reservoir and the various canal branches. Most deliveries to agricultural water users are currently measured using submerged sidegate orifices (commonly referred to as meter gates) that use the gate opening and the pressure differential between the canal and the downstream channel water levels to measure the water flow. When properly calibrated and with favorable field conditions, the submerged orifice can be a reasonably accurate method of measuring the instantaneous flow rate. MID has two portable Hach meters that are used to verify delivery flow rates when needed.

Table 13 shows typical levels of accuracy for various types of measurement devices currently used within the District.

The main disadvantage of calculating delivered water volumes based on an instantaneous measurement is that the measurement device doesn't directly record the volume of delivered water. This can be problematic for two reasons. First, an accurate record of the duration of the delivery must be maintained to convert the instantaneous measurement of flow rate into a volume. Secondly, if there are fluctuations in water surface elevations during the course of a delivery, these fluctuations will affect the rate of discharge, and hence, the volume of water delivered. In the case of MID, because the canal water level at nearly every check structure is controlled by a long-crested weir, there is little variation in canal water levels regardless of the flow in the canals. The District is able to maintain a fairly constant canal side, or upstream, water level on the meter gate, but the District has very little control on the landowner, or downstream, water level.

Ditchtenders calculate the volume of a water delivery by measuring the differences in water elevations and the meter gate opening, using calibrated tables to compute the flow rate which corresponds to these parameters, and multiplying that flow rate by the recorded duration of delivery. The calculated water delivery is input into the District's TruePoint water management system which tracks cumulative water delivered to each water user during the irrigation season. This data is then used to bill the agricultural water user on a volumetric basis. The current pricing system is volumetrically based, along with a fixed per acre charge.

Although the District is currently able to bill for water deliveries volumetrically, the District believes that the measurement methodology in some cases may be improved to increase agricultural water use efficiency. Section VIII of the AWMP discusses the proactive steps the District is taking to assess the most viable measurement device(s) and apportionment method and to comply with the water measurement requirements of SBx7-7.

Table 13 - Water Delivery Measurements

Type of Measurement	Frequency of Measure (Days)	Frequency of Calibration (Months)	Frequency of Maintenance (Months)	Est. Level of Accuracy (+/- % error)
Orifices	As required	Infrequently	As needed	10
Propeller meters	"	"	"	5
Weirs	Continuous (hourly)	Occasionally	"	10
Flumes	As required	Infrequently	"	7
Venturi meters	"	"	"	5
Pump, runtime	"	"	"	10
Pump, kwh	"	"	"	10
Hach Meter	As Required for flow rate verification	As per manufacturer recommendations	"	2-4

## 2. Water Rate Schedules and Billing

The MID Board annually establishes a water rate based on budget requirements and board policy. Factors such as cropping doesn't play a role in the Board's determination of water rates. Historically the District rates included a base water charge (per acre) that entitled the agricultural water user to use up to the allocated amount, and then an increasing block rate (tiered) pricing structure was applied for agricultural water users who exceeded the base amount of allocated water. In 2015, the District implemented a revamped water rate structure inclusive of a volumetric component to comply with one of the mandatory EWMPs of SBx7-7. The water rate structure used in 2015, and to be used in the future, is to assess a fixed charge (based on acres served) to all agricultural water users, and to volumetrically charge for all water use on an increasing block rate or tiered pricing structure. Raw water sent to the City of Modesto is billed at the same rate as agricultural water users. Table 14 indicates the basis for the District's water rates.

Table 14 - Water Rate Basis

Type of Billing	Check if Used	Percent of Water Deliveries (%)	Description
Volume of Water Delivered (acre-foot based)	X	100%	Tiered pricing structure for all lands receiving MID water
Fixed Charge – Land Assessment (acres based)	X	100%	Basic fixed charge applied to all lands, regardless of how much water is used
Crop			NA

As a result, MID has a pricing structure that combines a uniform fixed charge to all lands, along with a block rate structure with increasing price rates applied to lands receiving irrigation water. Table 16 provides the water rate structure. **Appendix D** provides detailed information on past and current water allocations and rates.

Table 15 - Rate Structure

<b>Type of Billing</b>	<b>Check if Used</b>	<b>Description</b>
Declining Block Rate		NA
Uniform	X	Based on annual allocation and rate
Increasing Block Rate	X	Based on annually defined block structure and associated rates

On March 24, 2015, the MID Board adopted the 2015 irrigation rates and volumetric pricing. The pricing structure consists of a fixed per acre charge and tiered pricing based on volume delivered. The volumetric pricing is structured as follows:

Table 16 – 2015 Volumetric Pricing Structure

<b>Category</b>	<b>Cost\$/Acre (AC) or \$/Acre Foot (AF)</b>
Fixed Charge <sup>1</sup>	\$40.00/AC
Volumetric – Tier 1 (up to 24")	\$1.00/AF
Volumetric – Tier 2 (24" to 36")	\$2.00/AF
Volumetric – Tier 3 (36" to 42")	\$3.00/AF
Volumetric – Tier 4 (42" and up)	\$10.00/AF

1 - Facilities and Maintenance charge will be ½ of the fixed charge or \$20 per acre. No minimum charge will be applied.

The Farmer to Farmer and Allocation Return Program described later under *Drought Management Plan* also have volumetric pricing. Some pricing tiers didn't apply for the 2015 season, as there was a capped allocation of 16 inches.

The drought has required the District to increase groundwater pumping and rely on several drought management programs to help meet water demands. As a result, MID implemented a special drought surcharge in 2015 to account for additional drought-related operational expenses. The drought surcharge was calculated using 2014 actual drought expenditures. Given the added scarcity of available water supplies in 2015, MID found this approach to be representative of ongoing and planned 2015 drought operations. The 2015 drought surcharge of \$16 per irrigated acre was intended to cover the following additional expenditures incurred as part of MID's 2015 drought operations:

1. Additional electrical costs
2. Additional manpower
3. Additional pump maintenance costs
4. Use of an outside security guard for added patrols of the conveyance system.

Currently MID bills its agricultural water users annually at the end of the irrigation season, as shown on Table 17. This bill is payable in two equal installments due on or about December 20 of the same year and June 20 of the following year. Water is currently billed, at least in part, on a volumetric basis.

Table 17 - Frequency of Billing

Frequency	Check if Used
Annually	X

### 3. Water Shortage Allocation Policies

Water supplies on the Tuolumne River vary depending on watershed precipitation, snow melt runoff, and the prior year's carryover storage in New Don Pedro Reservoir. As such, water supply planning must take into consideration the amount of water that will be available when the irrigation season starts, the current year water requirements, and the expected carryover for the following season. MID has developed an internal planning tool to determine the annual allocation of water available to its customers (ag and urban). This tool identifies all of the estimated water resources available to MID within a given irrigation season, adjusts for the estimated commitments and accounts for the number of irrigated acres, and forecasts the final allocation and carryover storage of water MID can provide to its customers (ag and urban).

MID also implemented a Drought Management Plan (DMP) in 2014 and 2015, including several special programs to conserve and redistribute water. The DMP is discussed below in Section II.B.4. During consecutive dry years, MID may decrease the water allocation and shorten the irrigation season. MID will also conjunctively use groundwater pumps to supplement surface water diversions during years of short supply, and agricultural water users may turn on their private irrigation wells to supplement District-supplied water. These practices are documented in MID Policy 89-77. Table 18 lists the measures that MID may exercise to respond to water shortages.

Table 18 - Decreased Water Supplies Allocation

Allocation Method	Check if used
Decrease Allocated Water	X
Shorten Irrigation Season	X
Restrict Water to Certain Crops	NA

Section 4.2 of the *Rules and Regulations Governing the Distribution of Irrigation Water within the Modesto Irrigation District* specifically addresses consequences to agricultural water users who waste water. Section 4.2.3 states the following:

*“4.2.3. The District may refuse to deliver District water to any Irrigator who misuses or wastes water either willfully or carelessly, in any way, including but not limited to the following:*

*4.2.3.1. Flooding of roads, vacant land, or land previously irrigated.*

*4.2.3.2. Defective or inadequate non-District Canals or Facilities.*

*4.2.3.3. Inadequately prepared land.*

*4.2.3.4. Flooding any part of any land to an unreasonable depth or amount, including for the purpose of irrigating other portions of the land.*

*4.2.3.5. Flooding across one parcel to irrigate another parcel.”*



Table 19 summarizes enforcement methods available to curtail wasteful water uses.

Table 19 - Enforcement Methods of Allocation Policies

<b>Enforcement Method</b>	<b>Check if used</b>
Shut-off of Water	X
Refuse service	X
Fines/Penalties	X

**Basis for Reporting Water Quantities**

Given water year types which have ranged from critical to wet in the recent past, MID chose 2012, a dry year type which was preceded by a very wet year (2011) and followed by two very dry years (2013 and 2014), as the representative year to serve as the basis for reporting water use and water supply data listed in subsequent tables. Although 2012 was dry (Tuolumne River natural flow was only 45% of long-term average), it was preceded by a very wet year, resulting in a near average surface water allocation for the District (99% of long-term average).

Figure 3 displays a time series of key hydrologic parameters extending from 1972 (the year New Don Pedro Dam was commissioned) through 2014. This figure illustrates the great range of computed natural flow (CNF), Tuolumne River flows below La Grange Dam, and New Don Pedro Reservoir maximum storage which characterize the system. Figure 3 also illustrates that in spite of great fluctuations in CNF, MID diversions have remained relatively stable.

The selection of calendar year 2012 as the representative year is presented in Table 20.

Table 20 - Representative Year

	<b>Description</b>
<b>Representative Year based upon</b>	2012
<b>First month of representative year</b>	January
<b>Last month of representative year</b>	December

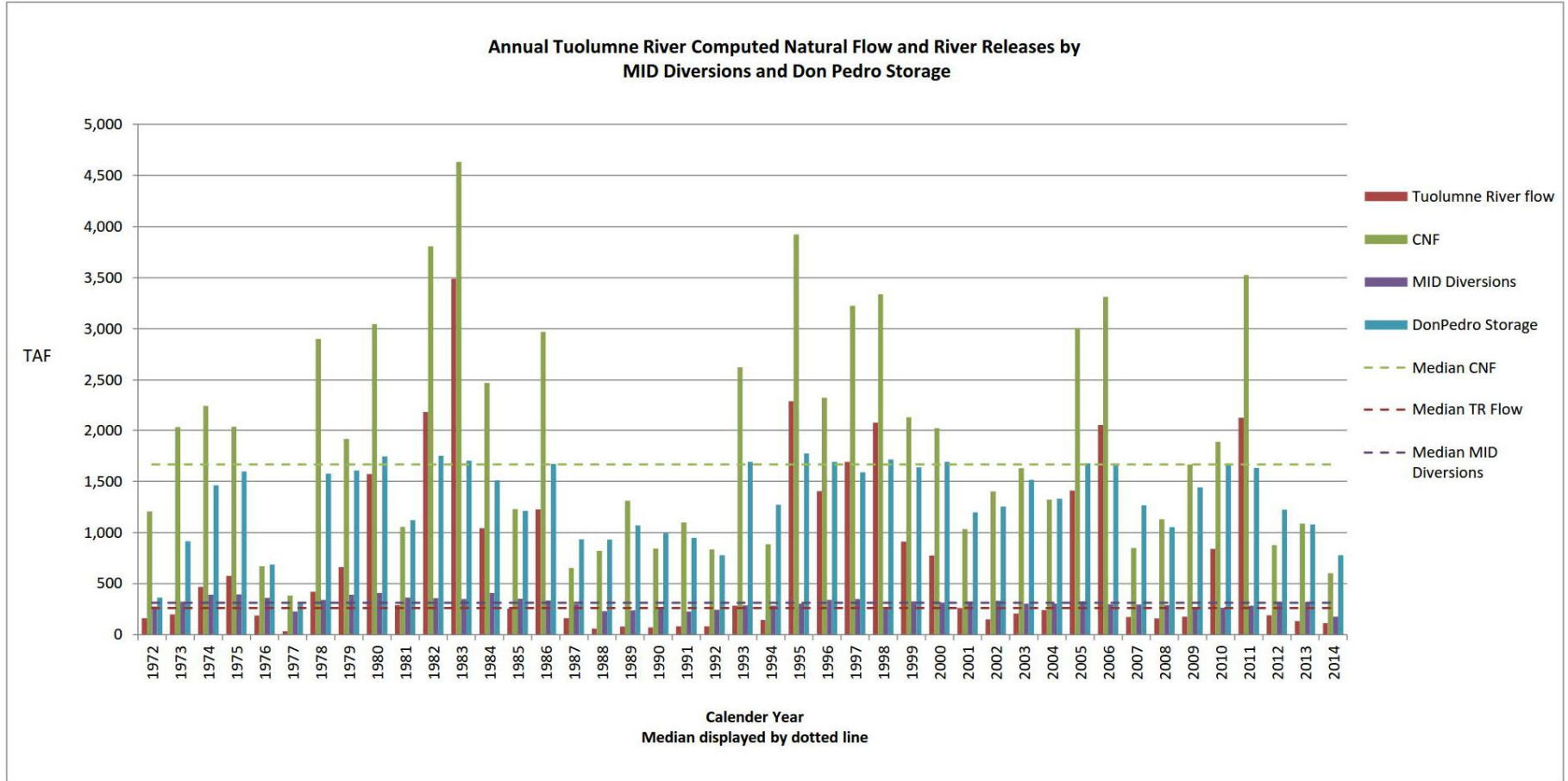


Figure 3- Annual Tuolumne River Computed Natural Flow and River Releases by MID Diversions and New Don Pedro Storage

#### 4. Drought Management Plan

In September 2015, MID developed a detailed Drought Management Plan (Plan) (see **Appendix E**). The Plan defines three ‘Levels of Surface Water Shortage’ and a variety of innovative strategies to cope with drought for each level. Most of these strategies have already been used during the current prolonged drought, and have proven effective in conserving water and sustaining crops. The current Drought Management Plan is subject to revision by the MID Board.

MID, in cooperation with neighboring Turlock Irrigation District, held a drought workshop entitled ‘*Farming in a Drought*’ on May 29, 2014. **Appendix E** includes flyers from the workshop. Topics covered included effective deficit irrigation strategies, suggestions for coping with dry conditions, and technical and financial assistance opportunities.

With a capped allocation of 24 inches of water per acre in 2014 (half of the historical average), MID created several special drought tools and programs to help growers have a successful season. First and foremost, MID found it important to increase all levels of communication with their agricultural water users. Through letters, Board meetings and a drought webpage, MID kept agricultural water users updated and informed throughout the irrigation season. An online water usage report was created to help agricultural water users keep track of how much water they were using and more importantly how much water they had left. To monitor usage, MID also increased patrolling along the canals to ensure efficient use of water consistent with the Rules and Regs. With the continuation and worsening of the drought in 2015 (allocation of only 16”), the programs and procedures initiated in 2014 were expanded and improved in 2015.

##### Special Drought Programs

For the 2014 irrigation season, the MID Board approved three special voluntary drought programs, which continued in 2015:

**Farmer to Farmer Delivery Program** – Eligible landowners were able to transfer all or a portion of their surface water allocation to other landowners in MID boundaries.

**Allocation Return Program** – Eligible landowners had the opportunity to sell back their allotted water to MID for \$400 per acre (\$200 per acre foot) to create a supplemental water supply. Other landowners could then apply to buy the supplemental water for \$200 per acre foot.

**Water Management Alternative Program** – Eligible landowners had the opportunity to deliver privately pumped well water into MID facilities for water allocation credit during the 2014 irrigation season.

These programs were successfully implemented in 2014. Data on the number of agreements and potential benefits are listed in Table 21.

Table 21 – Implementation of Drought Management Plan

<b>Special Drought Program</b>	<b>Number of Agreements</b>	<b>Level of Participation in 2014 (AF)<sup>1</sup></b>
Farmer to Farmer Delivery Program	194	3,300
Allocation Return Program	Cont- 15	Cont – 2,700
	Rec - 28	Rec – 1,060
Water Management Alternatives Program	3	275

1 – Level of participation is approximate

More information on these programs is available on the MID website at:

<http://www.mid.org/water/drought/default.html>.

### **Section III. Description of Quantity of the Water Uses of the Agricultural Water Supplier**

Tuolumne River water is diverted to storage in the New Don Pedro Reservoir and re-diverted downstream at La Grange Dam into the District's canal system under water right licenses issued by the State Water Resources Control Board (SWRCB). The District also diverts water according to a series of pre-1914 appropriative and storage rights recognized by the State of California. In addition, MID also maintains 93 water wells (including production and drainage wells) that are used to supplement the surface water supply, particularly during consecutive dry years.

#### **A. Agricultural Water Use**

The primary crops grown within the MID irrigation service area are deciduous trees (mostly almonds), grape vines, grains, row crops, and pasture for livestock. The District serves approximately 3,100 irrigation accounts with an average of 20 acres per account. Improvements in irrigation water delivery systems and changing economic conditions have brought many changes to the crop mix within the District. Nut trees including almonds and walnuts have been the crops with the most rapidly expanding acreages. During the last several years, thousands of acres of pasture and annual crop land have been converted to orchards and other high value permanent crops.

As the cropping pattern changes, low-volume irrigation systems such as drip and micro-sprinkler are replacing flood irrigation resulting in improvements in on-farm irrigation water use efficiency. Despite these changes the total water requirement for the MID irrigation service area has remained constant over the years as the total annual crop water requirement doesn't appreciably change with a corresponding change in irrigation system. Table 22 summarizes the agricultural water use within the District in 2012.

Table 22 - Agricultural and Municipal Water Use for 2012

<b>Source</b>	<b>2012 (AF)</b>
<b>Agricultural Water Supplies</b>	
Surface and groundwater <sup>1</sup>	296,100
Other (City of Modesto M&I use) <sup>2</sup>	32,660
<b>Other Water Supplies</b>	
Surface Water	NA
Groundwater (Private Pumping)	NA
Other	NA

<sup>1</sup>Includes total surface water diversions – M&I deliveries + MID production wells + MID drainage wells

<sup>2</sup>Includes MID surface water deliveries to the City of Modesto. Does not include City of Modesto groundwater pumping.

Table 23 describes water needs for specific crops grown within MID’s irrigation service area.

Table 23 - Agricultural Crop and Water Demand Data for 2012

<b>Crop Category</b>	<b>Total ET Demand (in/yr)</b>	<b>Acres</b>	<b>Total ET Demand (AF/yr)</b>
Alfalfa Hay and Clover	40.23	3,034	10,200
Almonds	35.52	23,758	70,300
Apple, Pear, Cherry, Plum and Prune	35.04	54	200
Corn and Grain Sorghum	24.76	10,204	21,100
Flowers, Nursery and Christmas Tree	33.47	183	500
Grain and Grain Hay	14.96	859	1,100
Grape Vines with 80% canopy	24.16	1,415	2,800
Idle	0.00	3,138	0
Melons, Squash, and Cucumbers	14.96	3	0
Misc Subtropical	33.47	2	0
Misc. Deciduous	33.47	1,332	3,700
Misc. field crops	21.76	340	600
Pasture and Misc. Grasses	41.18	9,373	32,200
Peach, Nectarine and Apricots	33.10	2,526	7,000
Rice	35.74	506	1,500
Small Vegetables	15.14	1,101	1,400
Strawberries	21.76	29	100
Walnuts	37.70	8,594	27,000
	<b>TOTAL</b>	<b>66,451</b>	<b>179,700</b>
	<b>Double Cropping (assumed)</b>		<b>5,000</b>
	<b>Total</b>		<b>184,700</b>

Notes: 1 - ET Demand takes into account contribution from effective precipitation  
 2 - Calculations performed using regional ET rates for a Typical Year published by the Irrigation Training & Research Center (ITRC)

The District's gross irrigation service area encompasses approximately 103,733 acres. As shown on Table 24, in 2012 approximately 63,313 acres (66,451 acres less 3,138 idle acres) were irrigated with surface water, MID groundwater and private groundwater. Total evapotranspiration demand of applied water (after effective precipitation) was 184,700 AF.

The majority of the non-irrigated land in the irrigation service area is within the City of Modesto’s sphere of influence.

Since submission of MID’s 2012 AWMP, MID has also started using remote sensing data to determine evapotranspiration (ET) within its irrigation service area. Mapping of EvapoTranspiration with Internal Calibration (METRIC) computes ET using LandSAT Thematic Mapper (LandSAT) data. While METRIC-based ET data wasn’t available for the 2012 water budget year used in this AWMP, a comparison of ET from 2010 (calculated using METRIC) showed that the ET was within approximately 5% of the calculated ET using standard ET rates for water balances as published by ITRC. Moving forward in subsequent planning cycles, MID expects to continue using METRIC-based ET data to better define its water balance.

Table 24 - Irrigated Acres for 2012

Irrigation Service Area	103,733
Surface and Groundwater Irrigated Area	63,313

For the purposes of this AWMP, cropped acres are essentially the same as irrigated acres. The amount of irrigated land that isn’t cropped at any time during the year is shown on Table 23 as Idle. Over 50 percent of the cropped acres are planted with permanent crops with almonds being the predominant permanent crop with 23,758 acres. Permanent crops cover about 36,266 acres, and pasture and grain crops used primarily for dairy cattle feed cover about 23,470 acres. All other crops cover less than 4,000 acres. Land planted to grain crops is typically double cropped during the winter and spring months with winter forage also used primarily for dairy cattle. As shown in Table 25, inter-cropping isn’t a common practice within the MID irrigation service area.

Table 25 - Multiple Crop Information for 2012

Cropped	63,313 acres
Inter-cropping	Negligible
Double Cropping	Not available

Based on cropping records, it is estimated that about 74% of cropped land is irrigated with flood/furrow irrigation, 25% is irrigated with high efficiency drip or sprinkler irrigation, and about 1% of the area has no data.

**B. Environmental Water Use**

MID and TID own New Don Pedro Dam and Reservoir and operate these facilities under a license from the Federal Energy Regulatory Commission (FERC). The Districts are currently in the process of renewing the FERC license. The FERC license currently requires minimum releases of between 94,000 and 301,000 acre-feet per year downstream of the dam to protect fisheries, specifically salmon. As a result of an agreement signed in 1995, the minimum flows below La Grange Dam are based on a 10-step water year classification as used by DWR. During wet years the mandated minimum flows are as high as 300 cfs and in consecutive dry years as low as 50 cfs. In addition to the minimum flows, MID and TID release pulse flows in the spring to encourage juvenile salmon to migrate downstream through the Delta and into the open ocean. They also release fall attraction flows to entice and encourage salmon to return to the river for spawning. The actual pre-release flood flows can be several thousand cfs during wet winters. The required minimum flows may be revised in the future as a result of the FERC license renewal.

Required minimum flows have an impact on the amount of water available for beneficial uses. Storage limitations imposed by the State Water Resources Control Board, the minimum in-stream flow requirements imposed by the FERC, and flood control rules issued by the USACE, are all factors that govern storage and releases from New Don Pedro Reservoir. The volume of in-stream flow releases shown in Table 26 is based on the 2012 FERC minimum flow requirement; MID’s share in 2012 was 33,825 AF.

Table 26 - Environmental Water Uses for 2012

<b>Environmental Resources</b>	<b>Volume (AF)</b>
In-stream flow releases	33,825 <sup>1</sup>
Streams	0
Lakes or reservoirs	0
Riparian vegetation	0
<b>Total</b>	<b>33,825</b>

1 - The boundary for the MID water balance presented in the AWMP begins at LaGrange Dam, the point where MID diverts water from the Tuolumne River. Since, instream flow releases from New Don Pedro Reservoir aren’t diverted at LaGrange Dam, these releases are an element of MID operations, but aren’t included in the accounting of water diverted into the irrigation system that is presented in the AWMP water budget.

### **C. Recreational Water Use**

New Don Pedro Reservoir, also known as Don Pedro Lake, has a capacity of 2,030,000 acre-feet. Recreational activities at Don Pedro Lake include swimming, camping, fishing, and boating. MID, TID, and the CCSF are partners in the operation of the Don Pedro Recreation Agency (DPRA) which administers the recreational activities at Don Pedro Lake.

Modesto Reservoir is also a popular recreational facility offering activities similar to those available at Don Pedro Lake. MID is the sole owner of Modesto Reservoir. Through an agreement, MID leases the recreational facilities at Modesto Reservoir to the County of Stanislaus.

Table 27 summarizes the facilities’ non-consumptive recreational water uses. As seepage and evaporation from New Don Pedro Reservoir occur outside of the boundary of the AWMP water balance, and as seepage and evaporation from Modesto Reservoir are accounted for as losses which would occur with or without recreational activity, there are no consumptive uses attributable to recreation that apply to the AWMP water balance.

Table 27 - Recreational Water Uses for 2012

<b>Recreational Facility</b>	<b>Volume (AF)</b>
New Don Pedro Reservoir <sup>1</sup>	1,685,000
Modesto Reservoir <sup>2</sup>	32,000
<b>Total</b>	<b>1,716,20</b>

1 - USGS Water Data Report for 2012.

2 - Includes reservoir seepage and evaporation

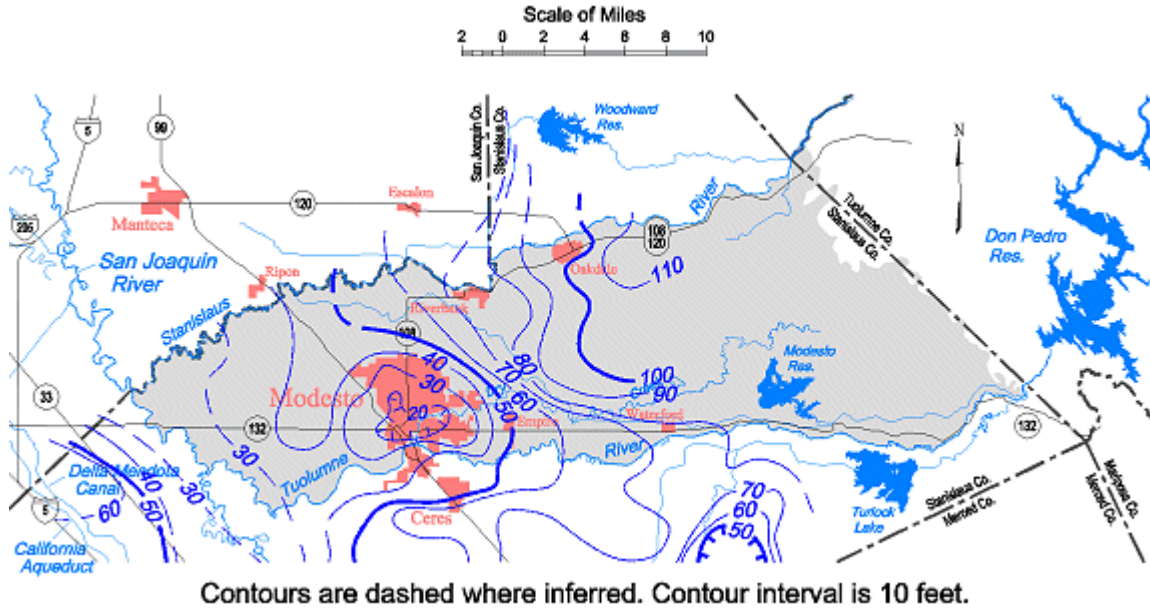


**D. Municipal and Industrial Water Use**

Prior to 1995 all M&I water use in the MID irrigation service area was from groundwater pumping. The City of Modesto, other local communities, rural residences and businesses all pumped groundwater from the Modesto Groundwater Sub-basin for domestic and commercial uses. Beginning in the 1940’s, drought conditions and the communities' growth demands contributed to a reduction in groundwater levels and created a cone of depression under the City of Modesto. This cone of depression, combined with increasingly stringent federal and state water quality requirements, prompted a 1983 study of the groundwater supply that recommended a conjunctive water use program that would supplement the M&I groundwater supply with water from the Tuolumne River. Following the recommendations of the 1983 study, in 1986 MID and the City of Modesto signed an agreement to allow MID to pursue the construction of a surface water treatment plant to supply treated water from the Tuolumne River to the City of Modesto. In 1994, MID completed Phase One of the Modesto Regional Water Treatment Plant (MRWTP), a 30-million gallon per day (33,000 acre-foot/year) domestic water project. Since its completion, the plant has been operated by MID and provided approximately 600,000 AF of treated Tuolumne River water to the City of Modesto. Absent this cooperative local agreement, that volume of water would have come from the Modesto Sub-basin. The City still pumps groundwater to meet their remaining needs, but as intended, the delivery of Tuolumne River water to supply the area’s urban needs has contributed to the significant rebound of groundwater levels within the Modesto Sub-basin. Since 1994 groundwater levels beneath the City of Modesto have rebounded by approximately 20’. Depth to groundwater maps for 1994 and 2010 are shown as **Figures 4 and 5**. With completion of Phase II MRWTP by the end of 2015, MID will have the capacity to deliver up to 70,000 AF of treated surface water to the City of Modesto or nearly 85% of the City of Modesto’s estimated 2015 demand as published in the City of Modesto/MID 2010 Joint Urban Water Management Plan (UWMP).

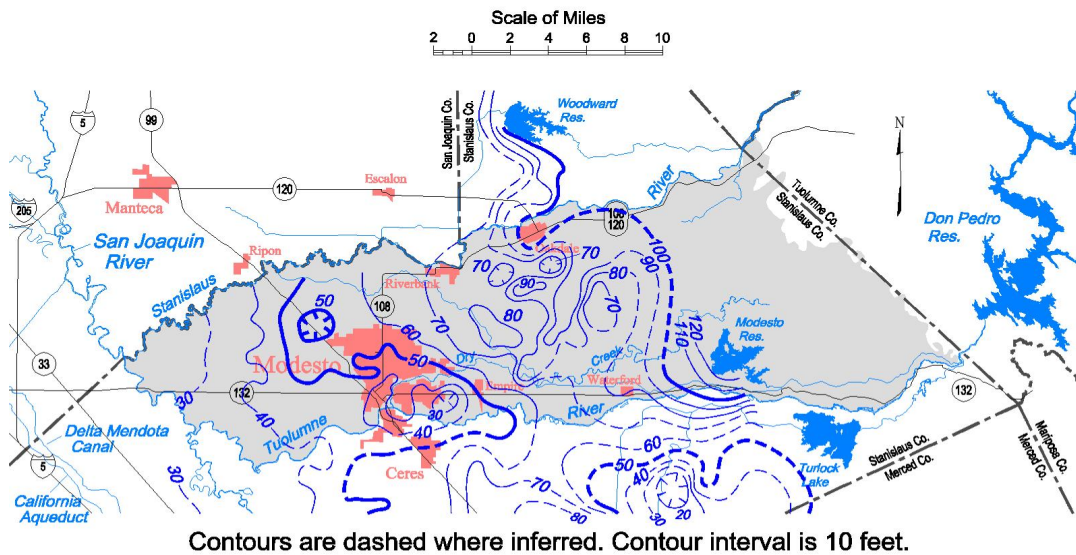
Table 28 - Municipal/Industrial Water Uses for 2012

<b>Municipal/Industrial Entity</b>	<b>2012 Volume (AF)</b>
Municipal Entity	
City of Modesto – Surface Water (from MID)	32,700
City of Modesto - Groundwater (City wells)	28,700
Industrial Entity	
NA	
Total	61,400



Source: California Department of Water Resources

Figure 4 - Modesto Groundwater Basin, Spring 1994 Groundwater Elevations, Unconfined Aquifer



Source: California Department of Water Resources

Figure 5 - Modesto Groundwater Basin, Spring 2010 Groundwater Elevations, Unconfined Aquifer

### E. Groundwater Recharge Use

Most of the groundwater recharge in the Modesto Groundwater Sub-basin is the result of deep percolation of applied surface water to agricultural lands, seepage from canals and reservoirs, and

deep percolation of precipitation and urban storm runoff. Seepage from Modesto Reservoir is estimated to be approximately 24,000 acre-feet per year. Approximately 91 percent of MID canals are concrete lined; thus, the amount of canal seepage is relatively small.

Groundwater recharge also occurs in the City of Modesto through deep percolation of landscape irrigation water. Wastewater from the City of Modesto is treated within the City but then exported outside of MID’s irrigation service area and doesn’t contribute to local crop water demands or groundwater recharge. Deep percolation of City stormwater is included in a water balance parameter Deep Percolation from Precipitation, which is assumed to be 20% of all precipitation falling over the District’s irrigation service area.

The overall efficiency of on-farm irrigation application in MID is assumed to be approximately 63 percent when the efficiencies of both level-basin and low volume application systems are combined (USGS, 2004). Because on-farm runoff from MID fields is negligible, the remaining 37 percent of the applied water is assumed to be destined to groundwater recharge with a portion of this recharge satisfying leaching requirements. Total groundwater recharge is estimated to be 152,000 acre-feet in 2012 (see Table 29). The deep percolation from irrigation exceeds the requirements for crop leaching and, therefore, satisfies the leaching requirement. The importation of surface water contributes substantially to the local groundwater recharge, and far exceeds estimates of groundwater inflow to the District.

It is anticipated that as irrigation methods evolve from surface irrigation to more efficient low-volume micro-irrigation systems, there may be a negative impact on the effective amount of groundwater recharge since the majority of groundwater recharge is obtained currently through on-farm irrigation. Increasing the efficiency of an on-farm irrigation system in a conjunctive use district may reduce the total amount of applied water, but won’t have a net positive effect on the groundwater because less deep percolation will occur.

The University of California Davis is currently researching the feasibility of flooding permanent crop fields during dormancy as a means of recharging groundwater. This could be a viable opportunity for MID and other local agencies to replenish groundwater within the Modesto Sub-basin. Artificial recharge projects are likely to be developed in MID to achieve continued sustainability and compliance with SGMA.

Table 29 - Groundwater Recharge Water Uses for 2012

<b>Location/Groundwater Basin</b>	<b>Method of Recharge</b>	<b>2012 (AF)</b>
MID Service Area	On-farm Irrigation <sup>1</sup>	108,500
MID Service Area	Canal Seepage	8,000
Modesto Reservoir	Reservoir Seepage	24,000
Modesto Urban Area	M&I Deep Percolation	11,500
<b>Total</b>		<b>152,000</b>

<sup>1</sup> – Calculated assuming all ET demands are met and a 63% irrigation efficiency

**F. Transfer and Exchange Use**

During the 1987 through 1992 drought, MID transferred several thousand acre-feet of water to CCSF. The water was released into the San Joaquin River and was pumped to the CCSF service area through a cooperative agreement between CCSF and the Santa Clara Valley Water District. MID has also participated in the transfer of water through a U. S. Bureau of Reclamation program for river and fishery enhancement known as the Vernalis Adaptive Management Program (VAMP). Under VAMP, between 1999 and 2010 pulse water flows required during critical stages of salmon growth were released from various San Joaquin River tributary reservoirs in the spring to entice young salmon to leave the spawning areas and swim to the Delta. Table 30 summarizes MID activity in water transfers in 2012. The District hasn't transferred any water outside its irrigation service area from 2010 to 2014.

For the 2014 irrigation season, the MID Board approved three special voluntary drought programs, which continued in 2015. The Farmer to Farmer Delivery Program allowed eligible landowners to transfer all or a portion of their surface water allocation to other landowners in MID's irrigation service area. This allowed for redistribution of water supplies (on a voluntary basis) and help local agricultural water users to better meet their water demands. Values from this internal transfer program are not reflected in Table 30.

Table 30 - Transfers and Exchanges Water Uses for 2012

From What Agency	To What Agency	Type of Transfer or Exchange (Ag to M&I, M&I to Ag, or Ag to Ag)	Volume (AF)
Modesto ID	-	-	0

**G. Other Water Use**

All water uses of any significance have been described previously in this section. Negligible volumes of water are used within the District for livestock watering, mixing with agricultural chemicals before spraying, and dust abatement. Table 31 notes that the cumulative water use for these purposes is insignificant.

Table 31 - Other Water Uses for 2012

Water Use	2012 (AF)
No other uses of significance	NA

**H. Projected Water Use**

As the developed areas of the City of Modesto and other communities within the MID irrigation service area expand, irrigated land is being replaced by urban land uses. As noted earlier, in 2012 MID delivered 32,660 acre-feet to the MRWTP for the City of Modesto. With completion of Phase II by the end of 2015, MID will have the capacity to deliver up to 70,000 AF of treated surface water to the City of Modesto or nearly 85% of the City of Modesto's estimated 2015 demand as published in the City of Modesto/MID 2010 Joint Urban Water Management Plan.

Future changes in agricultural water use will be driven by changes in cropping, irrigation practices, climate change, and fluctuations in the hydrology of the Tuolumne River watershed. Although the irrigated service area within MID is expected to remain relatively stable, even considering the impacts of urban expansion, changes in the availability of surface water will continue to influence the annual allocation of water.

Given the unknown nature of the impacts of climate change, as well as possible regulatory impacts on water supply from the FERC relicensing process and the Bay-Delta restoration process, it appears likely that surface water supplies will become less dependable which will lead to an increasing reliance on groundwater and on the conjunctive management practices needed to sustain groundwater elevations. Among the consequences of any future increases in groundwater pumping needed as a substitute for surface water delivered by gravity will be an increase in the energy required for groundwater pumping, as well as the air quality impacts of increased energy use.

## **Section IV. Description of Quantity and Quality of the Water Resources of the Agriculture Water Supplier**

### **A. Water Supply Quantity**

#### **1. Surface Water Supply**

Water that flows from New Don Pedro Reservoir and is re-diverted at La Grange Dam flows through the MID Upper Main Canal and into the Modesto Reservoir. Some water is supplied to water users directly from the Upper Main Canal before it arrives at Modesto Reservoir. From Modesto Reservoir water is diverted into the lower lying downstream irrigation canals for delivery to agricultural lands. Water is diverted directly from the Modesto Reservoir to the MRWTP. Table 32 shows MID’s water diversions from the Tuolumne River for the years 2010-2014 in acre-feet per year. Table 33 lists restrictions or imposed limitations on sources of MID water supply.

Table 32 - Surface Water Supplies– Agricultural and Municipal for 2012

Source	Diversion Restriction	2010	2011	2012	2013	2014
MID Water Diverted from the Tuolumne River at La Grange	Water year type, conveyance capacity and licenses	261,728 AF	282,640 AF	311,500 AF	316,571 AF	176,087 AF

Table 33 - Restrictions on Water Sources

Source	Restrictions or Imposed Limitations	Name of Agency Imposing Restrictions	Operational Constraints
Tuolumne River	Pre-1914 Water Rights Pre-1914 Storage Rights	Prior appropriation and use	Limited to unimpaired flow
Tuolumne River	Storage Rights	SWRCB	SWRCB license limits
Tuolumne River	Minimum In-Stream Flow Requirements	FERC	In-stream water volume and rate of change in river flow, water year type, FERC license requirements
Tuolumne River	Flood Control	USACE	USACE flood control rule curve

#### **2. Groundwater Supply**

Groundwater is pumped in the MID irrigation service area to supplement the surface water supply and to help control high water tables on the west side of the District. The combined pumping capacity of the approximately 93 groundwater wells owned by the District (including production wells and drainage wells) is approximately 250 cfs. However, based on MID's experience during prolonged droughts, pumping at this rate by MID, combined with pumping by other users within the Modesto Groundwater Sub-basin, wouldn't be sustainable over extended periods of time.

MID participates in local groundwater management through strategic operation of district-owned production and drainage wells. Groundwater management at the sub-basin level is achieved through cooperation with the Stanislaus and Tuolumne Rivers Groundwater Basin Association (STRGBA, <http://www.strgba.org/news/>). The STRGBA was created to provide a forum in which the participating

agencies could work cooperatively to provide coordinated planning to make the best use of available water resources of the subbasin to meet the needs of the agencies, and to accomplish the Association's stated purposes. The purposes of the Association are:

- To determine and evaluate the subbasin's groundwater supply
- To promote coordination of groundwater management planning activities, including the preparation of the groundwater management plan.
- To develop a hydrologic groundwater model of the groundwater basin.
- To determine the subbasin's need for additional or improved water extraction, storage, delivery, conservation, and recharge facilities.
- To provide information and guidance for the management, preservation, protection, and enhancement of groundwater quality and quantity in the subbasin.

The current members of the STRGBA include: City of Modesto, MID, City of Oakdale, Oakdale Irrigation District, City of Riverbank, and Stanislaus County. Revisions to the existing MOU includes addition of the City of Waterford, the remaining local agency within the subbasin.

Some previous documents prepared by the STRGBA include:

1. Recharge Characterization for Stanislaus and Tuolumne Rivers Groundwater Basin Association, WRIME, May 2007
2. Integrated Regional Groundwater Management Plan for Modesto Subbasin, Bookman-Edmonston, June 2005.

MID, through its involvement with the Stanislaus and Tuolumne Rivers Groundwater Basin Association (STRGBA), was instrumental in contracting with the USGS to conduct a basin groundwater study. The 2004 study, entitled "*Hydrogeologic Characterization of the Modesto Area, San Joaquin Valley, California*", provided the District and the other basin water users and suppliers with information regarding the hydrologic structure of the basin. The USGS completed an update to the study in 2015 entitled "*Hydrologic Model of the Modesto Region, California, 1960-2004*". The update includes a three-dimensional groundwater model for the Modesto Groundwater Subbasin called the MERSTAN model. The MERSTAN model will be an important platform for analyzing future water management actions within the Modesto Sub-basin.

The STRGBA will also likely take a lead role in complying with the Sustainable Groundwater Management Act. MID currently attends monthly meetings for the STRGBA.

The depth to groundwater in the District ranges from approximately ten feet on the west side of the District near the San Joaquin River to over 100 feet east of the City of Modesto. The hydraulic gradient of the unconfined groundwater is generally southwesterly from the mountains toward the valley parallel to the slope of the river channels. In areas influenced by the rivers, by urban pumping centers or by agricultural pumping, the direction of the local groundwater flow gradient is altered significantly.

Long term water-level data in selected wells representing the unconfined to semiconfined aquifer east of Modesto, adjacent to Modesto, and west of Modesto suggest that water levels generally decreased in the eastern and central Modesto area until the early 1990s. A series of wet years, as well as the completion of the MRWTP in 1994, resulted in recent recovery of water levels under the City of Modesto. By contrast, water levels in the unconfined aquifer in the northwestern part of the study area have remained relatively constant during this same period.

Deep percolation of applied surface water to agricultural areas comprises the major source of groundwater recharge for the groundwater basins. Other significant sources of recharge include stream-aquifer interactions and precipitation. Table 34 summarizes information on the size and capacity of the Modesto Groundwater Sub-basin; Table 35 lists the firm responsible for preparation of the District’s most recent Groundwater Management Plan. The executive summary of this plan is presented as **Appendix F**.

Table 34 - Groundwater Basins

<b>Basin Name</b>	<b>Size (Sq. Mi.)</b>	<b>Estimated Capacity (AF)<sup>1</sup></b>	<b>Safe Yield (AFY)</b>
Modesto Groundwater Sub-basin	385	6,500,000	Unknown

DWR San Joaquin District Modesto Groundwater Basin Information:

[http://www.water.ca.gov/pubs/groundwater/bulletin\\_118/basindescriptions/5-22.02.pdf](http://www.water.ca.gov/pubs/groundwater/bulletin_118/basindescriptions/5-22.02.pdf)

1 – DWR Bulletin 118 also states that 14 million AF were stored to a depth of 1,000 feet in 1961. A more recent estimate was not provided.

Table 35 - Groundwater Management Plan

<b>Prepared By:</b>	<b>GEI Consultants</b>
Year:	2005
Is Appendix Attached?	Yes

More information on MID groundwater can be found at: <http://www.mid.org/water/gw/default.html>.

### Sustainable Groundwater Management Act

In September 2014, Governor Jerry Brown signed The Sustainable Groundwater Management Act of 2014 (SGMA). The SGMA is a comprehensive three bill package that includes Assembly Bill (AB) 1739 (Dickinson), Senate Bill (SB) 1168 (Pavely) and SB 1319 (Pavely). From MID’s perspective, the SGMA sets the framework for statewide sustainable groundwater management by local agencies. The SGMA requires, among other items, the formation of Groundwater Sustainability Agencies (GSA) and the preparation of Groundwater Sustainability Plans (GSP) with a focus on long-term sustainability in the sub-basin.

MID has been actively engaged in sustainable groundwater management within their irrigation service area for more than 20 years. MID will continue to represent the best interests of its growers through a multitude of local groundwater organizations, and the District is optimistic that through State law and



the continued cooperation of local water purveyors that MID will bring careful, deliberate and coordinated action to continued groundwater sustainability moving forward.

MID will likely comply with SGMA through a regional effort involving the six current members of the STRGBA, as well as the City of Waterford which is being added now through an MOU revision as noted previously. STRGBA is currently discussing their strategy for complying with SGMA and strategizing on forming GSAs and developing GSPs as guidelines are developed.

### Irrigation Wells

Because of the availability of high quality surface water, groundwater pumping by the District as a source of supply has generally been used only to supplement reduced availability of water from the Tuolumne River during consecutive dry years, and to serve areas where it is more difficult to deliver adequate amounts of surface water.

Groundwater pumping becomes crucial in areas adjacent to downstream laterals where flow fluctuations in canals occur most frequently. In gravity water delivery systems, flow fluctuations towards the ends of canals are common due to various factors including farm delivery mismatches, evaporation losses, water being turned on and off, and flow restrictions. In some cases, to avoid the need to divert additional surface water to minimize delivery shortages, groundwater pumping is used to balance differences between water orders and water deliveries. By using the pumps to minimize these fluctuations, the overall system efficiency is improved. One of the functions of the District's Decision Support System (DSS) is to aid in determining which wells to use to most efficiently minimize mismatches between demands and the availability of water for delivery.

Beginning in the late 1940's, irrigator reliance on District surface water began to change as some field crop land was converted to permanent crops such as orchards and vineyards. Since the mid-1970's, this conversion has accelerated as additional irrigators converted from flood irrigation to low-volume irrigation technologies for convenience and to maximize crop yields. Because low-volume irrigation requires more frequent irrigations and water free from debris, some agricultural water users began converting to groundwater to supply their pressurized irrigation systems when desired rather than continuing to receive surface water on a rotation basis and having to install filtration required for operation of low-volume systems. In an effort to combat this shift and encourage agricultural water users to remain on canal water, and to support conjunctive management, the District is providing incentives and developing/implementing management strategies which include, but are not limited to:

- Rehabilitation of headworks;
- Deep well optimization to decrease response time;
- Allowing agricultural water users to install new delivery points to provide for more responsive water delivery;

- Recommending the construction of private regulating reservoirs;
- Recommending coordinated planning and construction of multi-landowner pressurized irrigation systems;
- Using full canal capacity to maximize instream storage opportunities;
- Enhancing and encouraging groundwater recharge during wet years;
- Implementing a conservation program, including providing funding for some water delivery system improvements, and
- When possible, making water available on demand or arranged demand rather than rotation.

Many of these proposed improvements are documented in the MID CWRMP, which is currently being evaluated with a Programmatic Environmental Impact Report.

The ability to use groundwater to augment surface water supplies to more efficiently deliver water through the conveyance system is one important benefit of the conjunctive water management approach implemented by the District. If groundwater levels decline to the extent that the operational flexibility afforded by conjunctive management is compromised, additional groundwater management measures will need to be exercised by the District to protect the sustainability of groundwater. Without these measures, increases in private pumping could have far-reaching effects on the area's water supply reliability.

The volume of groundwater pumped by MID and the City of Modesto in 2012 is shown in Table 36. Although not shown in the table, total agency groundwater pumping decreased by almost 20% from 2009 to 2012. Although privately-owned wells are also pumped within the District irrigation service area, the District doesn't have a reliable estimate of the volume of private pumping. As MID progresses with implementation of SGMA, and use of the MERSTAN model to better understand groundwater conditions, they will seek methods to estimate private groundwater pumping. One potential opportunity could be provided in the coming years as SmartMeters come on line which will enable MID to track energy usage by privately-owned wells, and to possibly apply this usage as a factor in estimating pumpage based on pump test results.

Table 36 - Groundwater Supplies for 2012

<b>Groundwater Basin</b>	<b>2012 Total (AF)</b>
MID Direct Pumping <sup>1</sup>	17,300
City of Modesto Pumping <sup>2</sup>	28,500
City of Waterford	200
<b>Total</b>	<b>46,000</b>

1 - MID pumping includes deep well irrigation pumping as well as drainage pumping on the western part of the District

2 - City of Modesto M&I pumping based on city records

### 3. Other Water Supplies

During the 2012 irrigation season, approximately 10,200 acre-feet of operational outflow from OID entered MID canals. The OID operational outflows entering the MID system aren't scheduled; therefore, MID can't always fully utilize this inflow. However, MID is currently planning system improvements which would allow it to make use of this source in the future, including the Main Canal Reservoir Project.

### 4. Drainage from the Water Supplier's Surface Area

#### Drainage Wells

Drainage wells have been employed by the District to control shallow groundwater in the western part of the District since 1918. Drainage wells are relatively shallow (usually less than 100 feet deep) and are perforated throughout their depth. They are generally pumped during the irrigation season to maintain groundwater levels below the crop root zone, which helps control root zone salinity and allows for healthy root development and growth.

Where sufficient downstream demand exists, drainage wells are used as irrigation water supply wells to supplement surface water. In these areas, the groundwater levels are below the root zone and are not damaging to the crops. Although drainage well water is generally of poorer quality than surface water, it is suitable for agriculture. As Table 37 summarizes, there are no flows to saline sinks and flows to a perched water table are minimal.

Table 37 - Drainage Discharge for 2012

Surface/Subsurface Drainage Path	AF
Flows to saline sink	N/A
Flows to perched water table	Minimal

### **B. Water Supply Quality**

MID's groundwater and surface water quality is generally good to excellent. Surface water diverted from the Tuolumne River originates from snowmelt in the high Sierras. The water is of excellent quality with a total dissolved solids (TDS) content of less than 40 ppm as shown in Table 38. Groundwater used for irrigation is also of relatively high quality with a TDS generally less than 500 ppm.

MID performs water quality monitoring consistent with the CVRWQCB Irrigated Land Regulatory Program (ILRP) through participation in the East San Joaquin Water Quality Coalition. MID conducts real-time water quality analyses on several operational outflows. Water quality sensors collect data for temperature, conductivity and pH which can be monitored through SCADA.

## 1. Surface Water Supply

The Tuolumne River watershed covers approximately 1,880 square miles of the western slopes of the central Sierra Nevada Mountains including portions of the Yosemite National Park. Snowmelt from the central Sierra Nevada is of excellent quality. For example, surface water diverted from the Tuolumne River at La Grange has a TDS of approximately 36 milligrams per liter (mg/l). Other water quality constituents that impact agricultural and domestic water use are also very low or negligible. The quality of the river water is fairly consistent from year to year. As runoff from agricultural and developed land is introduced into the lower part of the river, the overall water quality degrades some, but the overall quality remains good.

Table 38 - Modesto Reservoir Average Water Supply Quality - 2012

Parameter	Units	2012
Al	mg/l	0.4
As	µg/l	ND
Ba	mg/l	ND
Ca	mg/l	3.0
Cu	µg/l	5.5
Fe	mg/l	0.3
Mg	mg/l	1.3
Se	µg/l	ND
Na	mg/l	1.5
TDS	mg/l	24

## 2. Groundwater Supply

Groundwater quality in the District ranges from mostly good in the unconfined aquifer to poor in some areas of the confined aquifer. Total TDS in groundwater in the eastern two-thirds of the District is generally less than 500 mg/L with a range from 90 mg/L to 700 mg/L. High TDS (2,000 mg/L) groundwater is present beneath the District at a depth from about 400 feet in the west to about 800 feet in the east. This degraded water originates in marine sediments underlying the San Joaquin Valley and is not used for irrigation. The shallowest high TDS groundwater (TDS greater than 1,000 mg/L) occurs around 120 feet below land surface within a 5 to 6-mile-wide zone parallel to the San Joaquin River.

## 3. Other Water Supplies

Other water supplies include operational outflows from Oakdale Irrigation District into Modesto Irrigation District. In the past these have been estimated to average 17,000 AF/year. In recent years operations within OID have improved and the spills were estimated to be 10,200 AF in 2012.

#### 4. Drainage from the Water Supplier’s Surface Area

Subsurface drainage for lands served by MID is controlled with drainage wells. Subsurface drainage control is required in the western portions of the District where high water tables are typical. Therefore, there is currently no need for on-farm subsurface drainage systems, because the shallow groundwater is generally of good quality (less than 500 ppm of dissolved solids) and is suitable for most irrigation purposes. During the irrigation season, some drainage well water is used to supplement the District's irrigation water supply. The use of drainage wells to supplement surface water serves as a source of supply during consecutive dry years and improves the overall efficiency of the water delivery system by making water available where and when it is needed.

On-farm tailwater drainage within the District's service area is minimal due to the prevalence of low-volume and level-basin irrigation systems. In cases where on-farm tailwater is generated, the water users typically contain it within their property, especially at dairies. In some instances, surface drainage water is recycled by downstream water users. As presented in Table 40, the quality of water which enters the MID system from Modesto Reservoir is high. As a result, water quality throughout the system remains very good and doesn't limit the reuse of drainage water as shown in Table 39.

Table 39 - Drainage Reuse Effects

Analyte	Drainage Reuse Limitations				
	Increased Leaching	Blending Supplies	Restricted Area of Use	Restricted Crops	Other
TDS	No limitation	No limitation	No limitation	No limitation	NA

### C. Water Quality Monitoring Practices

#### 1. Source Water

MID monitors the quality of water diverted from the Modesto Reservoir and pumped from groundwater in compliance with several water quality monitoring programs. Table 40 provides general information on monitoring of source water quality in the District.

Table 40 - Water Quality Monitoring Practices

Water Source	Monitoring Location	Monitoring Practice	Frequency of Analysis
Surface water	Various canal locations	Agricultural Suitability, state-wide aquatic herbicide general permit	Periodically and in compliance with permit requirements
Surface water	Real-time monitoring locations on Lateral 3, Lateral 4, and Lateral 6	Agricultural Suitability	Continuous
Surface water	Modesto Reservoir	Domestic Water Quality Standards	Daily
Groundwater	Irrigation water wells	Agricultural Suitability	Annually

## 2. Drainage Water

As noted on Table 41, MID conducts periodic monitoring and analyses of surface drainage and groundwater.

Table 41 - Water Quality Monitoring Programs for Surface/Sub-Surface Drainage

<b>Monitoring Program</b>	<b>Analyses Performed</b>	<b>Frequency of Analysis</b>
Surface Water	Ag-Suitability Lab	Periodically
Groundwater	Ag-Suitability Lab	Annually
Surface Water	EC, Temp, pH	Continuously
Aquatic Herbicide general permit	Permit Requirements	Permit Requirements

## **Section V. Water Accounting and Water Supply Reliability**

### **A. Quantifying the Water Supplier’s Water Supplies**

#### **1. Agricultural Water Supplier Water Quantities**

Tuolumne River water diversions at La Grange Dam vary from year to year depending on the weather, the amount of runoff, and operational considerations. For purposes of the AWMP, 2012 is the reference year. Water year 2012 was classified as a dry year on the Tuolumne River watershed but was a typical water delivery year for MID in terms of surface water diversions from the Tuolumne River, probably because the previous year was very wet. The irrigation season started on March 4, 2012 (March 15 is the typical start) and ended on October 22, 2012 (typical end being October 31). Table 42 summarizes monthly diversions from the Tuolumne River to the MID water delivery system in 2012.

Table 42 - Surface and Other Water Supplies for 2012

<b>Source</b>	<b>Jan</b>	<b>Feb</b>	<b>Mar</b>	<b>Apr</b>	<b>May</b>	<b>Jun</b>	<b>Jul</b>	<b>Aug</b>	<b>Sep</b>	<b>Oct</b>	<b>Nov</b>	<b>Dec</b>	<b>Total</b>
Tuolumne River	22,040	3,940	20,930	16,360	45,250	45,610	53,460	46,960	26,960	26,660	3,130	202	311,502
Transfers & Exchanges	0	0	0	0	0	0	0	0	0	0	0	0	0
Recycled Water	0	0	0	0	0	0	0	0	0	0	0	0	0
OID operational outflow*	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	10,200
<b>Total</b>	-	-	-	-	-	-	-	-	-	-	-	-	328,502

Notes:

1 - OID operational outflow was estimated to be 10,200 acre-feet. There are no measurements of the monthly distribution of this flow.

2 - All totals in acre-feet

In addition to water diverted from the Tuolumne River, MID, the City of Modesto, and other local communities and agricultural water users all pump groundwater. MID reporting of groundwater pumping includes drainage water pumped to lower the shallow water table in the western part of the District. Most of the water pumped by MID was used to supplement surface water when the local demand was greater than the available surface water supply, a practice that eliminates ordering make-up water from a reservoir several miles away.

Table 43 summarizes the quantity of groundwater pumped by MID and the City of Modesto in 2012. The quantity of water pumped from privately-owned wells within the District boundaries isn’t included in this AWMP’s accounting of groundwater pumping because there are now no reliable estimates of the extent of private pumping.

Table 43 - Groundwater Supplies Summary for 2012

<b>Month</b>	<b>MID Total<sup>a</sup> (AF)</b>	<b>City of Modesto<sup>b</sup> (AF)</b>	<b>Total (AF)</b>
January	1,254	1,828	3,082
February	36	1,594	1,630
March	745	1,474	2,219
April	1,490	1,372	2,862
May	1,984	2,882	4,866
June	2,175	3,520	5,695
July	2,859	3,793	6,652
August	2,727	4,231	6,958
September	1,574	3,043	4,617
October	2,347	2,177	4,524
November	0	1,209	1,209
December	44	1,324	1,368
<b>Total</b>	<b>17,235</b>	<b>28,447</b>	<b>45,682</b>
	<b>City of Waterford (AF, monthly data not available)</b>		<b>185</b>
	<b>Total (AF)</b>		<b>45,867</b>

a. MID pumping includes deep well irrigation pumping and drainage pumping in the western part of the District

b. City of Modesto M&I pumping based on city records

## 2. Other Water Sources Quantities

Surface water diverted from the Tuolumne River and groundwater are the two sources of water actively managed by MID. Another, and more variable, source of water available to District lands is effective precipitation. Effective precipitation was estimated using a method developed by DWR specifically for the San Joaquin Valley and documented in a 1989 DWR publication entitled *Effective Precipitation* (MacGillivray and Jones). Table 44 includes estimated values of effective precipitation for 2012.

Lastly, operational inflows from Oakdale Irrigation District are another supply. They were estimated to be 10,200 AF in 2012.



Table 44 - Effective Precipitation Summary for 2012

<b>Month</b>	<b>2012 (AF)</b>
January	3,600
February	2,100
March	5,200
April	0
May	0
June	0
July	0
August	0
September	0
October	300
November	4,600
December	4,600
Total	20,400

**B. Quantification of Water Uses**

Table 45 shows the volume of surface water delivered to MID irrigation customers in 2012. The volume of water delivered is based on measurements to customers used as the basis for computing water charges.

Table 45 - Applied Water for 2012

	<b>Volume (AF)</b>
Delivered surface water charged to landowners	152,990

During 2012, there were estimated to be 63,313 acres irrigated by either groundwater or surface water within the District and crop evapotranspiration of applied water during that year was estimated to be 184,700 acre-feet (after effective precipitation).

Seepage losses from the canal system are based on canal loss calculations performed by the Kings River Water Conservation District on canals of similar characteristics as those at MID and preliminary canal seepage tests conducted by MID. Modesto Reservoir seepage losses are based on preliminary water seepage calculations performed at the end of each irrigation season. The 32,661 acre-feet for M&I surface water usage were based on the actual 2012 water deliveries to the City of Modesto in accordance with the treatment and delivery agreement. Table 46 summarizes the amount of on-farm surface and subsurface drainage water leaving the service area. As discussed earlier, the amount of on-farm drainage water leaving the service area is minimal.

Table 46 - Quantify Water Leaving the District for 2012

<b>Drain Water</b>	<b>2012 (AF)</b>
Surface drain water leaving district	Minimal
Subsurface drain water leaving district	Minimal
Subtotal	Minimal

There are no flows to saline sinks or perched water tables within the District as indicated in Table 47

Table 47 - Irrecoverable Water Losses for 2012

<b>Drain Water</b>	<b>2012 (AF)</b>
Flows to saline sink	None
Flows to perched water table	None
Subtotal	None

**C. Overall Water Budget**

Surface water is the volume of water diverted from the Tuolumne River to the MID water system (Table 42). The groundwater volume includes MID pumping from deep wells and drainage pumping on the western part of the District, City of Modesto pumping, City of Waterford pumping, and an estimate of private groundwater pumping. The total rainfall in Modesto for the period of January 2012 to December 2012 was 13.84 inches. Annual effective rainfall precipitation was determined using empirical equations developed for the San Joaquin Valley. The effective precipitation based on annual rainfall over 63,313 acres of irrigated land was 20,400 AF or 0.32 feet per acre. This parameter is also called Evapotranspiration of Precipitation.

An overall water balance for MID is presented in Table 48. The water balance shows all of the water supplies, demands, modes of groundwater recharge, and non-recoverable losses. The inflows and outflows to the groundwater basin are compared to the estimated change in groundwater storage from changes in groundwater levels. The two values agree relatively well, and the annual water balance is considered acceptable.

The water balance shows that MID water contributes significantly to the local groundwater recharge through deep percolation of irrigation water, canal seepage and reservoir seepage.

**Modesto Irrigation District – 2015 Agricultural Water Management Plan Update**

Table 48 Overall Water Balance for 2012

(all units in acre-feet)

Description	Irrigation Eff.	63%	Source
	Symbol	Volume	
<b>Supply</b>			
1) Surface Water - Irrigation	Qirr	278,800	Measured
2) Surface Water - M&I	Qmi	32,700	Measured
3) Groundwater Pumping - Irrigation (Agency Wells)	Gwirra	17,300	Measured
4) Groundwater Pumping - Irrigation (Private Wells)	Gwirrp	81,200	Residual
5) Groundwater Pumping - M&I (Agency Wells)	Gwmia	28,700	Measured
6) Groundwater Pumping - M&I (Private Wells)	Gwmip	0	Calculated
7) Precipitation	P	119,600	Measured
8) Spill Inflows: Oakdale Irrigation District	Si	10,200	Calculated
9) Other Supply	Os	0	
<b>Total Supply</b>		<b>568,500</b>	
<b>Demand</b>			
<i>Consumptive Use</i>			
10) Evapotranspiration - Applied Water	ETc	184,700	Calculated
11) Evapotranspiration - Effective Precipitation	ETp	20,400	Calculated
12) Evapotranspiration - M&I	ETmi	26,800	Calculated
13) Other Consumptive Use:	Od	0	
<i>Consumptive Subtotal</i>		<b>231,900</b>	
<i>Groundwater Recharge</i>			
14) Groundwater - Inflow	GWi	7,600	Calculated
15) Deep Percolation - Irrigation	PRCirr	108,500	Calculated
16) Deep Percolation - Precipitation	PRCp	23,900	Calculated
17) Deep Percolation - M&I	PRCmi	11,500	Calculated
18) Seepage - Channels (& Pipeline Leakage)	Sch	10,500	Calculated
19) Seepage - Reservoirs	Sr	24,000	Calculated
20) Urban Stormwater - Recharge	Rus	0	Calculated
21) Local Streams/Rivers - Recharge	Rst	-10,000	Calculated
22) Groundwater - Intentional Recharge	Rint	0	Measured
23) Other Recharge:	Or	0	
<i>GW Recharge Subtotal</i>		<b>176,000</b>	
<i>Nonrecoverable Losses</i>			
24) Groundwater - Outflow	GWo	1,900	Calculated
25) Evaporation - Channels	Ech	2,100	Calculated
26) Evaporation - Reservoirs & Recharge Basins	Er	8,000	Calculated
27) Precipitation - Evaporation and Runoff	Ep	75,300	Residual
28) Operational Spills	S	49,700	Measured
29) Groundwater - Export	GE	0	Measured
30) Other Losses:	OI	0	
<i>Nonrecoverable Subtotal</i>		<b>137,000</b>	
<b>Method 1</b>			
<i>Estimated Annual Change in Groundwater Storage</i>		<b>46,900</b>	
GW Recharge - #14 thru #23	176,000		Calculated
GW Pumping - #3 thru #6	(127,200)		
GW Outflow - #24 and #29	(1,900)		
<b>Method 2</b>			
<i>Calculated Annual Change in Groundwater Storage</i>		<b>29,600</b>	
Average water level change	1.90 feet/year		Measured
District size	104,000 acres		
Assumed specific yield	0.15		
<i>Water balance closes within acceptable limit</i>			

## Modesto Irrigation District – 2015 Agricultural Water Management Plan Update

Some of the parameters in the water balance aren't discussed in this AWMP, but were calculated as part of the District's water balance model. Table 49 shows each parameter in the water balance shown in Table 48, where it is found in this AWMP, or the basis for its calculation if it isn't presented herein.

Table 49 - Water Balance Parameters – Source of Information

No.	Parameter	Source
1	Surface Water - Irrigation	Table 32
2	Surface Water - M&I	Table 22, 32
3	Groundwater Pumping - Irrigation (Agency Wells)	Table 36 and 43
4	Groundwater Pumping - Irrigation (Private Wells)	No data available. Back calculated from other parameters.
5	Groundwater Pumping - M&I (Agency Wells)	Tables 36 and 43
6	Groundwater Pumping - M&I (Private Wells)	Assumed to be negligible (USGS Report 2015-5045)
7	Precipitation	Table 8 (annual precipitation x District area)
8	Spill Inflows – Oakdale ID	Data provided by Oakdale ID
9	Other Supply	Not used
10	Evapotranspiration - Applied Water	Table 23
11	Evapotranspiration - Effective Precipitation	Table 44
12	Evapotranspiration - M&I	Assumed 65% of M&I water used outdoors with 70% landscape irrigation efficiency
13	Other Consumptive Use	Not used
14	Groundwater – Inflow	Calculated using data in USGS Reports 2004-5232 and 2015-5045. USGS acknowledged this parameter is very difficult to estimate and can be a source of error.
15	Deep Percolation – Irrigation	Table 29
16	Deep Percolation – Precipitation	Assumed to be 20% of total precipitation based on data in USGS Professional Paper 1766
17	Deep Percolation - M&I	Table 29
18	Seepage - Channels (& Pipeline Leakage)	Table 29 plus 2,500 AF City of Modesto pipeline leakage (USGS Report 2004-5232)
19	Seepage – Reservoirs	Table 29
20	Urban Stormwater – Recharge	Included in 16 – Deep Percolation - Precipitation
21	Local Streams/Rivers – Recharge	Estimated from data in USGS Report 2015-5045 (Fig. 35)
22	Groundwater - Intentional Recharge	None in the area
23	Other Recharge	Not used
24	Groundwater – Outflow	Calculated using data in USGS Reports 2004-5232 and 2015-5045. USGS acknowledged this parameter is very difficult to estimate and can be a source of error.
25	Evaporation - Channels	Previously estimated by MID
26	Evaporation - Reservoirs & Recharge Basins	Previously estimated by MID
27	Precipitation - Evaporation and Runoff	Calculated as Precip – Effective Precip – Deep Percolation of Precip
28	Operational Spills	Measured annually by MID
29	Groundwater - Export	Table 30
30	Other Losses	Not used
	Irrigation Efficiency	USGS Report 2004-5232
	Change in Groundwater Storage	Estimated with change in average groundwater level in MID

#### **D. Water Supplies and Demands for 2013, 2014 and 2015**

The Governors Executive Order B-29-15, dated April 1, 2015, required reporting of water supplies and demands for 2013, 2014 and 2015, to the extent that the data is available, in AWMPs. Below is a synopsis of available data.

Surface water supplies in 2013 were slightly above average at 316,571 AF. As a result, MID groundwater pumping was slightly lower than average at 14,170 AF. Agricultural water demands were near average with about 57,500 acres using MID water.

Water supplies in 2014 included lower than average surface water supplies (176,100 AF) and higher than average groundwater pumping (46,850 AF from production wells and 11,350 from drainage wells for a total of 58,200 AF). Preliminary data show that demands for MID delivered water for 2014 were about 8-10% higher in 2014 than in 2013 and 2012, since a greater number of landowners requested water from MID due to a multiyear drought. In 2014, 62,400 acres requested MID water, about a 5,000 acre increase over 2013. This was accomplished largely with the District's new Farmer-to-Farmer transfer program, which proved to be a successful Drought Management program.

Water supplies in 2015 included lower than average surface water supplies (140,020 AF) and higher than average groundwater pumping (44,260 AF from production wells and 8,482 AF from drainage wells for a total of 52,742 AF) due to the drought. In 2015, 63,464 acres requested MID water.

#### **E. Water Supply Reliability**

The average calculated median annual unimpaired runoff from the Tuolumne River basin at La Grange is approximately 1,900,000 acre-feet (1901 – 2014 records). However, the annual runoff is highly variable with no predictable year-to-year correlation. Historic annual runoff values have ranged from 382,600 acre-feet in 1977 to 4,632,000 acre-feet in 1983. Therefore, water storage facilities and conjunctive management practices that carry over water from years of abundance to dry years are critical for the well-being of the communities who depend on the river. The importance of water storage and conjunctive management became particularly apparent during the prolonged drought of 1987-1992 and the current on-going drought.

Excluding consecutive dry years, sufficient natural precipitation and watershed runoff occurs to satisfy the local agricultural and domestic needs as well as those of the CCSF. During consecutive dry years, the Districts rely on carryover storage and irrigation wells to supplement river water diversions. However, in recent years new demands on MID's water supplies, such as additional fish flows and domestic water needs, are creating greater uncertainty. As a result, MID is continuously developing new technologies and adopting conservation techniques to manage its water supply. For example, MID has expanded its Supervisory Control and Data Acquisition (SCADA) system to better monitor and manage the water flows in the water distribution system, and has implemented a Well Field Optimization Decision Support System (DSS) to increase the efficiency of groundwater use. MID also works with agricultural water users to improve on-farm water application to both increase crop productivity and to improve on-farm water use efficiency.

The MID Comprehensive Water Resources Management Plan (CWRMP) will be an important guidance document in helping to improve water supply reliability by reviewing previous planning efforts and performing additional analysis through a comprehensive water management approach. More information on the CWRMP can be found in Section I.A.

## **F. Future Water Supply**

MID derives all of its surface water from diversions from the Tuolumne River; therefore, future changes in the MID water supply will be driven by changes in hydrology and particularly by the volume, nature and timing of precipitation in the Tuolumne River watershed. Other unknown but potential impacts on the District water supply include the on-going FERC relicensing process and the Bay-Delta restoration process. The discussion presented in Section VI of this AWMP describes how climate change may affect the hydrology of the Tuolumne River watershed.

Future surface water supplies are also threatened by loss of Tuolumne River diversions to assist river fisheries and Delta water quality. The Substitute Environmental Document is proposing to reduce MID diversions by up to one-third (100,000 AF out of long term average 300,000 AF). This would substantially alter water use in the area resulting in severe water shortage for municipal and agricultural water users in addition to potentially significant impacts to continued groundwater sustainability within the Modesto Sub-basin.

The secondary source of water supply for the District is groundwater. Although not immediately affected by changes in surface water hydrology, local groundwater is a derivative of surface water hydrology in that groundwater recharge is driven by percolation of applied irrigation water, municipal water, and precipitation. Conversion of irrigation methods from surface irrigation to more efficient low-volume micro-irrigation systems will have a negative impact on the effective amount of groundwater recharge since the majority of groundwater recharge is obtained currently through on-farm irrigation. While MID has no way to control the volume of water flowing into New Don Pedro Reservoir, the District's conjunctive management program provides mechanisms for generating deep percolation needed to maintain sustainable groundwater levels within MID's irrigation service area. Therefore, while changes in watershed hydrology may reduce the reliability of surface water from the Tuolumne River watershed in ways the District can't control, the District is committed to adapting its water management practices, particularly its exercise of conjunctive management, to respond to these changes as best it can so long as adequate surface water supplies exist.

MID has actively managed their groundwater for many years and cooperates in regional groundwater management with the STRGBA. The STRGBA is likely to take a lead role in complying with the Sustainable Groundwater Management Act. While the region must satisfy numerous requirements for SGMA, the Department of Water Resources has presently determined that the Modesto Groundwater Sub-basin is not critically overdrafted (see Figure 6 below).

**Critically Overdrafted Groundwater Basins – August 6, 2015 Draft — North Central and South Central Regions**



Figure 6 - Critically Overdrafted Groundwater Basins

## ***Section VI. Analysis of Effect of Climate Change***

### **A. Effects of Climate Change on Water Supply**

The future availability of the MID water supply will be driven by changes in hydrology and particularly by the volume, nature and timing of precipitation in the Tuolumne River watershed. In addition to direct impacts on surface water supplies, climate change may indirectly affect groundwater resources. This section describes analyses of how climate change may affect the hydrology of the Tuolumne River watershed.

A study of the possible effects of climate change on the Tuolumne River watershed was conducted by the San Francisco Public Utility Commission (SFPUC) as part of an assessment of the Hetch Hetchy Reservoir and other reservoirs in the basin. As part of this study, a literature review of recent assessments of climate change was conducted to identify the current status of available information and to determine potential impacts of climate change on SFPUC water resources in the watershed. Based on the review, climate change could result in the following types of water resources impacts:

- Reduction in the average annual snowpack due to a rise in the snowline and thinner snowpack in low- and medium-elevation zones
- Changes in the timing, intensity, location, amount, and variability of precipitation, including a shift in snowmelt runoff to earlier in the year and an increased amount of precipitation falling as rain instead of as snow
- Long-term changes in watershed vegetation and increased incidence of wildfires that could affect water quality
- Increased water temperatures with accompanying adverse effects on some fisheries
- Increase in evaporation and concomitant increased demand by water users

The implications of climate change noted by the SFPUC researchers are similar to those identified by other researchers modeling water resource impacts in the Sierra Nevada due to warming trends associated with climate change and are believed to be representative of the types of impacts that may affect MID operations.

Following their qualitative assessment of the potential impacts of climate change, SFPUC staff performed an initial evaluation of the effect on the regional water system of a 1.5 degree Celsius (°C) temperature rise between 2000 and 2025 (SFPUC, 2006a). The temperature rise of 1.5°C is based on a consensus among many climatologists that current global climate modeling suggests a 3°C rise will occur between 2000 and 2050 and a rise of 6°C will occur by 2100. The evaluation predicts that an increase in temperature of 1.5°C will raise the snowline approximately 500 feet every 25 years. Therefore, the SFPUC evaluation indicates that a rise in temperature of 1.5°C between 2000 and 2025 will result in less or no snowpack below 6,500 feet and faster melting of the



snowpack above 6,500 feet. Similarly, the snowline will have risen to 7,000 feet in 2050 and to 8,000 feet in 2100. The snow-free portion of the basin will rise from 13 percent in 2000 to 57 percent by 2100. This shift in snowline implies that more of the basin will receive rain during a storm and less will receive snow. This change will produce a shift in runoff timing: more runoff during the early winter and less snowmelt at the end of the winter.

The SFPUC evaluated the shift in the timing of runoff with their current runoff forecasting model. By raising daily maximum and minimum temperatures to simulate climate change, the results indicated that about 7 percent of the runoff currently draining into Hetch Hetchy Reservoir will shift from the spring and summer to the fall and winter by 2025. This percentage is within the current interannual variation in runoff and is within the range accounted for during normal runoff forecasting and existing reservoir management practices. As the warming process continues and if even larger shifts occur, reservoir operational strategy will have to be changed in response.

The findings of SFPUC staff for the Hetch Hetchy Watershed provide a useful indication of the nature and extent of the impacts of climate change on inflow to New Don Pedro Reservoir. Trends observed in the SFPUC report are supported by observations presented in the DWR study *Progress on Incorporating Climate Change into Management of California's Water Resources*. Based on analysis of flows of four rivers in the San Joaquin River watershed (Stanislaus, Tuolumne, Merced, and San Joaquin), the report notes April through July runoff has declined by approximately 7 percent relative to total water year runoff over the past 100 years. Therefore, while total runoff in these watersheds has decreased, April through July runoff has decreased at a greater rate. The DWR paper states that, "It is reasonable to conclude that this trend (toward reduced runoff) is the likely result of climate change and warming and an attendant decline in Sierra snowpack. A portion of the trend may also be attributable to progressively earlier melting of Sierra snowpack due to warming."

The watershed of New Don Pedro Reservoir includes lands lying at lower elevations than the watershed of Hetch Hetchy Reservoir. Therefore, while both studies predict a substantial reduction in percentage of their respective watersheds covered in snow, the DWR report predicts that a 5°C rise in temperature will result in 35 percent reduction in the New Don Pedro Reservoir watershed area being covered by snow, while the SFPUC report predicts that a 6°C rise in temperature will reduce the percentage of the Hetch Hetchy Reservoir watershed to be covered by snow to 43 percent.

## **B. Effects of Climate Change on Agriculture's Water Demand**

Climate change is expected to increase temperatures in the Central Valley resulting in changes to growing season and higher daytime and nighttime temperatures. The general increase in temperatures coupled with greater variability in precipitation in the valley is expected to lead to increases in evapotranspiration resulting from warmer seasons; thereby creating a general increase in agricultural water demand for irrigation water and an increase in the year-to-year variability of demand.

The effects of increased temperatures are expected to be particularly pronounced on fruit crops such as apples, cherries and pears, due, in part, to the reduction of winter chill hours likely to result from

warmer temperatures. By the end of the century, the safe winter chill needed for these orchard crops is predicted to disappear. Today, the number of hours of winter chill in the San Joaquin Valley has sunk from about 1,500 a few decades ago, to approximately 1,000 to 1,200 hours. Some farmers are beginning to overcome this change by planting trees closer together and using new varieties.

Studies are now underway to breed varieties of fruit trees which can withstand the decreased winter chill hours. However, replanting orchards to varieties of these crops better suited to warming temperatures may not be feasible for many irrigators.

### **C. MID Response to Effects of Climate Change**

While changes in watershed hydrology and in temperature-driven crop water demand may result from climate change, there is little consensus about the rate at which climate change will occur or the magnitude of the impacts. Given the general agreement that climate change is taking place and the general uncertainty regarding the rate of change, the District is committed to monitoring key indicators of climate change that affect the hydrology of the Tuolumne River watershed and growing conditions in the District's irrigation service area and to adapting its water management practices to respond to changes as they become evident.

In addition to adaptive management, implementation of the water conservation initiatives now underway at MID is intended to help the District and its agricultural water users prepare for the potential impacts of climate change by improving operational control within the District. Improving operational control will enable the District to exercise adaptive management measures should they become necessary.

## **Section VII. Water Use Efficiency Information**

### **A. EWMP Implementation and Reporting**

Table 50 summarizes the status of implementation of EWMPs at MID. As the table indicates, each of the EWMPs required by SBx7-7 and listed in the DWR publication *A Guidebook to Assist Agricultural Water Suppliers to Prepare a 2015 Agricultural Water Management Plan* is now being implemented.

The District has chosen to implement some EWMPs that, when viewed in isolation, aren't locally cost effective water conservation measures. These measures are being implemented because MID's goal is to provide the flexibility and reliability of water service necessary to maintain the District's system as the water source of choice by all irrigators within the District's irrigation service area. Maintaining irrigators' preference to receive water from gravity deliveries is fundamental to MID's ability to manage water conjunctively, to conserve energy, and to maintain the District's financial viability. Therefore, when viewed as an overall strategy for serving its agricultural water users, the benefits of implementing the full program of EWMPs are clear.

MID's integrated program for implementation of EWMPs is apparent in the District's *Comprehensive Water Resources Management Plan*. This plan includes a comprehensive program of new and rehabilitated facilities and improved control systems to improve the efficiency and effectiveness of water management throughout the District.

From 2012 to 2014 MID completed several projects that enhance water measurement, automation of facilities, reduce operational spills, and line canals. These projects contribute to the EWMPs and are discussed in Section 1.D – AWMP Implementation.

Table 50 - Report of EWMPs

Water Code Reference	EWMP	Current Status	Notes
10608.48.b(1)	Measure the volume of water delivered to customers with sufficient accuracy to comply with subdivision (a) of Section 531.10 and to implement paragraph (2) of the legislation.	Proceeding with implementation	<p>MID currently measures, monitors, and controls flows throughout its water delivery system. The District also measures deliveries in order to bill agricultural water users accurately under the District's tiered water pricing structure. As agricultural water users convert their on-farm systems from flood to low volume irrigation systems, cumulative water measuring devices such as meters are being installed. MID is financially supporting the upgrading of water users' water delivery facilities devices by contributing up to 50% of the installation cost of water measuring devices including water meters.</p> <p>MID has compiled an inventory, survey and classification of its turnouts and is concurrently pilot testing various flow measurement devices at representative sites within the District. These efforts will allow it to establish a comprehensive, planned and economical corrective action plan to bring non-compliant turnouts into compliance. The District is committed to comply with the requirements of SBx7-7 by verifying the accuracy of seasonal measurement of irrigation water deliveries using the methodology described in Section VIII of this AWMP.</p>
10608.48.b(2)	Adopt a pricing structure for water customers based at least in part on quantity delivered	Currently adopted (as of 2015)	MID has adopted a pricing structure based at least in part on volume used. As it implements this, the District will pay careful attention to the implications of volumetric pricing on water use efficiency, irrigation service, conjunctive management and other aspects of the District's mission to ensure that water pricing strategies serve their intended purpose.
10608.48.c(1)	Facilitate alternative land use for lands with exceptionally high water duties or whose irrigation contributes to significant problems, including drainage	Currently Implemented	MID facilitates and considers requests for alternative land uses, including assistance with drainage problems. On-farm tailwater drainage within the District's service area is minimal due to the prevalence of low-volume and level-basin irrigation systems, however it is common on dairies. In cases where on-farm tailwater is generated, the water users typically contain it within their property. In some instances, surface drainage water is recycled by downstream water users.
10608.48.c(2)	Facilitate use of available recycled water that otherwise would not be used beneficially, meets all health and safety criteria, and does not harm crops or soils	Currently Implemented	MID facilitates and considers requests for use of recycled water. Currently, one MID water user has a contract with the community of Salida to use reclaimed water on their property.
10608.48.c(3)	Facilitate financing of capital improvements for on-farm irrigation systems	Currently Implemented	For many years, MID has financially assisted its water users and has contributed up to 50% of the cost of projects to replace private ditches and pipelines. The District has also provided low interest loans for the other 50% of the projects' costs. When state grants are available, MID has contributed up to 67% of the projects' cost. MID has recently developed a detailed formal application process for funding future on-farm improvements. The program provides up to 50% funding for physical improvements and management practices. <b>Appendix G</b> includes details on applicant eligibility, eligible projects, available funding, the application process, payment procedures, project ranking, contractual obligations, and suggested design requirements.
10608.48.c(4)	Implement an incentive pricing structure that promotes one or more of the following goals: (A) more efficient water use at the farm level; (B) conjunctive use of groundwater; (C) appropriate increase of groundwater recharge; (D) reduction in problem drainage; (E) improve management of environmental resources; (F) effective management of all water sources throughout the year by adjusting seasonal pricing structures based on current conditions.	Currently Implemented	MID has adopted a pricing structure based at least in part on volume used. Therefore, growers have incentives to conserve water. In addition, over the last few years, the water pricing structure has increased the cost of water at a rate of about 10% per year, but increased 40% in 2015. Furthermore, 2014 and 2015 included a special Drought Surcharge of \$11.91 and \$16/irrigated acre respectively, to cover drought related operations, such as increased groundwater pumping and enforcement of Rules and Regulations.

Table 50 - Report of EWMPs

Water Code Reference	EWMP	Current Status	Notes
10608.48.c(5)	Expand line or pipe distribution system, and construct regulatory reservoirs to increase distribution system flexibility and capacity, decrease maintenance and reduce seepage.	Currently Implemented and ongoing	<p>MID has concrete lined approximately 91% of its canals. The remaining 9% lie in soils with low permeability and in areas where groundwater recharge is beneficial. The B/C ratio for this EWMP is low due to the small amount of water that can be conserved by lining 20 miles of canal when compared with the estimated cost of \$2,500,000. The District accepted this EWMP because, in addition to water conservation, there could be reasons such as improving water supply reliability by reducing the threat of canal bank failures that could decrease the potential for liability.</p> <p>The District is currently designing a new regulating reservoir east of Modesto which is one of the first major system improvements that will provide significant distribution system flexibility. The District is also investigating several potential recapture reservoirs at the end of the irrigation system to help reduce operational spills.</p> <p>In the 2016 budget, MID has budgeted approximately \$200,000 for a non-evasive pipeline lining pilot program. The results of this pilot program will be discussed in the 2020 AWMP.</p>
10608.48.c(6)	Increase flexibility in water ordering by, and delivery to, water customers within operational limits.	Currently Implemented	<p>MID strives to add flexibility to water ordering and delivery. Most water orders and deliveries are based on an arranged demand system where the frequency and duration is flexible. The rate of flow is flexible to the extent that capacity of the delivery system allows. As water users convert from flood to low volume irrigation systems, the District's ability to provide greater water delivery flexibility increases. In addition, MID policy allows water transfers between water users within the boundaries of the District. The policy allows water users to transfer water to parcels owned or rented by the water user.</p> <p>Implementation of the EWMP has been supported by District programs that have replaced some of its own pipelines and contributed to funding for the replacement of private pipelines. These projects were financed by the District to improve service and are timely elements of the District program to improve flexibility and reliability of deliveries as the District replaces its old cast-in-place pipelines. The District is attempting to minimize the number of water users who leave surface water in favor of groundwater for 100% of their irrigation water needs.</p> <p>To increase flexibility the District has also allowed some agricultural water users to construct their own turnout to better serve new pressurized irrigation systems. MID is also planning to construct the 265 AF Main Canal Reservoir and is studying numerous smaller reservoirs at the lower end of the system to help improve operational flexibility.</p>
10608.48.c(7)	Construct and operate supplier operational outflows and tailwater recovery systems	Currently Implemented (grower tailwater recovery)	<p>Currently MID has no District-operated recovery system and tailwater returns to the District system are minimal. Some agricultural water users, especially at dairies, re-circulate their water on site. An operational outflow recovery system may assist in recovering district spills that flow to local rivers and streams and are irrecoverable for use within MID's irrigation service area.</p>

Table 50 - Report of EWMPs

Water Code Reference	EWMP	Current Status	Notes
10608.48.c(8)	Increase planned conjunctive use of surface water and groundwater within the supplier service area	Currently Implemented	<p>Conjunctive use of water has been practiced by the District for many years. The District uses groundwater supplies to supplement the water supply during dry years and as needed to minimize operational outflows by using wells to supply nearby water user needs rather than diverting water from several miles away. The District's water treatment and supply agreement with the City of Modesto specifies that in dry years the District may have access to the City's wells to supplement irrigation water in exchange for river water being diverted for domestic purposes.</p> <p>In dry years, the District can pump up to 45,000 acre-feet of groundwater to supplement river diversions. MID also delivers up to 35,000 acre-feet of surface water annually to the City of Modesto in-lieu of using city pumps. A large number of surface water users have also installed private groundwater pumps which can be used for irrigation during dry years.</p> <p>Prior to 1995, the City of Modesto relied solely on groundwater to meet its municipal and industrial needs. MID now delivers up to 36,600 acre-feet of treated Tuolumne River water per year to the City of Modesto for M&amp;I uses. With completion of Phase II by the end of 2015, MID will have the capacity to deliver up to 70,000 AF of treated surface water to the City of Modesto or nearly 85% of the City of Modesto's estimated 2015 demand as published in the City of Modesto/MID 2010 Joint Urban Water Management Plan.</p> <p>MID is working jointly with the Stanislaus and Tuolumne Rivers Groundwater Basin Association to comply with the Sustainable Groundwater Management act, which will require that groundwater be managed sustainably with no net long-term overdraft. MID will accomplish this through a combination of continued improvements to water management, spill reduction, municipal water deliveries described above, potentially groundwater recharge basins and continued conjunctive use.</p>
10608.48.c(9)	Automate canal control structures	Currently Implemented	<p>MID has automated approximately 45 monitoring and flow control stations at Modesto Reservoir and water diversion points and installed monitoring stations along some reaches of its canals. The District has identified another 30 locations that could be automated for greater water management flexibility. The District has added, and will continue to add, canal automation to its in-house SCADA system in order to enhance water delivery flexibility to water users. The District has also installed controls to automate some irrigation water wells. With this automation, the wells can be turned on and off remotely based on demand within the canal.</p> <p>As with other district initiatives, MID has proceeded with implementation of this EWMP as a vehicle to improve customer service by increasing the flexibility of deliveries to support the increasing number of conversions from annual to permanent crops and from flood to low volume irrigation systems.</p>
10608.48.c(10)	Facilitate or promote customer pump testing and evaluation	Currently Implemented	<p>Upon request by the customer, MID tests private water supply pumps. MID has installed water flow meters on approximately 70% of its pumps and has developed a well field Decision Support System to efficiently operate the pumps.</p>
10608.48.c(11)	Designate a water conservation coordinator who will develop and implement the water management plan and prepare progress reports	Currently Implemented	<p>Through a Board Resolution, the MID Board of Directors has appointed John B. Davids, the current Irrigation Operations Manager, as the Water Conservation Coordinator for Modesto Irrigation District.</p>
10608.48.c(12)	Provide for the availability of water management services to water users.	Currently Implemented	<p>MID financially supports the following: 1) CIMIS telephone and website water use information; 2) water flow and measurement information; 3) publishes a periodic newsletter; 4) dissemination of co-op extension and other data; 5) water well pump testing; 6) supports local agricultural education programs at both the college and high school level.</p>

Table 50 - Report of EWMPs

Water Code Reference	EWMP	Current Status	Notes
10608.48.c(13)	Evaluate the policies of agencies that provide the supplier with water to identify the potential for institutional changes to allow more flexible water deliveries and storage.	Currently Implemented	MID owns pre- and post-1914 water rights on the Tuolumne River. The MID Board of Directors has the legal authority to directly set and implement policies that affect the distribution of water. Given MID's total reliance on water to which the District holds the rights (including local groundwater), there is no need to identify policies of agencies or other institutional changes with agencies that will result in increased water supply flexibility for MID.
10608.48.c(14)	Evaluate and improve the efficiencies of the supplier's pumps	Currently Implemented	The District's well operation Decision Support System was instituted specifically to improve the efficiency and effectiveness of the District groundwater pumping program. In addition, MID has implemented a well field optimization program for regular inspection and maintenance of pumps and wells to keep them in good working order. As part of this program, MID personnel are formally trained and educated in well/pump maintenance and operation. MID also follows the recommendations and improvements set forth in a well field optimization study.

**Evaluation of Water Use Efficiency Improvements**

The EWMPs presented in Table 50 can help to improve water use efficiency. Quantifying the improvements in water use efficiency is difficult, or in some cases impossible, due to the complexity of the MID conveyance system, varying water supply on an annual basis, limited implementation periods, and lack of certain data needed for evaluations. However, a qualitative assessment using existing data in consideration of completed and proposed projects and/or policies is a more feasible approach in quantifying the magnitude of efficiency improvements. Table 51 discusses the qualitative improvements in water use efficiency for each EWMP. Table 51 shows ‘improvements’ in water use efficiency that have occurred since MID’s 2012 AWMP, and those that are anticipated to occur in the next 5-year reporting period. The improvements are qualitatively denoted as potentially Negligible, Minor, Moderate or Significant. Some EWMPs have already made contributions to water use efficiency, but no changes or further improvements have occurred recently or are anticipated in the future. Potential projects and improvements below are all contingent on available funding.



Table 51 - Report of EWMPs Efficiency Improvements

EWMP No.	EWMP Description	Estimate of Water Use Efficiency Improvements Since Last Report (2012-2014)	Estimated Water Use Efficiency Improvements 5 to 10 Years in the Future
10608.48.b(1)	Measure the volume of water delivered to customers with sufficient accuracy to comply with subdivision (a) of Section 531.10 and to implement paragraph (2) of the legislation.	<p><b>Moderate</b></p> <p>Flowmeters are installed when an agricultural water user converts their on-farm system from flood to low-volume micro irrigation systems. Maintenance activities continue on existing meter gate turnouts. MID also uses Hach meters to verify flow rates in many areas. The current measurement system has proven adequate for volumetric billing. Refer to Section VIII for information on recent work related to SBX7-7 compliance and a successful meter pilot-testing program.</p> <p>New SCADA projects have improved flow measurements on main canals and laterals, and provide ditchtenders real-time access to flow data at multiple SCADA sites. MID also measures boundary outflow at many sites, which provides for real time system management.</p>	<p><b>Significant</b></p> <p>Substantial improvements may be made over the next 5-10 years to comply with the water measurement provisions of SBx7-7, including but not limited to the installation of calibrated flowmeters or provisions for measurement at every farm delivery point to allow for improved volumetric measurement. The District will also complete construction of boundary outflow facilities with a goal of measuring all outflow locations.</p> <p>MID has compiled an inventory, survey and classification of its delivery points and is concurrently pilot testing various flow measurement devices at representative sites within the District.</p>
10608.48.b(2)	Adopt a pricing structure for water customers based at least in part on quantity delivered	<p><b>Minor</b></p> <p>The Board of Directors establishes the tier prices in MID volumetric billing structure on an annual basis. This water pricing structure generally increased the cost of water delivered at the rate of approximately 10% per year. MID also implemented a special Drought Surcharge to cover increased costs for groundwater pumping and drought management programs as previously discussed. These were both adopted in 2015.</p>	<p><b>Significant</b></p> <p>The District established a new water pricing rate structure in 2015 that includes a fixed charge along with a volumetric component for the quantity of all water delivered. The volumetric charge is tiered and increases the more water per acre is delivered. The rate structure, including volumetric pricing will continue into the future, with the Board of Directors annually establishing the water rates. The District may continue to implement a special Drought Surcharge, as needed.</p>
10608.48.c(1)	Facilitate alternative land use for lands with exceptionally high water duties or whose irrigation contributes to significant problems, including drainage	<p><b>None</b></p> <p>The need to facilitate alternative land use in MID is minimal.</p>	<p><b>None</b></p>
10608.48.c(2)	Facilitate use of available recycled water that otherwise would not be used beneficially, meets all health and safety criteria, and does not harm crops or soils	<p><b>None</b></p> <p>Water recycling opportunities are currently limited in MID. The City of Modesto disposes of their treated wastewater outside MID's irrigation service area.</p>	<p><b>None</b></p>
10608.48.c(3)	Facilitate financing of capital improvements for on-farm irrigation systems	<p><b>Minor</b></p> <p>The District has provided cost share to fund several on-farm improvement projects. From 2013 to 2015, the financing has ranged from approximately \$100,000 to \$250,000/year.</p>	<p><b>Moderate</b></p> <p>The District has developed a new Conservation Funding Program to provide financial incentives to agricultural water users for capital improvements. See detailed guidelines in <b>Appendix G</b>. With formal guidelines, advertisement and more focus on lands that have a high water use per acre, it is expected that agricultural water users will be more interested in the program and the potential impact on efficiency will be greater.</p>
10608.48.c(4)	Implement an incentive pricing structure the promotes one or more of the following goals: (A) more efficient water use at the farm level; (B) conjunctive use of groundwater; (C) appropriate increase of groundwater recharge, (D) reduction in problem drainage; (E) improve	<p><b>Minor</b></p> <p>MID continues to implement a tiered pricing structure and annual rate increases to encourage efficient water management consistent with MID's Rules and Regulations Governing the Distribution of Irrigation Water Within the Modesto Irrigation District. The tiered pricing structure was adopted in 2015.</p>	<p><b>Moderate</b></p> <p>The rate structure established by the Board of Directors in 2015 includes a volumetric pricing component that is evaluated each year. It's expected to continue to promote efficient water use at the farm level, although there may be a negative impact on groundwater recharge. By establishing the price of water each year, the Board of Directors must be cognizant of establishing a price for surface water that encourages agricultural water users to use</p>

EWMP No.	EWMP Description	Estimate of Water Use Efficiency Improvements Since Last Report (2012-2014)	Estimated Water Use Efficiency Improvements 5 to 10 Years in the Future
	management of environmental resources; (F) effective management of all water sources throughout the year by adjusting seasonal pricing structures based on current conditions.		surface water rather than groundwater.
10608.48.c(5)	Expand line or pipe distribution system, and construct regulatory reservoirs to increase distribution system flexibility and capacity, decrease maintenance and reduce seepage.	<p><b>Minor</b></p> <p>Ninety one percent of the District’s canals are already lined. In 2013 MID lined 940 lineal feet of the Waterford Lower Main Lateral</p>	<p><b>Significant</b></p> <p>The District is currently designing a new 265± AF reservoir east of Modesto that will re-regulate the majority of water used in the District and significantly improve distribution system flexibility. The reservoir design will be completed in 2015 and environmental approvals are on-going. The project will be ready for construction in 2016 if funding is available. MID is also evaluating numerous smaller re-regulating reservoirs within the lower reaches of the system through the Comprehensive Water Resources Management Plan. MID continues to allocate within its annual budget funding for further canal lining, pipeline lining, and pipeline replacement. In the 2016 budget, MID has budgeted approximately \$200,000 for a non-evasive pipeline lining pilot program. The results of this pilot program will be discussed in the 2020 AWMP.</p>
10608.48.c(6)	Increase flexibility in water ordering by, and delivery to, water customers within operational limits.	<p><b>Minor</b></p> <p>Flexibility continues to increase as the District modernizes its conveyance system to serve growers who convert to low volume irrigation systems.</p>	<p><b>Moderate</b></p> <p>Continued conveyance facility improvements and reservoir construction will allow more flexible water delivery for conversion to more efficient on-farm irrigation systems. Implementation of several projects in the MID Comprehensive Water Resources Management Plan will further the goal of increasing operational flexibility.</p>
10608.48.c(7)	Construct and operate supplier operational outflows and tailwater recovery systems	<p><b>Minor</b></p> <p>Several SCADA and automation projects constructed from 2012-2014 will help to reduce operational outflows.</p>	<p><b>Significant</b></p> <p>MID has installed boundary outflow measurement using SCADA at numerous locations, and will continue to add outflow monitoring stations. These provide for realtime monitoring and control and can allow for better water management and spill reduction.</p>
10608.48.c(8)	Increase planned conjunctive use of surface water and groundwater within the supplier service area	<p><b>None</b></p> <p>The District has effectively practiced conjunctive use for many years through the use of both surface water and groundwater to serve irrigation customers. In addition, surface water deliveries to the City of Modesto for domestic use in lieu of groundwater pumping adds to the District’s conjunctive use portfolio. In the last 20 years, approximately 600,000 AF of treated surface water has been delivered to the City of Modesto. Absent MID’s cooperative relationship with the City of Modesto, this volume of water would have come from aquifers in the Modesto Sub-basin</p>	<p><b>Significant</b></p> <p>Prior to 1995, the City of Modesto relied solely on groundwater to meet its municipal and industrial needs. MID now delivers up to 36,600 acre-feet of treated Tuolumne River water per year to the City of Modesto for M&amp;I uses. With completion of Phase II by the end of 2015, MID will have the capacity to deliver up to 70,000 AF of treated surface water to the City of Modesto or nearly 85% of the City of Modesto’s estimated 2015 demand as published in the City of Modesto/MID 2010 Joint Urban Water Management Plan.</p>

EWMP No.	EWMP Description	Estimate of Water Use Efficiency Improvements Since Last Report (2012-2014)	Estimated Water Use Efficiency Improvements 5 to 10 Years in the Future
10608.48.c(9)	Automate canal control structures	<p><b>Significant</b></p> <p>The District completed several projects that help to automate controls (see Section I.D.) including:</p> <ul style="list-style-type: none"> <li>• Upper Main Canal Tunnel Gate Rehabilitation Project (2012)</li> <li>• Waterford Upper Main Canal Headworks (2012)</li> <li>• Modesto Reservoir Outlet Automation (2013)</li> <li>• Lateral 5 Pump Automation Project (2013)</li> <li>• MID Lower Main Canal Automation Project (2014)</li> <li>• Gate Integration of the Lower Main Lateral Drop 21 (2014)</li> </ul>	<p><b>Moderate</b></p> <p>The District has identified 30 additional locations that could be automated for greater water management flexibility, and will pursue these projects using funding approved through 2018.</p>
10608.48.c(10)	Facilitate or promote customer pump testing and evaluation	<p><b>None</b></p> <p>MID has continued to provide pump testing of private wells on request, which can lead to more efficient pumping and less energy use.</p>	<p><b>None</b></p>
10608.48.c(11)	Designate a water conservation coordinator who will develop and implement the water management plan and prepare progress reports	<p><b>Moderate</b></p> <p>A new Water Conservation Coordinator was recently appointed. Several water conservation measures were enacted; In particular, programs to address the current drought (see Section II.B.4) and changes to the MID Rules and Regulations (see Section II.B.1).</p>	<p><b>Moderate</b></p> <p>Water conservation is a key component of MID water management and will be continually pursued through a variety of programs and projects directed by the Water Conservation Coordinator.</p>
10608.48.c(12)	Provide for the availability of water management services to water users.	<p><b>None</b></p>	<p><b>Minor</b></p> <p>The new Conservation Funding Program implemented by the District (see detailed guidelines in <b>Appendix G</b>) provides financial incentives for water management practices such as scientifically based irrigation scheduling and soil moisture monitoring.</p>
10608.48.c(13)	Evaluate the policies of agencies that provide the supplier with water to identify the potential for institutional changes to allow more flexible water deliveries and storage.	<p><b>None</b></p> <p>The MID Board of Directors has the legal authority to directly set and implement policies that affect the distribution of water.</p>	<p><b>None</b></p>
10608.48.c(14)	Evaluate and improve the efficiencies of the supplier's pumps	<p><b>Minor</b></p> <p>MID has continued to use a well field operation decision support system and optimization program. Continuous improvements in pump efficiency are realized through these efforts.</p>	<p><b>Minor</b></p>

Table 52 presents the schedule for implementing EWMPs.

Table 52 - Schedule to Implement EWMPs

<b>EWMP</b>	<b>Implementation Schedule</b>	<b>Finance Plan</b>	<b>Budget Allotment<sup>1,2</sup></b>	<b>AWMC MOU Demand Measures</b>
<b>Critical</b>				
1 - Water Measurement	Implemented/On-Going	Annual Irrigation Operations Budget	\$207,900	C-1
2 - Volume-Based Pricing	Implemented/On-Going	Annual Irrigation Operations Budget	\$19,500	
<b>Conditional</b>				
1 - Alternate Land Use	Implemented/On-Going	Annual Irrigation Operations Budget	\$125,800	B-1
2 - Recycled Water Use	Implemented/On-Going	Annual Irrigation Operations Budget	\$409,400	B-2
3 - On-Farm Irrigation Capital Improvements	Implemented/On-Going	Annual Irrigation Operations Budget	\$140,500	B-3
4 – Incentive Pricing Structure	Implemented/On-Going	Annual Irrigation Operations Budget	\$27,100	C-2
5 – Infrastructure Improvements	Implemented/On-Going	Annual Irrigation Operations Budget	\$1,684,000	B-5
6 – Order/Delivery Flexibility	Implemented/On-Going	Annual Irrigation Operations Budget	\$182,800	B-6
7 – Supplier Operational Outflow and Tailwater Systems	Implemented/On-Going	Annual Irrigation Operations Budget	\$409,400	B-7
8 – Conjunctive Use	Implemented/On-Going	Annual Irrigation Operations Budget	\$398,500	B-8
9 – Automated Canal Controls	Implemented/On-Going	Annual Irrigation Operations Budget	\$424,600	B-9
10 – Customer Pump Test/Evaluation	Implemented/On-Going	Annual Irrigation Operations Budget	\$5,000	
11 – Water Conservation Coordinator	Implemented/On-Going	Annual Irrigation Operations Budget	\$54,100	A-2
12 – Water Management Services to Customers	Implemented/On-Going	Annual Irrigation Operations Budget	\$124,400	A-3
13- Identify Institutional Changes	Implemented/On-Going	Annual Irrigation Operations Budget	\$165,900	A-5
14 – Supplier Pump Improved Efficiency	Implemented/On-Going	Annual Irrigation Operations Budget	\$289,600	A-6
Grand Total all EWMPs			See Note 1	

Notes

1. Budget allotments are not necessarily applicable to a specific EWMP and may spread across multiple EWMP's. Consequently they aren't additive.
2. Amounts shown are rounded to nearest \$100 and are specific to the 2016 Budget Year.

**B. Documentation for Non-Implemented EWMPs**

MID has chosen to implement each of the recommended EWMPs. Although certain measures aren't locally cost-effective as individual water conservation measures, the District views them as elements

of a broad program that enables MID to provide a high level of service to its agricultural customers and to responsibly manage surface water and groundwater resources in the District’s irrigation service area. This position is summarized below in Table 53.

Table 53 - Non-Implemented EWMP Documentation

EWMP #	Description	(check one of both)		Justification/Documentation
		Technically Infeasible	Not Locally Cost-Effective	
NA				All EWMPs are being implemented as they support MID's long-term water management objectives

## **Section VIII. Agricultural Water Measurement Regulation Documentation**

### **A. Introduction**

MID recognizes the need for uniform standards and procedures for measuring and recording farm water deliveries in order to: (1) improve water management by equitably distributing water to each agricultural water user; (2) provide cost-effective service to all agricultural water users; (3) improve operational records for analysis and planning purposes, and (4) comply with recent regulatory requirements. MID currently measures all farm water deliveries, but the current measurement methods may not comply with regulated accuracy requirements in all circumstances. Regulations requiring a specified level of delivery point measurement accuracy were incorporated into California Code of Regulations Title 23 Division 2 Chapter 5.1 Article 2 Section 597 (23 CCR §597) in July 2012 as an outgrowth of Senate Bill X7-7 (SBx7-7), the Water Conservation Act of 2009. MID's existing farm delivery point measurement devices, referred to as meter gates, and current measurement methods have been adequate to allow MID to measure water at the farm delivery point level for many years with sufficient accuracy to bill for water use. However, recent analysis and field investigations have indicated that a more accurate measurement method could be employed at some delivery point locations to help satisfy the accuracy requirements of SBx7-7.

Briefly summarized, SBx7-7 (23 CCR §597) requires that agricultural water suppliers providing water to 25,000 irrigated acres or more measure the volume of water delivered to customers with sufficient accuracy to comply with AB 1404 and bill water customers based at least in part on the quantity of water delivered (volumetric pricing). AB 1404 (2007) amended the California Water Code to add §531.10 regarding water measurement and water delivery reporting as follows:

- Any agricultural water supplier, either public or privately owned, supplying 2,000 AF or more of surface water annually for agricultural purposes, or serving 2,000 or more acres of agricultural land, must comply with reporting requirements.
- An agricultural water supplier shall submit an annual report to DWR that summarizes aggregated farm-gate delivery data, on a monthly or bi-monthly basis, using best professional practices.
- §531.10(a) states that a water supplier is to use best professional practices in reporting annual aggregated farm-gate delivery data, while §531.10(b) states that “*nothing in this article shall be construed to require the implementation of water measurement programs or practices that are not locally cost effective*”.

The final SBx7-7 Agricultural Water Measurement regulation (Regulation) that was prepared by DWR and adopted in July 2012 requires that the volume of water delivered by an agricultural water supplier be measured at the delivery point where the agricultural water supplier transfers control of delivered water to a customer or group of customers, and be of sufficient accuracy to meet the requirements of AB 1404. In most cases, the transfer of control occurs at the farm-gate, but the regulation does allow for measurement upstream in a lateral under certain conditions. Regardless of where the measurement is made, the following numeric accuracy standards apply to the volume of delivered water:

- Existing measurement devices shall be certified to be accurate within  $\pm 12\%$  by volume.

- New or replacement measurement devices shall be certified to be accurate within  $\pm 5\%$  by volume in the laboratory if using a laboratory certified device (such as an ultrasonic meter) or  $\pm 10\%$  by volume in the field if using a device that is non-laboratory certified (such as meter gates).

If a device measures a value other than volume, for example, flow rate, velocity or water elevation, the accuracy certification must incorporate the measurements or calculations required to convert the measured value to volume, such as flow rate and elapsed time. If existing measurement devices don't meet the accuracy requirements, water suppliers must include in the AWMP a plan to take corrective action to comply with the SBx7-7 requirements.

The Regulation requires measurement at the location where the agricultural water supplier transfers control of delivered water to a customer or group of customers. In most cases, the transfer of control occurs at the individual delivery point or farm-gate, but the regulation does allow for measurement upstream in a lateral under certain conditions. If a water supplier elects to measure upstream on a lateral, the water supplier shall document in their water management plan the criteria used to apportion the volume of water delivered to individual downstream customers, and document that the method is sufficient to establish a pricing structure based at least in part on the volume delivered.

This document describes MID's proactive efforts over the course of the last reporting period to establish a comprehensive, planned and locally cost effective corrective action plan to bring non-compliant turnouts into compliance with the water measurement provisions of SBx7-7, including a schedule, budget and financing plan. Implementation will be a dynamic process that may potentially be impacted by emerging technologies, drought and various other local drivers. As a result, MID will continually assess progress and adapt the plan as necessary to ensure that compliance is achieved through practical engineering, cost analysis and efficient program management.

## **B. Existing Facilities and Measurement Practices**

MID distributes a combination of Tuolumne River water and groundwater via a network of storage facilities, canals, pipelines, pumps, drainage facilities and control structures. MID's canal system begins at La Grange Dam where Tuolumne River water released from New Don Pedro Reservoir for irrigation purposes is diverted into the MID Main Canal for conveyance to Modesto Reservoir. MID operates Modesto Reservoir as a regulation reservoir to store and release irrigation water supplies, to balance irrigation deliveries with irrigation demands, to minimize flow rate fluctuations in the District's irrigation canals and laterals, and as a buffer for hydroelectric power generation. From Modesto Reservoir, water is released into the Lower Main Canal and Waterford Lower Main for distribution through a gravity flow system to downstream agricultural water users for irrigation purposes. MID's distribution system is comprised of approximately 147 miles of concrete lined canals, 15 miles of unlined canals, 42 miles of pipelines and 39 miles of drains. The conveyance canals generally run in an easterly to westerly direction. Private ditches and pipelines used to convey water from the MID distribution facilities to a group of landowner fields are owned by "Improvement Districts", a subdivision of MID. These Improvement Districts use the technical and financial expertise of MID, while leaving the basic decision of whether or not to make any improvements in the hands of those using the community facility. There are a total of 230 Improvement Districts within MID.

MID has a total of approximately 760 existing delivery points, or turnouts, where MID transfers control of delivered water to a customer or group of customers. Most of these delivery points have existing measurement devices that are known as meter gates (also known as rated gate or calibrated gate) which operate as a submerged variable area orifice. This device can provide a good estimate of the instantaneous flow rate under the correct conditions, and the volume delivered can be determined by employing a time factor to convert the flow rate to volume of water delivered. Some of the initial testing of this type of device was conducted in the late 1920's, and was later updated by USBR in the early 1950's and more recently by the Irrigation Training and Research Center (ITRC) at Cal Poly San Luis Obispo. This type of measurement device is based on measuring the head differential between the upstream water surface and the downstream water surface as water flows through the gate. A stilling well is placed a certain distance (usually 12") behind a turnout gate that measures the water level in the pipeline downstream of the gate. Information on meter gates and recent testing conducted by ITRC is contained in **Appendix H**. The instantaneous flow rate is determined by cross referencing the known gate opening with the head differential on a standard rating table. Recent testing by ITRC indicates that the best accuracy is obtained when the gate is between 20% and 75% open. This type of measurement device requires full pipe flow downstream of the turnout and a constant head differential for the duration of the delivery in order to provide a constant flow rate. Meter gates can provide accurate flow rate measurements as long as the device is installed properly and can provide accurate volumetric quantities with proper water level measurement, flow rate consistency, and time factor conversion.

The source canal water level (upstream water surface) is maintained relatively stable by the numerous long-crested weirs MID has installed throughout its distribution system, however, MID has no control over the downstream water level. Many of the MID delivery points are very large for the flow rate currently being delivered and the gate opening may not fall within the desired range for device accuracy. Additionally, the time factor may not always be well documented since the District doesn't open and close every delivery point. Therefore, the current measurement method doesn't always comply with the accuracy requirements of SBx7-7.

MID is in the process of completing an extensive and time consuming physical inventory of every delivery point in the District, comprised of both canal and pipeline turnouts. The District is using this physical inventory and other District records to associate each irrigated parcel to a delivery point and has divided the turnouts into acreage ranges that will be used to identify the type of measurement device or method that may be used to comply with SBx7-7 as explained in Section G below. The physical review and inventory will also be used to determine the modifications that must be made to those delivery points where a measurement device will be installed. The turnout inventory is currently being reviewed and verified, but the preliminary turnout inventory indicating the number of turnouts for each acreage group is presented below:



Table 54 - Preliminary Turnout Inventory

Acreage Range	Delivery Points		Acreage Billed		Avg acres/ turnout
	Number	%	Acres	%	
< 5 ac	22	3%	65	0%	3.0
5 - 10 ac	73	10%	514	1%	7.0
10 - 50 ac	368	48%	9,817	17%	26.7
50 - 100 ac	126	17%	8,673	15%	68.8
> 100 ac	171	23%	38,784	67%	226.8
Total	760	100%	57,853	100%	76.1

As shown above, approximately 67% of the acreage is served by only 23% of the delivery points. In addition, approximately 13% of the delivery points serve less than 10 acres and only account for approximately 1% of the acreage. The amount of acreage served by each delivery point generally corresponds to the amount of water delivered, so it is reasonable that the most “bang for the buck” regarding compliance can be obtained by focusing efforts on those delivery points that serve more than 10 acres. As the delivery point inventory is finalized, it is expected that there will be some revisions to fine tune the above acreage breakdown but it is anticipated that the general trend will remain consistent.

**C. Legal Certification and Apportionment Required for Water Measurement – Lack of Legal Access to the Farm-gate**

The District has legal access to measure water at every Delivery Point, defined by MID as the location where the District transfers control of delivered water to the irrigator or a group of irrigators. Rule 5.5.2 of the District Rules and Regulations (see **Appendix C**) states that “*The District has the authority to install or require the installation and maintenance of irrigation flow measurement devices or structures at all District Delivery Points in compliance with the prevailing state law and regulations promulgated by the California Department of Water Resources or other regulatory agency as may be applicable*”.

Most of the MID irrigators receive water through Improvement District (ID) facilities, which are privately owned community facilities, usually pipelines. Most often, only one irrigator at a time draws water from the ID pipeline when typically using the flood irrigation method because the ID systems were originally designed to deliver “one-head” of water for one user at a time on a rotation basis. Since only one irrigator is typically irrigating at a time, measurement at the head of an ID facility or “one-headed” lateral is equivalent to measurement at individual delivery points on the lateral. Hence, measurement of the individual delivery points on ID facilities or one-headed laterals can’t be economically justified as allowed under California Water Code §531.10(b). MID currently measures the water flowing into the head of the pipeline (the delivery point), which is upstream of the actual turnout to the irrigator’s place of use. The flow rate into the pipeline is currently based upon a rated meter gate.

A standard head (or delivery) of water within MID is fifteen (15) cfs, as noted by Rule 2.6.1 of the Rules and Regulations (**Appendix C**) which states: “*All new Private or Improvement District*

*Facilities used for flood irrigation purposes shall provide for a minimum gravity flow of fifteen (15) cubic feet per second. A variance from this minimum flow shall be evaluated by the District on a case-by-case basis based on the impact on the operation of the District's water delivery system".*

As cropping patterns change within the District, more agricultural water users are converting their irrigation systems to pressured delivery systems, which use much smaller delivery rates than the standard head for flood irrigation. As such, the District is seeing more instances where two agricultural water users might be irrigating at the same time from an ID facility, where one agricultural water user is flood irrigating and another user is irrigating with an on-farm pressurized delivery system such as solid set sprinkler or micro-irrigation system (drip/micro-spray). If multiple agricultural water users are taking water from the ID pipeline at the same time, then the water use is apportioned to each agricultural water user by the ditchtender who takes into account that amount of water that is being delivered to the pressure system and subtracts the pressure delivery amount from the total flow, with the balance being the amount of water delivered to the flood irrigated delivery point. When an irrigator takes water from the ID pipeline into an on-farm pressurized delivery system, the volume of water delivered can be determined several ways: 1) by reading a flowmeter that was installed by the landowner on the system, or 2) by measuring the system flow rate with a portable meter operated by the District (such as a Fuji strap-on meter) and multiplying the flow rate by the time interval, or 3) by estimating the flow rate based on the design of the pumping system, and multiplying the flow rate by the time interval.

This method of apportionment has been verified by use over many years and has been found to be sufficient for allocating water use among agricultural water users and establishing the basis for how each agricultural water user is charged for the amount of water delivered during each irrigation event and the total amount over the irrigation season. The District is currently conducting a survey of the pressure system locations and preferred measurement method to assist in apportioning the water use.

A 2012 report published by ITRC regarding SBx7 Compliance (see **Appendix H – SBx7 Flow Rate Measurement Compliance for Agricultural Irrigation Districts**) contained the following as one of its conclusions:

*The wording of SBx7 appears to clearly indicate that the proper, most downstream flow measurement location would be at the head of any "community ditches". "Community ditches (sometimes called "improvement districts") are defined as privately owned distribution systems that receive water from the irrigation district. The distribution, partitioning, and scheduling of water deliveries within the "community ditch" is not done by irrigation district personnel.*

#### **D. Engineer Certification and Apportionment Required for Water Measurement – Technically Infeasible**

Not applicable – there are no turnout locations within the District that are technically infeasible to measure, although conditions at some locations make measurement challenging.

## E. Description of Water Measurement Best Professional Practices

### Description of District Operations

MID operates a decentralized water ordering and delivery system. The ditchtenders take water orders from agricultural water users and coordinate deliveries based on demand and the flow capacity of the distribution system. As MID moves away from rotation to the more flexible arranged demand water delivery system, the ditchtenders' functions have become less routine and more customer-oriented.

Agricultural water users with flood irrigated lands may continue to irrigate on a fairly constant rotation while the water users with pressurized irrigation systems may request irrigation water on a more frequent arranged demand basis. Therefore, water order lead times vary depending on the time of year, system capacity, and where water is being routed, and can vary from very short notice to up to 120 hours notice. For example, a water user close to Modesto Reservoir with land near a large canal may have a greater probability of receiving water on short notice than a user who is more distant from the reservoir and from main MID delivery facilities. The District's goal is to supply water to the irrigator when the water is needed and to maintain that delivery for the duration necessary to refill the soil profile or to satisfy the crop water requirement.

Rule 5.4.2 of the District Rules and Regulations (see **Appendix C**) states that *“Where possible, irrigation water will be provided to the Irrigator based on an arranged demand delivery, under which the delivery rate is fixed, but the frequency and duration of use are requested by the Irrigator. Where the capacity of the system is limited, rotation delivery may be used by the Ditchtender. The Ditchtender may, at the Ditchtender’s discretion, alter the rotation or cause water to be delivered upon request. Advance notice for rotation deliveries will be made with an appropriate amount of warning time to take into consideration the preparation needed to commence irrigation.”*

### Collection of Water Measurement Data

MID uses the “TruePoint” water accounting system, a database which is an established program for scheduling, tracking, monitoring and billing of agricultural water use. The database allows input of various measurement methods including meters, meter gates and other rated measurement devices. The ditchtender enters the pertinent delivery information into a laptop computer in the field. This information is downloaded at least once a week. The delivery measurements are reviewed by a highly trained irrigation supervisor, well experienced in water measurement methods and historical usage patterns.

The water delivery data are currently made available to the agricultural water user by the District by posting the water usage reports online weekly. MID will be implementing a grower portal prior to the 2016 irrigation season that will allow agricultural water users to log on and access their account to view past usage, remaining water allocation, etc, allowing each agricultural water user to monitor their water usage. The billing system uses the volumetric pricing structure adopted by the MID Board of Directors each year to determine the appropriate pricing tiers and subsequent bill amount. Starting in 2015, each bill sent to an agricultural water user takes into account the amount of water measured and delivered to the agricultural water user on a volumetric basis.

### Frequency of Measurements

The District measures the water levels at operating meter gates and measures the gate opening at least once a day, sometimes more often. A measurement is made each time a scheduled flow rate change is made. District staff will measure the head differential and gate opening generally within 1 hour of the scheduled change in flow rate.

Recent meter gate testing conducted by ITRC (see **Appendix H** – *Improving Flow Measurement Accuracy at Farm Delivery Gates in California*) concluded that an error in the delivery duration estimate of 4% (1 hour in 24 hours) coupled with conservative expected errors of upstream and downstream water level measurements would still allow meter gates to measure the volume of water within the required  $\pm 12\%$  accuracy as long as the instantaneous flow measurement uncertainty was within  $\pm 10.7\%$ .

### Method for Determining Irrigated Acres

The amount of irrigated acres is annually determined in compliance with Rule 5.3.1 of the Rules and Regulations (see **Appendix C**), which states – *No later than May 1 of each year, each Landowner or designee shall provide to the District a signed statement, on the District's form, of the kinds of crops and number of acres of each crop that will be irrigated on each parcel of land, and such other relevant information as the District may reasonably require on the same statement. After May 1 of each year, no changes to the amount of irrigated acreage or non-irrigated acreage will be allowed, but the kind of crop that is going to be planted may be changed at any time.*

The irrigated acreage is determined based upon a crop forecast report that is prepared by the agricultural water user each winter for the upcoming season. These crop reports include information provided by the agricultural water user and identify the following:

- previous year crop type and projected crop for the upcoming year
- irrigated and non-irrigated acreage for each crop from the previous year
- projected irrigated and non-irrigated acreage for each crop for the upcoming year
- irrigation methods

The agricultural water user indicates if only a portion of a parcel will be irrigated that year and accounts for non-irrigable acres such as home sites, storage yards, roads, etc. The irrigated acreage values are reviewed and verified by the District. The annual fixed charges are based on the assessed acreage, with different charges for the portion of a parcel on irrigation status versus that on facilities and maintenance status.

### Quality Control and Quality Assurance Procedures

MID staff monitors deliveries for quality assurance throughout the irrigation season by use of various control systems. These systems are both technological and based on personal experience. As noted previously, the standard delivery rate for flood irrigation is 15 cfs. Private systems installed for pressurized delivery will deliver a known flow rate that is provided to the District. The District currently utilizes two portable Hach meters to spot check and verify flow rates at delivery points throughout the District.

On meter gates a mark is painted on the gate stem to indicate the closed position, then a certain distance referred to as the “dead stem” (generally 1-inch) will be subtracted to obtain the “zero” point on the gate when water starts to trickle past the gate. This “dead stem” difference is to account for the gate movement required within the mechanism to get to the “zero” point and can vary slightly as the gate wears and more “slop” is encountered. The District will periodically have a senior ditchtender check the “slop” in a gate and make adjustments to the amount subtracted for the dead stem as necessary so an accurate gate opening is obtained, further improving the District’s QC/QA.

Water measurement data is currently posted on the District website within 7 days of measurement, allowing the District agricultural water users to track their water use. Information is available by customer ID on the website, as well as information from last year so growers can compare their water deliveries. This is one of the ultimate means of QC/QA, as the irrigators generally know how much water they are delivering and will raise any questions they have. This has especially been true during the current drought, as agricultural water users try to stretch their water allocation as far as possible. The grower portal that will be implemented prior to the 2016 irrigation season will make water use tracking even easier.

### **F. Documentation of Water Measurement Conversion to Volume**

SBx7-7 requires an annual volumetric accuracy of within 12 percent on existing devices. The main disadvantage of calculating delivered water volumes based on an instantaneous measurement is that the measurement device doesn’t directly record the volume of delivered water. This can be problematic for two reasons. First, an accurate record of the duration of the delivery elapsed time must be maintained to convert the instantaneous measurement of flow rate into a volume. Secondly, if there are fluctuations in either the upstream or downstream water surface elevation during the course of a delivery, or if the gate opening changes, these fluctuations will affect the rate of discharge, and hence, the volume of water delivered. In the case of MID, because the water level at nearly every check structure is controlled by a long-crested weir, there is little variation in canal water levels regardless of the flow in the canals, leading to very accurate upstream water level measurements. The District is able to maintain a fairly constant canal side, or upstream, water level on the meter gate, but the District has no control on the landowner side, or downstream, water level. Nonetheless, it is expected that fluctuations over the irrigation season will typically balance themselves out.

Ditchtenders calculate the volume of a delivery by measuring the differences in water elevations and the sidegate opening, using calibrated tables to compute the flow rate which corresponds to these parameters, and multiplying that flow rate by the recorded duration of delivery. The time component is manually recorded by the ditchtenders, which is an honor system and historically meant that recorded times may not always be precise. Recordation of delivery duration has improved

considerably in recent years as the drought has caused the District and agricultural water users to focus on efficient water use to stretch limited supplies.

The calculated water delivery is entered into the District's TruePoint water management system, which tracks cumulative water delivered to each agricultural water user during the irrigation season. This data is used to bill the agricultural water user on a volumetric basis in accordance with the tiered pricing structure established annually by the Board of Directors.

### **G. Device Corrective Action Plan Required for Water Measurement**

Although the District is currently able to bill for water deliveries volumetrically, the District believes that the current measurement methodology may be improved to ensure compliance with the provisions of SBx7-7 in all cases. To that end, the District conducted a Pilot Program in 2015 to test several different types of measurement devices to see which types work best for MID agricultural water users and irrigation staff. During development of the Pilot Program, MID screened dozens of available flow measurement devices and chose devices from three different manufacturers that staff felt had the greatest likelihood of meeting the unique circumstances in the District. Factors considered in the screening criteria included:

- Ease of installation and use
- Device accuracy
- Ability to accumulate volumetric delivery information
- Expected life
- Automation (SCADA) potential
- Capital cost
- Expected O&M cost

The measurement devices tested in the Pilot Program included:

- ◆ Rubicon FlumeMeter – A transit-time metering device that is attached to the canal turnout on the upstream, or canal side of the turnout. Transit-time meters send and receive sound waves and determine the difference in time, which correlates to a velocity and ultimately a flow rate. The FlumeMeter has 32 sensors across 8 planes providing 3D reconstruction of the velocity profile. The FlumeMeter is attached to the canal turnout with a frame that allows the existing canal gate to remain in place and operational. The FlumeMeter is for water measurement only, and comes complete with a pedestal mounted data logger, solar panel and battery backup. The FlumeMeter and pedestal can be moved from one location to another as long as the alternate location has a frame to receive the FlumeMeter and a pedestal mount to hold the pedestal. The device measures instantaneous flow rate and accumulates the volume of water delivered in the data logger. The FlumeMeter is primarily used on canal turnouts, and is difficult and costly to install on pipeline turnouts.
- ◆ SonTek IQ Pipe – An acoustic doppler meter that sends and receives acoustic pulses at a fixed frequency that collide with water particles, allowing for a determination of velocity. The SonTek IQ Pipe is designed for pipe flow and contains both depth and velocity sensors, meaning it can measure partial-pipe flow. The meter is strapped to the inside of a pipe or bolted down to the bottom of the pipe. Since the meter reads velocity and depth, the area for which it is placed must be known through the use of a rated section or known pipe size. Since the sensor is inside the pipe there are no right-of-way issues and the instrumentation and cabling can be near the canal.

The device measures instantaneous flow rate and accumulates the volume of water delivered in the data logger.

- ◆ Mace AgriFlo – An acoustic doppler meter similar to the SonTek IQ Pipe. The meter normally only measures velocity so it must be used in a full pipe, but a depth sensor can be added if partial pipe flow will be encountered.
- ◆ Hach portable flowmeter – A portable, handheld electromagnetic flowmeter that the District has used for spot measurements for many years. The device measures instantaneous flow rate only. Sensor is mounted on a calibrated rod to allow the District to take three measurements at different locations to develop a velocity profile of the pipe flow. Requires accurate knowledge of pipe size and an access vent in the pipe where the pipe is flowing full.
- ◆ Meter gate – Predominant measurement device currently in use in the District. As previously described, water level is measured upstream and downstream of a turnout gate through the use of stilling well downstream of gate. Knowing the head differential across the gate and the gate opening area, an instantaneous flow rate can be determined using a rating table. Volume of water delivered can be estimated if flow rate remains fairly constant and time interval is recorded.

A device that wasn't tested, but is being considered for the largest delivery points, is the Rubicon SlipMeter. The metering portion of the SlipMeter is identical to the FlumeMeter, but the SlipMeter includes a fully automated control slide gate in addition to the meter. The SlipMeter is sold as a complete unit with slide gate, meter, solar panel and battery backup. The existing turnout canal gate could be removed and the SlipMeter permanently mounted to the turnout structure to control the flow, or the SlipMeter could be installed in a frame similar to that used for the FlumeMeter where the existing turnout gate remains in place for redundant control.

Prior to the start of the 2015 irrigation season, a number of delivery point sites were reviewed to identify some representative sites for testing the selected measurement devices. A total of 8 sites were selected to test 3 devices with a total of 10 meters so there was some replication in testing the same device under different conditions. At all sites it was desirable to have an existing operational meter gate. Also at each site access would be provided for a Hach portable meter reading. The characteristics of each site that was selected to be included in the Pilot Program is shown below.

Table 55 - Pilot Program site characteristics

Turnout	Device	Name	Location	Task #	Map ID	Fields	Acres	DT	Pipeline Diameter	Dead stem (inches)	Metergate site	Hach site	Notes
L6-008	Rubicon 3'x3' Flumemeter	Coffee Davis Lat	Off Claribel E/Oakdale Rd	2.2	1	27	337	3	36"	1	No	Yes	not full pipe at gate
L6-008	Sontek IQ pipe meter	Coffee Davis Lat	Off Claribel E/Oakdale Rd	2.3	2				36"				Sontek D/S 1st box; with data recorder
L6-008	Mace FloSeries3 AgriFlo Xci	Coffee Davis Lat	Off Claribel E/Oakdale Rd	2.4	3				36"				mace near 2nd box
MLM-084	Rubicon 3'x3' Flumemeter	Potts Ditch Pvt	2nd gate west of Claus Rd	2.0	4	17	292	3	30"		Yes	Yes	Gate being replaced
MLM-074	Rubicon 3'x3' Flumemeter	Litt Ditch Pvt	4th gate east of Claus Rd	2.1	5	23	457	3	36"	1	Yes	No	Airvent D/S to be installed for Hach site
MLM-068	Sontek IQ pipe meter	Pvt	shared headwall & ctrl. sys.	2.6	6	3	306	3	30"	1	Yes	Yes	with dual data recorder
MLM-066	Sontek IQ pipe meter	Neagle ID	U/S Rice Rd	2.5	7	39	447	3	36"	1	Yes	Yes	with dual data recorder
L2-030	Mace FloSeries3 AgriFlo Xci	Cupp ID	U/S Church	2.8	8	2	24	3	30"	0.5	Yes	Yes	straight - clean water
L2-028	Sontek IQ pipe meter	Huff ID	D/S Root	2.7	9	4	66	3	36"	1.5	Yes	Yes	straight - clean water
MLM-110	Mace FloSeries3 AgriFlo Xci	Hardie Pvt	between Patterson & Oakdale	2.9	10	19	391	4	30"	0.5	Yes	No	Airvent D/S to be installed for Hach site

Rubicon typical installation includes:

3'x3' FlumeMeter with pedestal and solar panel (Product No. FM-900-900-2400), 12v dc data logger kit for FlumeMeter, Rubicon installation of FlumeMeter (\$750)

Sontek IQ pipe meter typical installation includes:

SonTek IQ pipe meter (SON-IQXP), 10-m power and RS232/SDI-12 Modbus (36-0012-010), flow meter data center (10PT-DA-CTR), and SonTek Flow Display (SON-FD) or HydroScientific West Interface Terminal Flow Display (HSW-0200) if multiple sites

Mace meter typical installation includes:

Mace FloSeries3-AgriFlo Xci, FloSeries3-Doppler Velocity Module, Sensor strap - 4m/s 10mCable-D89, Mounting Plate - Poly with mounting accessories (15 3/4"), solar panel for FloSeries3 12volt/5watt, Mace Device & Solar Panel mounting kit

The site locations were selected to represent different conditions within the District, but were purposely chosen to be in relatively close proximity to each other to aid in collecting test data. The location of the test sites is shown in Figure 7 below.



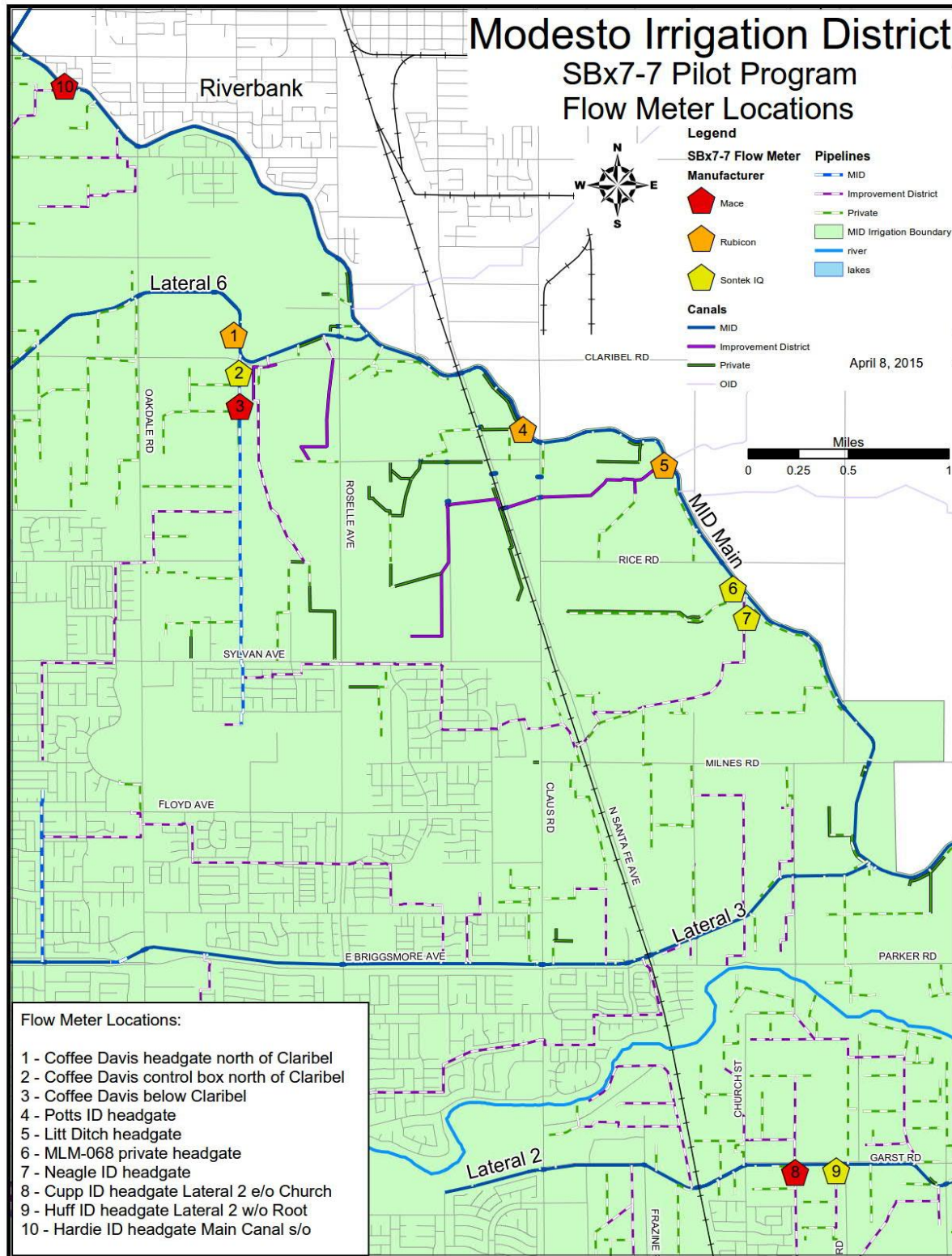


Figure 7 – Pilot Program Site Locations

Detailed cost information was maintained by the District for capital costs to purchase the meters that were tested and install the device with all associated appurtenances at each test site. The Rubicon SlipMeter was not tested, but costs were estimated for comparison. Costs incurred with the Pilot Program are shown below, but it should be noted that this was a small scale pilot project and it is expected that costs would be less with wide scale implementation.

Table 56 – Pilot Program costs per site

<b>Estimated Measurement Device Costs <sup>a</sup> per turnout site</b>					
	Device Capital	Installation Materials	Installation Labor	SCADA	Est. Installed Cost
Rubicon FlumeMeter <sup>b</sup>	\$ 16,200	\$ 2,000	\$ 4,000	\$ -	\$ 22,200
Rubicon SlipMeter (estimated)	\$ 25,000	\$ 2,000	\$ 4,000	\$ 8,000	\$ 39,000
SonTek IQ Pipe Meter	\$ 12,000	\$ 300	\$ 1,000	\$ -	\$ 13,300
Mace AgriFlo Xci (full pipe)	\$ 5,600	\$ 200	\$ 3,000	\$ -	\$ 8,800
Mace AgriFlo Xci (partial pipe)	\$ 8,900	\$ 200	\$ 3,800	\$ -	\$ 12,900
Hach portable meter <sup>c</sup>	\$ 50	\$ 300	\$ 200	\$ -	\$ 550
Meter gate <sup>d</sup>	\$ -	\$ -	\$ -	\$ -	\$ -

<sup>a</sup> Costs shown are what was incurred for Pilot Study, purchasing only 3 or 4 devices. It is expected that the capital cost would be reduced if a larger volume of devices are purchased, and labor costs would also likely be reduced as experience is gained in installation.

<sup>b</sup> If FlumeMeters are rotated to calibrate meter gates – assume each FlumeMeter stays at a site for at least the entire irrigation event (typically 21 days), then is moved to another site. Assuming one week to move to another site and set up, and assuming typical irrigation season of 7 months, each FlumeMeter could cover 7 sites once modifications have been made at each site to accept FlumeMeter. Capital cost per site would then be 1/7th that shown.

<sup>c</sup> Capital cost assumes 4 more Hach meters are purchased and utilized at 500 sites, and each site requires a new access vent.

<sup>d</sup> Assumes no capital modifications are required to existing meter gates.

Most measurement devices were operational in late April or early May, near the beginning of the 2015 irrigation season. During the irrigation season, District staff would periodically visit each site to collect data. Since SBx7-7 requires accuracy based on volume, two readings were always taken each day, one generally in the morning and one in the afternoon. While the time duration was not ideal (usually only a few hours between readings), drought conditions necessitated performing the test this way because growers typically ran shorter duration irrigations and it was desired to obtain readings while conditions were consistent. At each site the measurements were made sequentially for each device in the same order, so the time duration was constant for all devices on a given day.

- For the Rubicon, Sontek and Mace devices, the flow rate in cfs and volumetric reading in acre-feet from the device were obtained each time the site was visited. The measured volume delivered was the difference between the two readings. As a cross-check, the volume delivered by each device was also calculated based on the average of the flow rates over the time duration.

- For the meter gates, the gate opening and the head differential was recorded and the existing rating table for that gate was used to determine the instantaneous flow rate . The volume delivered was calculated based on the average of the flow rates over the time duration.
- For the Hach portable meter, the instantaneous flow rate was measured and recorded. The volume delivered was calculated based on the average of the flow rates over the time duration.

Because of the drought conditions and limited water supplies, some sites didn't operate very long and limited data was obtained. Table 56 illustrates how many days were measured at each site and the duration between the first reading and the last reading.

Table 57 – Measurement Devices tested

Site	Site ID No.	Acres Served	Measurement Device Type					Days Measured	Start Date	End Date	
			Rubicon	Sontek	Mace	Metergate	Hach				
L6-008	Coffee Davis	#1, 2 & 3	337	√	√	√		√	12	05/18/15	08/14/15
MLM-084	Potts Ditch	#4	292	√			√	√	27	05/06/15	09/17/15
MLM-074	Litt Ditch	#5	457	√			√	√	51	05/06/15	09/17/15
MLM-068	private	#6	306		√		√	√	35	04/22/15	09/14/15
MLM-066	Neagle ID	#7	447		√		√	√	15	04/22/15	09/08/15
L2-030	Cupp ID	#8	24			√	√	√	2	05/04/15	07/09/15
L2-028	Huff ID	#9	66		√		√	√	7	06/02/15	09/28/15
MLM-110	Hardie private	#10	391			√	√	√	7	05/21/15	09/16/15

*All three devices at the Coffee-Davis site (Rubicon, Sontek and Mace) contained data loggers which logged data continuously*

The Pilot Program data collected during the irrigation season is currently being analyzed. The Hach portable meter was the only measurement device that was used at all 8 sites, so the Hach portable meter was used to provide a comparison of the relative accuracy of the other devices. The District has confidence in the accuracy of the Hach portable meter, but inaccuracies could be introduced by comparing to a meter that is not calibrated for each site. Since SBx7-7 specifies an accuracy based on volumetric measurement, all comparisons we made to the *volume* of water estimated using measurements obtained with the Hach portable meter. However, since the volume of water delivered as measured by the Hach meter at each site was calculated based on the average of the instantaneous flow rates over the time duration, if the two instantaneous flow rate readings measured by the Hach on a given day were more than 10% different, the reading that day was not utilized in the analysis since it was unknown when the flow rate changed. The comparison of each device to the Hach meter on a volumetric basis for all sites is shown below. Site specific analysis of the data will be conducted to determine if there is variability across the sites. From the preliminary analysis it is apparent that additional testing is required, ideally under normal operating conditions.

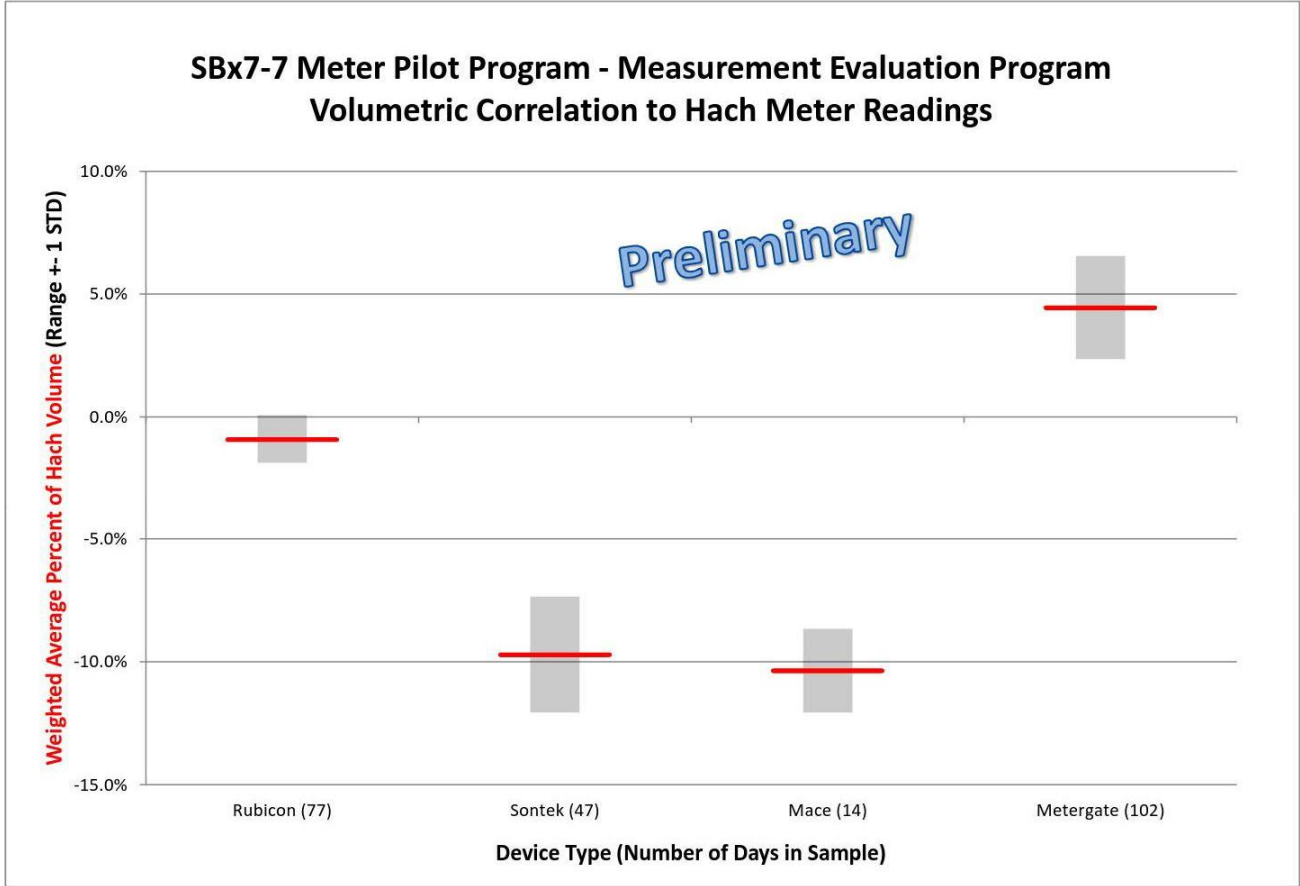


Figure 8 – Preliminary Pilot Program Test Results

Based on these preliminary findings, it appears that the Rubicon devices performed the best out of the three that were tested, but it should be noted that the other devices were installed in locations that ended up having limited water deliveries and hence limited data was collected. It should also be noted that the existing meter gates performed well compared to the Hach meter measurements. Additional testing and information is required before the District can select a device or method for wide scale implementation, but the District is utilizing these preliminary results from the Pilot Program to begin structuring a potential program to improve delivery point water measurement within the District.

The District is developing a program that it believes is implementable, locally cost effective, and compared to other alternatives has the most likely chance of being approved by landowners. Prior to plan implementation it will be necessary for the District to conduct a successful Proposition 218 election to fund the program costs. The District has developed the following goals for a water measurement program:

- The Program must be locally cost-effective and achieve the most “bang-for-the-buck” during implementation.
- The Program will employ water measurement using a combination of individual customer turnout measurement devices and lateral level (upstream) turnout measurements to multiple customers on private laterals (Improvement Districts).

- Measurement devices and methods will be standardized as much as possible, so that standardized operations can be used at delivery points throughout the District. Devices or methods used for canal and pipeline measurement will likely be different.
- For permanent installations on the largest delivery points, it may be desirable for the measurement device to indicate the instantaneous flow rate and the accumulated volume delivered and be readable in the field by both District staff and the agricultural water user, with the provision that data could be conveyed to the SCADA system in the future if desired.
- The measurement device must be a proven technology that the District and the agricultural water user can easily understand.
- The ability to secure the measurement device is important to prevent, or at least hinder, theft and vandalism.

Based on the results obtained in 2015 with the Pilot Program, the District has preliminarily selected a measurement method or device to be used at each delivery point in each acreage range group. Because drought conditions impacted the pilot program in 2015, some continued testing will occur during the 2016 irrigation season to verify or refine the recommended measurement devices or methods. For existing meter gates, a system for improved control of the time component, documenting the on and off times, is currently being developed to improve volumetric calculation accuracy. For purposes of developing a schedule and budget for implementation of measurement methods or devices at each delivery point, the District has preliminarily selected the following:

Table 58 - Preliminary\* Flow Measurement Implementation Plan

<b>Acreage Served by Turnout</b>	<b>Preliminary* Measurement Method / Device</b>
< 5 ac	No change. The volume of water delivered to Gardenhead** parcels is significantly less than 1% of the total volume delivered annually, so the existing turnout meter gate will continue to be used.
5 – 10 ac	No change. The acreage billed, and hence the amount of water delivered, through turnouts delivering water to parcels less than 10 acres is only approximately 1% of the total volume delivered annually, so the existing turnout meter gate will continue to be used for measurement.
10 – 50 ac	Serving approximately 17% of the irrigated acreage. Meter gate to be used for measurement. Hach or other portable meter will be used to verify meter gate flow readings. Measurement through meter gate at every turnout will be verified within approximately three years. Any meter gates that are found to have the measured flow rate outside $\pm 12\%$ accuracy will be re-calibrated with the Hach or other portable meter, or replaced if meter gate can't be properly calibrated.
50 – 100 ac	Serving approximately 15% of the irrigated acreage. <u>Canal turnouts</u> <ul style="list-style-type: none"> <li>• Installation of frame and pedestal mount for Rubicon FlumeMeter at each site. Rotate a FlumeMeter between approximately 7 sites during a typical year to verify or re-calibrate each individual meter gate. Each FlumeMeter to remain at a site for duration of at least one entire irrigation event. Once all meter gates have been verified with respect to accuracy or re-calibrated, rotation of FlumeMeters will continue and each meter gate will be verified/re-calibrated at least every five (5) years. Any meter gates that can't be verified or re-calibrated to within <math>\pm 12\%</math> accuracy will be replaced.</li> </ul> <u>Pipeline turnouts</u> <ul style="list-style-type: none"> <li>• Meter gate to be used for measurement. Hach or other portable meter will be used to verify or re-calibrate each individual meter gate. Once all meter gates have been verified or re-calibrated, Hach or other portable meter will continue to be used periodically to verify flow rates at each meter gate at least every five (5) years. Any meter gates that are found to have the measured flow rate outside <math>\pm 12\%</math> accuracy will be replaced if meter gate can't be properly calibrated.</li> </ul>

<b>Acreage Served by Turnout</b>	<b>Preliminary* Measurement Method / Device</b>
<p>&gt; 100 ac</p>	<p>Serving approximately 67% of the irrigated acreage. First priority for measurement improvements.</p> <p><u>Canal turnouts</u></p> <ul style="list-style-type: none"> <li>• Permanent installation of Rubicon SlipMeter (control gate, meter and data logger) at approximately one-half of the sites that deliver the largest volume of water. Volumetric delivery information will be stored in data logger. SCADA could be added in the future to collect flow and volumetric delivery data, and control the Rubicon SlipMeter if desired.</li> <li>• Installation of frame and pedestal mount for Rubicon FlumeMeter at remaining sites. Rotate a FlumeMeter between approximately 7 sites during a typical year to verify or re-calibrate each individual meter gate. Each FlumeMeter to remain at a site for duration of at least one entire irrigation event. Once all meter gates have been verified with respect to accuracy or re-calibrated, rotation of FlumeMeters will continue and each meter gate will be verified/re-calibrated at least every five (5) years. Any meter gates that can't be calibrated to within <math>\pm 12\%</math> accuracy will be replaced.</li> </ul> <p><u>Pipeline turnouts</u></p> <ul style="list-style-type: none"> <li>• Meter gate to be used for measurement. Hach or other portable meter will be used to verify or re-calibrate each individual meter gate. Once all meter gates have been verified or re-calibrated, Hach or other portable meter will continue to be used periodically to verify flow rates at each meter gate at least every five (5) years. Any meter gates that are found to have the measured flow rate outside <math>\pm 12\%</math> accuracy will be replaced if meter gate can't be properly calibrated.</li> </ul>

\* Preliminary Selection of Measurement Method/Device based on experience and limited testing in 2015 with Pilot Program. These selections are subject to change based on budget considerations and as more information is obtained and as technology develops. Additional testing is planned for the 2016 irrigation season.

\*\* Gardenhead parcels, which are typically less than five acres in size and separate or distinct from farm service parcels, are generally irrigated as a group with a standardized rotation.

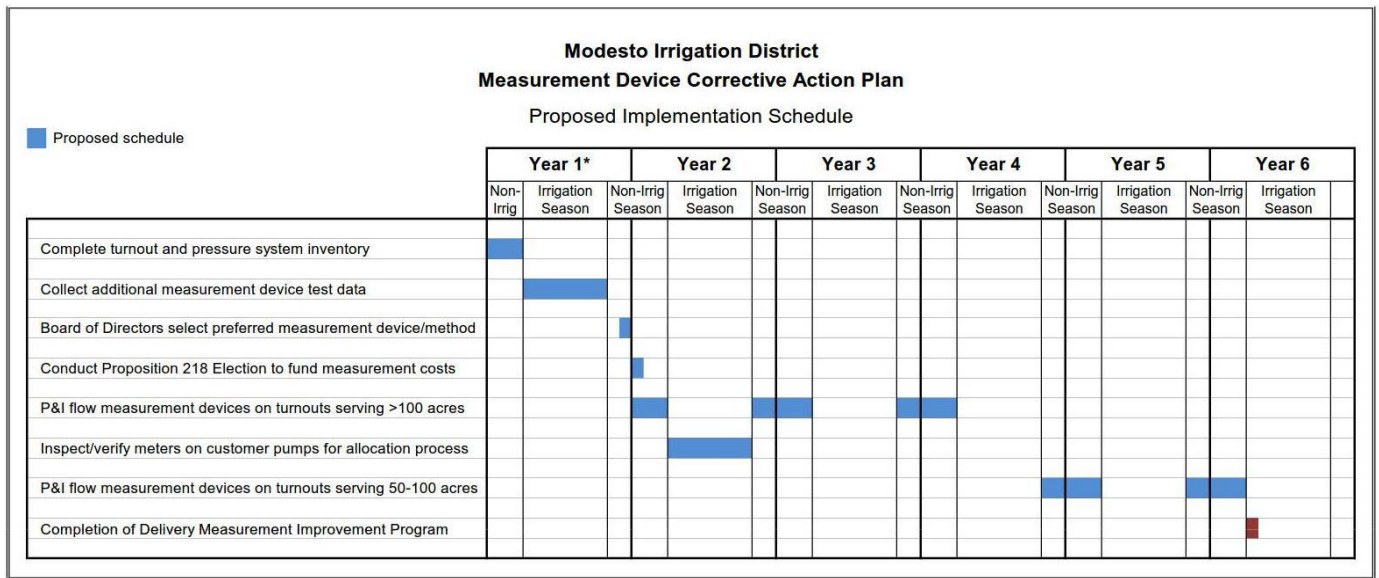
**Schedule**

The District has developed the schedule indicated below for implementation of its measurement device corrective action plan to comply with the measurement requirements of SBx7-7. As explained above, the District will utilize different levels of measurement depending on the acreage that is served by each delivery point and subsequent water use, but there are still approximately 300 locations that must be modified to accept a measurement device throughout the District for direct measurement or for re-calibration of the existing meter gate. Because of the large number of measurement devices that must be installed, the District has chosen a 5-year measurement device installation period, which results in over approximately 60 devices being installed each year on average. Installation of measurement devices will

begin following approval by the Board of Directors and a successful Prop 218 election to fund measurement costs. The anticipated time frame for compliance will be evaluated annually and may need to be revised depending on the availability of staff resources and funding needed to complete other programs and projects that MID is engaged in which also have a high priority, such as distribution system maintenance and other planned capital improvements consistent CWRMP.

Because drought conditions impacted the meter Pilot Program in 2015, some additional testing is desired during the 2016 irrigation season to verify and/or refine the recommended measurement devices or methods. Installation of measurement devices must occur during the non-irrigation season, generally mid-October to mid-March. Storm flows that are conveyed through the District distribution system may affect the ability to install measurement devices. Emphasis will be placed on installing measurement devices that serve the largest acreage first. The District’s proposed schedule for implementing a water measurement program at each delivery point is summarized below. Year 1 in the schedule below is assumed to be 2016, but purchase and installation of devices will not occur until the Board of Directors selects the preferred measurement device or method and the District conducts a successful Proposition 218 election to fund the program costs.

Table 59 - Proposed Implementation Timeline



P&I = Purchase and Install

\* Year 1 anticipated to be 2016

**Finance Plan**

The proposed water measurement implementation program may be funded through increased District assessments (fixed charge), which would require a Prop. 218 election. The cost to the District for the purchase and installation of the water measurement devices will be spread over time as measurement devices are installed each year during the anticipated 5-year installation period. The purchase of the water measurement devices will likely be funded by the District through a bond sale or from District reserves, and the fixed charges collected over time will pay the bond debt or reimburse the reserve fund.



**Budget**

The District currently budgets \$50,000 per year in its Water Operations Capital Budget program for measurement improvements. This annual budget must be substantially increased in order to fund the additional capital and O&M for water measurement that is required to comply with the measurement program set forth herein. Based on the meter pilot test program that was conducted in 2015 and preliminary selection of desired measurement devices, the implementation cost for the measurement program is estimated to be approximately \$4.5 million, as shown below:

Table 60 - Implementation Cost Forecast for Preliminary Measurement Implementation Plan

<b>Estimated Capital Purchase Schedule and Cost</b>							
Device	Cost / Unit	Year 1	Year 2	Year 3	Year 4	Year 5	Total
Hach	\$ 7,000	5	0	0	0	0	5
SlipMeter	\$ 25,000	29	28	28	0	0	85
FlumeMeter	\$ 16,000	7	7	6	5	5	30
<b>Subtotal:</b>		<b>\$ 872,000</b>	<b>\$ 812,000</b>	<b>\$ 796,000</b>	<b>\$ 80,000</b>	<b>\$ 80,000</b>	<b>\$ 2,640,000</b>
<b>Estimated Installation Schedule and Cost</b>							
Device	Cost / Unit	Year 1	Year 2	Year 3	Year 4	Year 5	Total
Hach (1/2)	\$ 500	60	60	60	0	0	180
SlipMeter	\$ 6,000	29	29	28	0	0	86
FlumeMeter	\$ 6,000	28	28	29	63	63	211
<b>Total</b>		<b>117</b>	<b>117</b>	<b>117</b>	<b>63</b>	<b>63</b>	<b>477</b>
<b>Subtotal:</b>		<b>\$ 372,000</b>	<b>\$ 372,000</b>	<b>\$ 372,000</b>	<b>\$ 378,000</b>	<b>\$ 378,000</b>	<b>\$ 1,872,000</b>
<b>Estimated Total:</b>		<b>\$ 1,244,000</b>	<b>\$ 1,184,000</b>	<b>\$ 1,168,000</b>	<b>\$ 458,000</b>	<b>\$ 458,000</b>	<b>\$ 4,512,000</b>

MID will monitor the measurement program activity on an on-going basis to determine whether or not this level of effort is sufficient and effective, and will make adjustments as needed to provide a technically sound, locally cost effective solution to improving water measurement at the farm-gate level.

As previously discussed, revised assessments and water toll charges that may be required to fund the measurement program are subject to Proposition 218. If the Proposition 218 election is unsuccessful, the District may not have sufficient funding available to implement the proposed water measurement program as set-forth herein.

## Section IX. References

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**MODESTO IRRIGATION DISTRICT**

**2015 AGRICULTURAL WATER MANAGEMENT PLAN**

**APPENDIX A**

**PUBLIC NOTICE OF PLAN PREPARATION**

# NOTICE OF PUBLIC HEARING

The Modesto Irrigation District (MID) Board of Directors will hold a public hearing regarding the 2015 Agricultural Water Management Plan on Tuesday, December 15, 2015, at 9 a.m. in the MID Board Room - 1231 11th Street, Modesto.

Agricultural water agencies in California are required to prepare an Agricultural Water Management Plan (AWMP). MID's current AWMP was adopted in 2012 and describes MID's water supplies and irrigation demand, local conditions, facilities and operations, rules and policies and a variety of water management activities, including a series of efficient water management practices (EWMPs) designed to improve water use efficiency. MID has updated its 2012 AWMP pursuant to the applicable regulatory requirements. In addition to the standard AWMP requirements, the draft revised AWMP also addresses the additional AWMP requirements established by the Governor's Executive Order B-29-15 (April 1, 2015).

MID's public review draft of the 2015 AWMP may be reviewed online at [www.mid.org](http://www.mid.org) or in the MID Irrigation Operation office located at 1231 11th Street, Modesto. Comments may be submitted at the public hearing or in writing before December 15 to:

John B. Davids, P.E.  
Irrigation Operations Manager  
Modesto Irrigation District  
P.O. Box 4060  
Modesto, CA 95352



Modesto  
Irrigation  
District

1231 11th Street | P.O. Box 4060 | Modesto, CA  
[www.mid.org](http://www.mid.org)

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Nov. 15, 2015; Dec. 1, 2015 and Dec. 8, 2015

**MODESTO IRRIGATION DISTRICT**

**2015 AGRICULTURAL WATER MANAGEMENT PLAN**

**APPENDIX B**

**RESOLUTION OF PLAN ADOPTION**

**RESOLUTION 2015-120**  
**ADOPTING MODESTO IRRIGATION DISTRICT'S 2015 AGRICULTURAL**  
**WATER MANAGEMENT PLAN COMPLETED IN ACCORDANCE WITH THE**  
**WATER CONSERVATION ACT OF 2009 (SBX7-7)**

WHEREAS, Modesto Irrigation District has been actively involved in agricultural water management planning efforts since Modesto Irrigation District joined the Agricultural Water Management Council in the late 1990's; and

WHEREAS, Modesto Irrigation District's first Agricultural Water Management Plan was prepared in voluntary compliance with provisions of California Assembly Bill 3616 in 1999; and

WHEREAS, Modesto Irrigation District's 1999 Agricultural Water Management Plan was submitted and approved by the Agricultural Water Management Council on behalf of the California Department of Water Resources; and

WHEREAS, Legislation passed in 2009, commonly referred to as SBx7-7, made the once voluntary program mandatory; and

WHEREAS, under the new requirements, Modesto Irrigation District is required to update the plan in 2015 and then every five years thereafter; and

WHEREAS, Modesto Irrigation District's current Agricultural Water Management Plan was adopted in 2012 and describes Modesto Irrigation District's water supplies and irrigation demand, local conditions, facilities and operations, rules and policies and a variety of water management activities, including a series of efficient water management practices designed to improve water use efficiency; and

WHEREAS, in addition to the standard Agricultural Water Management Plan requirements, the draft revised Agricultural Water Management Plan also addresses the additional Agricultural Water Management Plan requirements established by the Governor's Executive Order B-29-15 (April 1, 2015); and

WHEREAS, it is staff's intent to use this five year planning document as a short term strategic plan for the Irrigation Operations Division fitting under the umbrella of the larger, more long-term Comprehensive Water Resources Master Plan; and

WHEREAS, Modesto Irrigation District released the draft 2015 Agricultural Water Management Plan for a 30 day public review period on November 10, 2015 and subsequently held a public hearing on December 15, 2015 to hear and consider comments from the public on the draft 2015 Agricultural Water Management Plan.

BE IT RESOLVED, That the Board of Directors of the Modesto Irrigation District does hereby adopt Modesto Irrigation District's 2015 Agricultural Water Management Plan completed in accordance with the Water Conservation Act of 2009 (SBx7-7).

Moved by Director Wenger, seconded by Director Campbell, that the foregoing resolution be adopted.

The following vote was had:

Ayes: Directors Byrd, Blom, Campbell, Mensinger and Wenger


Noes: Director None

Absent: Director None

The President declared the resolution adopted.

o0o

I, Angela Cartisano, Secretary of the Board of Directors of the Modesto Irrigation District, do hereby CERTIFY that the foregoing is a full, true and correct copy of a resolution duly adopted at a regular meeting of said Board of Directors held the 15<sup>th</sup> day of December 2015.



Secretary of the Board of Directors  
of the Modesto Irrigation District



**MODESTO IRRIGATION DISTRICT**

**2015 AGRICULTURAL WATER MANAGEMENT PLAN**

**APPENDIX C**

**RULES AND REGULATIONS GOVERNING THE  
DISTRIBUTION OF IRRIGATION WATER WITHIN  
THE MODESTO IRRIGATION DISTRICT**

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**APRIL 2015**

**RULES AND REGULATIONS GOVERNING  
THE DISTRIBUTION OF IRRIGATION WATER  
WITHIN THE MODESTO IRRIGATION DISTRICT**

**PREAMBLE**

These Rules and Regulations Governing the Distribution of Irrigation Water Within the Modesto Irrigation District are established pursuant to Water Code Section 22257 to ensure the orderly, efficient and equitable distribution, use and conservation of the water resources of the District. The District will endeavor to deliver irrigation water in a flexible, timely manner consistent with the physical and operational limits of the delivery system facilities.

In addition to these Irrigation Rules, the District may enter into agreements and develop policies and programs to enhance service to our customers. To receive further information, please contact:

Modesto Irrigation District  
Irrigation Operations Division  
P.O. Box 4060  
Modesto, CA 95352  
(209) 526-7563

**This rules booklet supersedes  
"Rules and Regulations Governing the Distribution of Irrigation Water in the Modesto  
Irrigation District"  
(Last Revised April 2000)**

**MODESTO IRRIGATION DISTRICT MISSION STATEMENT**

To deliver superior value to our irrigation, electric and domestic water customers through teamwork, technology and innovation.

**IRRIGATION OPERATIONS DIVISION MISSION STATEMENT**

To responsibly manage the water resources of the District to provide a safe, reliable and sustainable supply for our agricultural and urban community.

**HISTORICAL NOTES**

Signing of the Wright Act in March of 1887 allowed for the formation of irrigation districts in California and gave them the power to conduct elections, issue bonds and acquire property. The Modesto Irrigation District was the second irrigation district formed under the new law. The Wright Act was named for C.C. Wright, the Modesto assemblyman who introduced the law.

The District first delivered irrigation water in 1904; the availability of such water changed the nature of the farming in the area within a few years. Large tracts of wheat were replaced with orchards and vineyards. Today Stanislaus County ranks among the top 10 agricultural counties in the nation.

**KEY DATES**

Established	July 23, 1887
Irrigation service started	1904
Electrical service started	1923
Waterford Irrigation District-merger	January 1, 1978
Domestic water treatment started	1994

**IRRIGATION FACTS**

No. of acres in the District	101,683
Irrigated acres	64,000
No. of accounts	3,400
Miles of canals and pipelines	208
Water source	Tuolumne River
Average Annual Modesto Rainfall	12 inches

## TABLE OF CONTENTS

SECTION 1: DEFINITIONS.....	1
SECTION 2: FACILITIES .....	4
2.1 CONTROL OF FACILITIES .....	4
2.2 ACCESS TO LANDS.....	4
2.3 ENCROACHMENTS .....	5
2.4 DESIGN AND CONSTRUCTION OF IRRIGATION FACILITIES.....	5
2.5 DESIGN AND CONSTRUCTION OF NON-IRRIGATION FACILITIES.....	6
2.6 DESIGN AND CONSTRUCTION OF PRIVATE OR IMPROVEMENT DISTRICT FACILITIES .....	6
SECTION 3: OPERATION OF DISTRICT FACILITIES .....	8
3.1 LIMITS OF LIABILITY .....	8
3.2 CONTROL OF DELIVERY POINTS.....	8
3.3 PUMPING OF IRRIGATION WATER.....	8
3.4 DISTRICT PUMPS .....	9
3.5 INTERFERENCE WITH DISTRICT FACILITIES .....	9
3.6 USE OF CANAL ROADS AND RIGHTS-OF-WAY .....	10
3.7 USE OF FACILITIES FOR WASTEWATER.....	11
3.8 USE OF FACILITIES FOR OTHER WATERS .....	11
3.9 MAINTENANCE OF PRIVATE OR IMPROVEMENT DISTRICT FACILITIES .....	11
3.10 FLOW THROUGH PRIVATE AND IMPROVEMENT DISTRICT FACILITIES .....	12
SECTION 4: DUTIES OF IRRIGATOR .....	13
4.1 IRRIGATOR RESPONSIBILITIES.....	13
4.2 USE OF WATER.....	13
4.3 LIABILITY FOR DAMAGE .....	14
SECTION 5: DELIVERY OF IRRIGATION WATER.....	15
5.1 WATER ALLOTMENT AND CHARGES.....	15
5.2 FAILURE TO PAY CHARGES.....	15
5.3 WATER USER INFORMATION .....	15
5.4 IRRIGATION SERVICE.....	15
5.5 MEASUREMENT OF WATER.....	16
5.6 REFUSAL OF WATER BY IRRIGATOR .....	17
5.7 INTERRUPTIONS OF SERVICE .....	17
5.8 SERVICE TO PRIVATE OR IMPROVEMENT DISTRICT SYSTEMS.....	17
5.9 IRRIGATION OF GARDENHEAD PARCELS.....	18
5.10 UNAUTHORIZED USE OF WATER.....	18

SECTION 6: DRAINAGE TO DISTRICT FACILITIES .....	19
6.1 DRAINAGE.....	19
6.2 TRANSPORTATION.....	19
6.3 DRAINAGE AND TRANSPORTATION CHARGES.....	20
SECTION 7: POLLUTION ABATEMENT .....	21
7.1 POLLUTION .....	21
7.2 CLEANUP .....	21
SECTION 8: ENFORCEMENT OF IRRIGATION RULES AND REGULATIONS.....	22
8.1 FAILURE TO COMPLY WITH RULES OR REGULATIONS .....	22
8.2 RESTORATION OF SERVICE .....	22
8.3 APPEAL OF A DECISION TO TERMINATE DELIVERY .....	22
PROCEDURES TO ORDER WATER, IRRIGATION EQUATIONS AND COMMON CONVERSIONS.....	23
APPENDIX "A" .....	24

## **SECTION 1: DEFINITIONS**

- 1.1. “Agreement” includes any license agreement or agreement of any nature, application, request for permission, permit, petition or contractual obligation entered into by and/or between a Landowner or Irrigator and the District.
- 1.2. “Irrigation Operations Manager” is the Irrigation Operations Manager of the District or the Irrigation Operations Manager’s authorized representative.
- 1.3. “Authorized agent / authorized representative” means a subordinate or other individual granted the authority to act on behalf of the District.
- 1.4. “Board” means the duly elected Board of Directors of the District.
- 1.5. “Canals” include canals, laterals, ditches, drains, flumes, pipelines, and all related water conveyance facilities.
- 1.6. “Canal Road” means the area within District Rights-of-Way maintained for the purpose of permitting the passage of District vehicles.
- 1.7. “Delivery Point” means the location at which the District transfers control of delivered water to the irrigator or group of irrigators.
- 1.8. “District” means the MODESTO IRRIGATION DISTRICT functioning under the Irrigation District Law of the California Water Code.
- 1.9. “District Canals” means Canals owned, operated and maintained by the District, but excluding Improvement District Facilities.
- 1.10. “District Facilities” means Facilities owned, operated and maintained by the District, but excluding Improvement District Facilities.
- 1.11. “District Rights-of-Way” includes all rights-of-way held by the District, in fee or by easement.
- 1.12. “Ditchtender” means the District employee, under the general direction of the Irrigation Field Services Manager, responsible for making direct irrigation deliveries to Landowners from the District’s irrigation system.
- 1.13. “Facilities” include dams, structures, wells, canals, pumps, reservoirs, and all other facilities and appurtenances thereto used for or in connection with the delivery, conveyance or receipt of water.
- 1.14. “Gravity Water” means water delivered to the end-user by means of gravitational flow.
- 1.15. “General Manager” or “GM” is the General Manager of the District or the GM’s authorized representative.

- 1.16. "Improvement District" is any sub-district involving two or more landowners within the District formed under the California Water Code and for the purpose of providing for the operation and maintenance of, and capital improvements to, Facilities not owned by the District.
- 1.17. "Improvement District Facilities" include all Facilities owned by an Improvement District.
- 1.18. "Irrigable" means all parcels with or without on-farm irrigation facilities that could be irrigated either by District or private water.
- 1.19. "Irrigation Field Services Manager" is the District employee, under general direction from the Irrigation Operations Manager, who is delegated the authority and responsibility to direct irrigation water deliveries and construction/maintenance of the District's irrigation system.
- 1.20. "Irrigation Rules" means these Rules and Regulations Governing the Distribution of Irrigation Water Within the Modesto Irrigation District, as duly adopted by the Board, and all regulations, policies, notices and procedures promulgated in accordance therewith.
- 1.21. "Irrigation Season" means that portion of the calendar year where surface irrigation water is generally made available to District Landowners. The Irrigation Season typically extends from March 1 to October 31, but may be modified each year as directed by the Board.
- 1.22. "Irrigator" means the Landowner or Renter of a parcel of land who has the primary responsibility for irrigating the parcel. The term includes the Irrigator's officers, employees, contractors and agents.
- 1.23. "Landowner" means holder of title or evidence of title to land.
- 1.24. "Laws" includes all federal, state and local statutes and ordinances, and all rules and regulations promulgated, and all orders and decrees issued, in connection therewith.
- 1.25. "Policy" means Agreements, rules, regulations, guidelines, and Procedures that authorize District staff to act on behalf of the District.
- 1.26. "Pollutant" means any foreign or deleterious substance or material, including but not limited to garbage, rubbish, refuse, animal carcasses, matter from any barnyard, stable, dairy or hog pen, herbicides, pesticides, fertilizers or any other material which is offensive to the senses or injurious to health, or which pollutes or degrades the quality of the receiving water or any flammable, explosive, or radio active material, toxic substance, hazardous waste, hazardous material, hazardous substance, or the equivalent, as those terms may now or in the future be defined by common practice or by Law. Filter station backflush water shall be allowed back into District facilities so long as chemical injection occurs



downstream of backflush location, proper backflow prevention is in place and the Landowner is in compliance with the irrigated lands regulatory program.

- 1.27. “Practice” is a customary activity or generally accepted method.
- 1.28. “Private Facilities” include all facilities owned by a person or entity other than the District or an Improvement District.
- 1.29. “Procedure” is an ordered series of steps developed by the District to guide interaction between District staff and the public.
- 1.30. “Program” is a plan or Procedure through which a Landowner may secure services, such as design, funding and/or financing, for irrigation system improvements.
- 1.31. “Renter” means a person or entity that leases, rents, or sharecrops land from a Landowner.
- 1.32. “Vehicle” means any motorized or self propelled vehicle, for air, water or land, including but not limited to boats, cars, motorcycles, bicycles, and all terrain vehicles.
- 1.33. “Water Allocation” means the quantity of water that is allocated annually by the Board for irrigation distribution to each acre of land within the District.

## **SECTION 2: FACILITIES**

### **2.1. CONTROL OF FACILITIES:**

2.1.1. District Facilities are under the exclusive direction, management and control of authorized District personnel. No persons other than authorized District personnel shall have any right to operate or interfere with said Facilities in any manner.

2.1.1.1. Each Irrigable parcel will be within an area assigned to a designated Ditchtender.

2.1.2. For assistance with Facilities, contact the Irrigation Field Services Manager at (209) 526-7637.

2.1.3. For emergency use of Facilities, contact the Irrigation Field Services Manager at (209) 526-7637.

### **2.2. ACCESS TO LANDS:**

2.2.1. Every District director, officer, employee, and authorized agent or representative shall have the right, at all times, to reasonably enter any land irrigated with water from the District for any of the following purposes:

2.2.1.1. Inspecting District Facilities; the flow of water within and through such Facilities (including measurement thereof); and the use of water on the land;

2.2.1.2. Determining the acreage of crops irrigated or to be irrigated;

2.2.1.3. Maintaining or operating District Facilities;

2.2.1.4. Investigating any incident or report involving District Facilities, or water originating from any District Facility;

2.2.1.5. Responding to an emergency upon notification from law enforcement or other person; and

2.2.1.6. Performing any work contemplated under these Irrigation Rules.

2.2.1.7. Should entry for the purposes set-forth herein be unreasonably denied, the current irrigation event may be terminated and re-establishment of irrigation event shall be coordinated with the Ditchtender.

2.3. ENCROACHMENTS:

- 2.3.1. No trees, vines, crops or other vegetation shall be planted and no encroachments shall be installed, constructed or placed in, on, over, under or across any District Facility or Right-of-Way unless such encroachment is consistent with District Policy and the District has given specific written approval for such encroachment. In granting such approval, the District may impose such conditions (including reasonable fees) and/or restrictions as District deems appropriate.
- 2.3.2. Upon written notification from the District to the Landowner owning the land adjacent to any unauthorized encroachment, said Landowner shall immediately remove such encroachment. If such encroachment is not promptly removed, the District may take all reasonable action to remove the encroachment at the sole expense of the Landowner.
- 2.3.3. Encroachments in, on, over, under or across any District Facility or District Right-of-Way that interfere with the operation or maintenance of that Facility may be removed by the District without notice, at the sole expense of the encroacher or adjacent Landowner.

2.4. DESIGN AND CONSTRUCTION OF IRRIGATION FACILITIES:

- 2.4.1. No irrigation system improvements, including Delivery Points, weirs, pump intakes, mechanical screens or structures of a similar nature, shall be planted, installed, constructed or placed in, on, over, under or across any District Facility or Right-of-Way unless written permission has first been granted therefore by the District. No person or entity receiving such Permission (a "Permittee") shall acquire any rights in District's Facilities or Rights-of-Way other than those set forth in District's written permission. Permittees shall, at their own expense, promptly upon receipt of notice from District, relocate or remove any improvement. In the event Permittee fails to do so, the District may perform such relocation or removal at Permittee's sole expense.
- 2.4.2. Unless otherwise specified by Agreement, all improvements shall be at the Permittee's sole expense, built to current District construction and engineering design standards, and shall become the property of the District upon completion.
- 2.4.3. All Delivery Points shall be capable of measuring the volume of water delivered in compliance with the prevailing state law and regulations promulgated by the California Department of Water Resources or other regulatory agency as may be applicable.
- 2.4.4. If the work can or has the potential to affect the flow of water in District conduits, the work must be performed during times pre-approved in writing by District. Ordinarily, in the absence of an emergency, such work will not be permitted during the period of March 1 to November 1.

2.5. DESIGN AND CONSTRUCTION OF NON-IRRIGATION FACILITIES:

- 2.5.1. No improvements, including buildings, bridges, gates, cross canal pipes, or structures of a similar nature, shall be planted, installed, constructed or placed in, on, over, under or across any District Facility or Right-of-Way unless written permission has first been granted therefore by the District. No Permittee shall acquire any rights in District's Facilities or District Rights-of-Way other than those set forth in District's written permission. Permittees shall, at their own expense, promptly upon receipt of notice from District, relocate or remove any improvement. In the event, Permittee fails to do so, the District may perform such relocation or removal at Permittee's sole expense.
- 2.5.2. Unless otherwise specified by Agreement, all authorized improvements shall be at the Permittee's sole expense, built to current District construction and engineering design standards, and shall become the property of the District upon completion.

2.6. DESIGN AND CONSTRUCTION OF PRIVATE OR IMPROVEMENT DISTRICT FACILITIES:

- 2.6.1. All new Private or Improvement District Facilities used for flood irrigation purposes shall provide for a minimum gravity flow of fifteen (15) cubic feet per second. A variance from this minimum flow shall be evaluated by the District on a case-by-case basis based on the impact on the operation of the District's water delivery system.
- 2.6.2. All new Private or Improvement District Facilities used for delivering water to pressurized irrigation systems shall be designed to meet the flow requirements of the land served by the Facility without impacting the irrigation operations of the District or other landowners served by the Facility.
- 2.6.3. Any proposed change in use or modification to an Improvement District Facility requires approval of two-thirds of the Improvement District members and obtaining consent shall be the sole responsibility of the Landowner.
- 2.6.4. The Irrigator will be required to install, operate, and maintain lift pumps, at Irrigator's expense, to receive water where the District is unable to deliver gravity water.
- 2.6.5. The location and tie-in of gravity or pump Facilities to District Facilities must meet District construction and engineering design standards and be approved in writing by the District prior to construction.
- 2.6.6. All plans for the installation, construction and placement of Private and Improvement District Facilities shall be submitted to the District for review. No installation, construction, or placement shall commence until

the District has reviewed the plans. The District's rights hereunder to review and accept the plans shall not impose any duties or obligations on the District, nor shall such rights relieve the Irrigator of the sole responsibility for the Facilities plans, schedules and installation, construction and placement work.

- 2.6.7. Pre-consultation with District Irrigation Operations Staff concerning the design and construction of improvements is strongly recommended.

## **SECTION 3: OPERATION OF DISTRICT FACILITIES**

### **3.1. LIMITS OF LIABILITY:**

- 3.1.1. The District's responsibility for the water shall absolutely cease when the water is diverted into any Private or Improvement District Facility or property.
- 3.1.2. The District shall not be liable for any nuisance or negligent, wasteful or other use or handling of water by any recipient or user thereof.
- 3.1.3. The District shall not be responsible for any trash, debris or other matter that may flow or accumulate in the water. The District shall not be responsible for any interference with, decrease in the operation or capacity of, or damage to Facilities as a result of such trash, debris or other matter.
- 3.1.4. The District is not a guarantor of service and shall not be liable for any damage any person may suffer as a result of water not being delivered.

### **3.2. CONTROL OF DELIVERY POINTS:**

- 3.2.1. The District has sole right and responsibility to operate Delivery Points and valves within District Canals. The Ditchtender may authorize an Irrigator to operate a Delivery Point or valve during the period when the Irrigator is scheduled to receive water. In such event the authorized Irrigator shall follow any Delivery Point or valve operational instructions issued by the Ditchtender and shall operate the designated Facilities in a safe and prudent manner. The Irrigator shall be liable for any and all damage resulting directly or indirectly from the Irrigator's operation of District's Facilities.
- 3.2.2. The District may take any action it deems appropriate to secure District Delivery Points, valves and other Facilities, including the use of locks and chains. Irrigators or groups of Irrigators may install locks on District Facilities only with the prior consent of the District. No lock installed by any Irrigator shall interfere with District's use or operation of the Facility.
- 3.2.3. The District may seal or remove, or require a Landowner to seal or remove, at Landowner's sole expense, any Delivery Point or valve where service from that Facility is no longer required by the Landowner.
- 3.2.4. All Delivery Points from District Facilities shall have a point of positive shut-off easily accessible to the Ditchtender within the District Rights-of-Way.

### **3.3. PUMPING OF IRRIGATION WATER:**

- 3.3.1. Water pumped from District Canals shall be subject to all rules and regulations governing the use of Gravity Water.

3.3.2. Water pumped from District wells shall be subject to all rules and regulations governing the use of Gravity Water.

3.4. DISTRICT PUMPS:

3.4.1. The District, within its sole discretion, shall determine when to run District owned irrigation and drainage pumps. The times of operation may depend upon a variety of circumstances, including the groundwater level near the pump, available supply, peak power load, and the quality of the water being pumped.

3.4.2. District drainage pumping Facilities will not be installed to serve individual acreage. Perched water table control on individual parcels is the responsibility of the Landowner.

3.4.3. District pumps shall be operated during the non-irrigation season, only at the District's discretion.

3.4.4. Irrigators may rent District pumps, as available, in accordance with the terms and conditions of District's Pump Rental Agreement.

3.5. INTERFERENCE WITH DISTRICT FACILITIES:

3.5.1. Any use of, interference with or damage to any District Facility, including Canals or Canal Roads, is, unless specifically permitted by these Irrigation Rules, prohibited.

3.5.2. No persons other than authorized District employees and agents, and persons permitted in accordance with these Irrigation Rules, shall:

3.5.2.1. Operate any District Facility.

3.5.2.2. Enter onto or into any District Facility

3.5.2.3. Attach, place or remove any boards, chains, ropes, or any other object to, on, in, or upon any District Facility or Canal Road.

3.5.2.4. Attach, place or remove any sign, board, post, fence, or gate to, on, in, or upon any District Facility or Canal Road.

3.5.2.5. Install, place, construct, operate or use any obstruction on, in, or upon any District Facility or Canal Road.

3.5.2.6. Operate, park, abandon or dispose of any Vehicle on, in, or upon any District Facility or Canal Road.

3.5.2.7. Use District property or Facilities for water sports or other recreational purposes, including without limitation surfing, skiing, boating, hunting or camping.

3.6. USE OF CANAL ROADS AND RIGHTS OF WAY:

- 3.6.1. Except as otherwise specifically permitted by the District in writing, no person shall cross any District Canal, including without limitation any weir, bridge or other crossing, except those clearly marked for public use.
- 3.6.2. No unauthorized vehicle shall be on or within District Canal Roads or Rights-of-Way. District Canal Roads and Rights-of-Way are for the exclusive use of authorized District employees and agents, and other authorized persons permitted in accordance with these Irrigation Rules. Persons requiring a specific use of a Canal Road or Right-of-Way may apply to the District for written permission prior to such use. Notwithstanding any permission granted by the District, use of District Canal Roads and Rights-of-Way is at the sole risk of the user.
- 3.6.3. The following persons have permission to operate a vehicle upon a District Canal Road or Right-of-Way consistent with District Rights-of-Way Policy 94-01.
  - 3.6.3.1. Any District Director, officer, employee, or authorized agent in the performance of their duties.
  - 3.6.3.2. Persons actively involved in farming a parcel of land adjacent to the specific District Canal Road or Right-of-Way.
  - 3.6.3.3. Persons actively involved in farming who use the specific District Canal Road or Right-of-Way for access to irrigation facilities serving their parcel of land.
  - 3.6.3.4. Persons whose property is directly adjacent to a District Canal and to whom permission for ingress and egress to the property has been granted by the District.
  - 3.6.3.5. Private parties who have made temporary ingress-egress arrangements in writing with the District for property maintenance or construction purposes.
  - 3.6.3.6. Any sheriff, police, fire, or public safety personnel on official business.
  - 3.6.3.7. Any District contractor who needs to use a specific District Canal Road or Right-of-Way to perform work under its contract with the District.
- 3.6.4. All vehicles using District Canal Banks or Rights-of-Way shall be operated in a safe and lawful manner at all times.



3.7. USE OF FACILITIES FOR WASTEWATER

3.7.1. No Pollutant, shall be, or permitted to be, placed, drained, spilled or otherwise discharged into or onto any District Facility or Canal Road.

3.7.2. No District Facilities shall be used for transportation of manure or other livestock waste of any kind, except with the prior written approval of the District which shall not be granted except under special circumstance, consistent with the District's Water Quality Policy.

3.7.2.1. Any person who violates this rule may be subject to criminal prosecution and civil liability.

3.8. USE OF FACILITIES FOR OTHER WATERS

3.8.1. Nothing other than District water, shall be transported through District Facilities at any time, except with the prior written approval of the District. All water transported through District Facilities shall be of a quality and quantity acceptable to the District.

3.8.2. Permission to use District Facilities as set forth in this Section 3.8 is at the sole discretion of the District and the District may impose reasonable conditions on such permission, including but not limited to the right of the District to approve and monitor the transporter's water measurement facilities. Any permission granted shall be revocable by the District at any time.

3.8.3. A service charge will be made by the District for transporting the water of others through District Facilities. The amount of this service charge will be set from time to time by the Board. All costs of transporting the water of others through District Facilities shall be borne by the person whose water is being transported.

3.8.4. Gates and/or pumps from waste water lagoons that are connected to District Facilities, in any way, must have a District approved and functional backflow prevention device.

3.9. MAINTENANCE OF PRIVATE OR IMPROVEMENT DISTRICT FACILITIES:

3.9.1. Each active Improvement District shall appoint at least two Improvement District Committee members who shall be authorized to approve all required maintenance and repair work.

3.9.1.1. Facilities maintenance and repair work for an Improvement District is the responsibility of the Improvement District.

3.9.1.2. Improvement District Landowners shall procure and pay for all materials and labor related to such maintenance and repair

work. Said costs shall be prorated on a per acre basis unless otherwise agreed by the Landowners.

3.9.1.3. The District may at its discretion, if requested by the Improvement District Committee, provide maintenance and repair services for Improvement District Facilities.

3.9.2. Private Facility maintenance and repair work is the responsibility of the Landowner(s) being served by the Private Facility.

3.9.3. Private or Improvement District Facilities may be cleaned or repaired by the District at the Landowner or Improvement District's expense when the District determines such action is necessary for the District's operations.

Maintenance and repair of irrigation valve structures on District or Improvement District Facilities are the responsibility of the Landowner of the property being served by those Facilities.

3.10. FLOW THROUGH PRIVATE AND IMPROVEMENT DISTRICT FACILITIES:

3.10.1. All Private and Improvement District Facilities must be free from weeds and other obstructions, and properly maintained, to permit sufficient capacity to carry the flow of water requested by any Irrigator, without the danger of levee breaks, overflow, or undue seepage.

3.10.2. The District may curtail or terminate the delivery of water to any Private or Improvement District Facility not meeting the above requirements and require the Facility to be cleaned, repaired, or reconstructed before water delivery is restored.

## **SECTION 4: DUTIES OF IRRIGATOR**

### **4.1. IRRIGATOR RESPONSIBILITIES:**

- 4.1.1. All land to be irrigated must be properly prepared to efficiently receive the water.
- 4.1.2. Landowners and Renter shall ensure that there is an Irrigator on the land at all times that water is made available to the land by the District.
- 4.1.3. The Irrigator shall be responsible for and shall attend and control the water at all times after it leaves District Facilities.
- 4.1.4. The Irrigator shall use the water continuously, day and night, from the commencement of water delivery until the completion of irrigation.
- 4.1.5. The Irrigator shall ensure that all irrigation Facilities are in working condition and ready to receive water at the irrigation start time, including but not limited to the opening and closing of valves and gates as needed.
- 4.1.6. The Irrigator is responsible for priming the pipeline prior to use. Priming shall be limited to 3” of stem opening or as directed by the Irrigation Field Services Manager; more than 3” of stem opening and landowner will be charged for water delivered, as determined by District.
- 4.1.7. The Irrigator shall close all gates and valves on the Irrigator's Private Facilities at the end of each irrigation.
- 4.1.8. The Irrigator shall call the Ditchtender immediately after each irrigation to report the irrigation start and stop times. If the Irrigator does not call promptly, irrigation time may be estimated by the District.
- 4.1.9. As directed by the Ditchtender, the Irrigator shall, at the end of the irrigation, call and notify the next Irrigator receiving water.

### **4.2. USE OF WATER:**

- 4.2.1. All water must be applied efficiently and used reasonably and beneficially.
- 4.2.2. Except as otherwise expressly permitted by these Irrigation Rules, all water shall be used solely for irrigation purposes; provided, however, that an Irrigator may use District water for crops related to cultural practices through the normal irrigation schedule.
- 4.2.3. The District may refuse to deliver District water to any Irrigator who misuses or wastes water either willfully or carelessly, in any way, including but not limited to the following:
  - 4.2.3.1. Flooding of roads, vacant land, or land previously irrigated.

4.2.3.2. Defective or inadequate non-District Canals or Facilities.

4.2.3.3. Inadequately prepared land.

4.2.3.4. Flooding any part of any land to an unreasonable depth or amount, including for the purpose of irrigating other portions of the land.

4.2.3.5. Flooding across one parcel to irrigate another parcel.

4.2.4. Any person, through acts or omissions, allowing water to discharge upon a public road or highway is liable for any resulting damages and may be subject to fines and/or penalties.

#### 4.3. LIABILITY FOR DAMAGE:

4.3.1. The Irrigator is responsible and liable for any damage caused by the Irrigator's failure to fulfill each of the obligations set forth in these Irrigation Rules, by the Irrigator's negligent or careless use or control of water, or by the Irrigator's improper operation or maintenance of any Facility for which the Irrigator is wholly or partially responsible.

## **SECTION 5: DELIVERY OF IRRIGATION WATER**

### **5.1. WATER ALLOTMENT AND CHARGES:**

- 5.1.1. Each year the Board of Directors shall establish the quantities of water available for each acre of service, the charges for water, the terms for the transfer of water, and any other provisions or charges for service as the Board may find appropriate.
- 5.1.2. Parcels less than or equal to ten acres in size may sign-off consistent with the District's Irrigation Water Activation and Reactivation Policy.
- 5.1.3. All water charges, Improvement District charges and assessments, and other irrigation or drainage related charges shall be due and payable as stated by Board Resolution and notices in billing statements.

### **5.2. FAILURE TO PAY CHARGES:**

- 5.2.1. The District may refuse to furnish water to any parcel of land if outstanding charges for water or services already furnished or rendered to such land (including any accrued interest and penalties) have not been paid in full by the District's prescribed payment date.
- 5.2.2. All charges placed on an individual parcel of land are the responsibility of the Landowner. In accordance with the provisions of Section 25806 of the Water Code, delinquent water service charges and/or assessments, together with all imposed penalties, for a parcel of land will be made a lien on the subject real property.

### **5.3. WATER USER INFORMATION:**

- 5.3.1. No later than May 1 of each year, each Landowner or designee shall provide to the District a signed statement, on the District's form, of the kinds of crops and number of acres of each crop that will be irrigated on each parcel of land, and such other relevant information as the District may reasonably require on the same statement. After May 1 of each year, no changes to the amount of irrigated acreage or non-irrigated acreage will be allowed, but the kind of crop that is going to be planted may be changed at any time.

### **5.4. IRRIGATION SERVICE:**

- 5.4.1. To schedule an irrigation, the Irrigator must place an order with the Ditchtender. The Ditchtender will generally schedule the water to be delivered within 5 days, subject to system limitations. In the event that an Irrigator is not ready to receive the water at the scheduled time, the Irrigator will be required to wait until the Ditchtender can reschedule water to the parcel.

- 5.4.2. Where possible, irrigation water will be provided to the Irrigator based on an arranged demand delivery, under which the delivery rate is fixed, but the frequency and duration of use are requested by the Irrigator. Where the capacity of the system is limited, rotation delivery may be used by the Ditchtender. The Ditchtender may, at the Ditchtender's discretion, alter the rotation or cause water to be delivered upon request. Advance notice for rotation deliveries will be made with an appropriate amount of warning time to take into consideration the preparation needed to commence irrigation.
- 5.4.3. Any Irrigator who desires irrigation water on a tailored delivery schedule is required to submit a detailed application to the District for consideration. Ditchtender will endeavor to meet the tailored delivery schedule, but District does not and cannot guarantee deliveries in accordance with the tailored delivery schedule.
- 5.4.4. The Ditchtender will inform each Irrigator of the anticipated date and time of water delivery to each of the Irrigator's parcel(s) of land. The Ditchtender will provide information on flows, Delivery Point(s) and valve operation, and any special instructions related to the delivery sequence.
- 5.4.5. In the event that an Irrigator cannot be contacted, located, or otherwise reasonably notified of the availability of water, the Ditchtender may move that water to another Irrigator. In doing so, the Ditchtender will make all reasonable efforts to make water re-available to the Irrigator as soon as feasible within the capacity limitations of the District's Facilities while maintaining efficient and equitable water distribution among Irrigators.
- 5.4.6. The Ditchtender will endeavor to meet the scheduled time of delivery within the capacity limitations of the District's Facilities while maintaining efficient and equitable water distribution among Irrigators.
- 5.4.7. The District shall not be required to raise water in its Canals to any height in order to deliver irrigation water to lands or ditches deemed by the District to be of unusually high elevation.
- 5.4.8. The District will strive to supply water of sufficient quality to those crops which are sensitive to certain constituents or parameters. However, the District does not and cannot guarantee the quality of water that is delivered to any irrigator and therefore shall not be liable for any damages that may result from the application of supplied irrigation water.

## 5.5. MEASUREMENT OF WATER:

- 5.5.1. All measurements of water delivered by the District to an Irrigator shall be made by the District at the Delivery Points or valve in District's Canal, or at such other appropriate location as the District may determine. The District shall maintain records of the names of each Irrigator, the parcel(s)

of land that each Irrigator has irrigated, the number of acre feet of water delivered to each parcel, and other information deemed appropriate by the District.

- 5.5.2. The District has the authority to install or require the installation and maintenance of irrigation flow measurement devices or structures at all District Delivery Points in compliance with the prevailing state law and regulations promulgated by the California Department of Water Resources or other regulatory agency as may be applicable.

5.6. REFUSAL OF WATER BY IRRIGATOR:

- 5.6.1. If an Irrigator fails or refuses to continuously use the full head of water delivered to a parcel of land or scheduled for delivery, then the following shall apply:

- 5.6.1.1. The full amount of water normally delivered will be charged to the Irrigator;

- 5.6.1.2. The Irrigator shall not be entitled to use the unused portion of water at any other time;

- 5.6.1.3. The Irrigator will be required to reschedule for delivery of water.

5.7. INTERRUPTIONS OF SERVICE:

- 5.7.1. When a break occurs in any water distribution facility requiring an interruption of irrigation service, the Irrigator whose irrigation was interrupted, shall be allowed to finish irrigating when service is restored and shall not claim another irrigation during the affected irrigation cycle or rotation.

- 5.7.2. Upon completion of the repair, and provided there is no conflict with current usage, the Ditchtender will endeavor to re-establish service based on the original schedule. Where use conflict occurs, service will be restored at the discretion of the Ditchtender.

5.8. SERVICE TO PRIVATE OR IMPROVEMENT DISTRICT SYSTEMS:

- 5.8.1. Water deliveries to Irrigators who use Private or Improvement District Facilities shall be delivered to the Delivery Point of these Facilities by the Ditchtender.

- 5.8.2. Caution is required when priming, operating and closing canal gates in order to avoid damage to Facilities and the disruption of service caused by such damage.

5.8.3. Landowners shall be responsible for the actions of their Irrigators when taking water through and from Private or Improvement District Facilities.

5.9. IRRIGATION OF GARDENHEAD PARCELS:

5.9.1. Gardenhead parcels, which are typically less than five acres in size and separate or distinct from farm service parcels, will be irrigated as a group, where possible, with a standardized rotation irrigation flow consistent with the capacity of the gardenhead parcel irrigation Facilities. The gardenhead irrigation rotation is normally established by the Ditchtender given the annual allocation, and is subject to modifications by the Ditchtender.

5.9.2. Deliveries of water for irrigation of gardenhead parcels will be scheduled by the Ditchtender and may be subject to interruption when water is in short supply or otherwise when it is necessary for the proper irrigation of farm service areas.

5.9.3. Such service to gardenhead parcels shall not interfere unreasonably with the regular irrigation of farm service areas.

5.10. UNAUTHORIZED USE OF WATER:

5.10.1. Any person who uses District water without the District's permission may be assessed a \$1,500 fine for unauthorized use of water as determined by the Board..

5.10.2. Any person who uses District water without the District's permission a second time as determined by the Board, may lose any remaining allocation.

5.10.3. Unauthorized use of water constitutes failure to comply with Rules or Regulations and enforcement of this section shall be consistent with Section 8.1.

5.10.4. Any and all conditions for re-establishment of service shall be as set-forth in Section 8.2.1.

5.10.5. Following decision by Board as set-forth in Section 5.10.1 or 5.10.2 an appeal may be made to the Board.

5.10.6. Following a decision to uphold the fine by the Board, such unauthorized use may be posted on District's public website.



## **SECTION 6: DRAINAGE TO DISTRICT FACILITIES**

### **6.1. DRAINAGE:**

- 6.1.1. Notwithstanding any other provisions of these Irrigation Rules, no surplus irrigation water, storm water, wastewater, tile drainage, or any other water or substance shall be drained, dumped, pumped, siphoned or otherwise discharged into any District Facility without the prior written agreement of the District. In granting permission to discharge, the District may impose reasonable conditions, including, without limitation, the right of the District to approve and monitor the discharger's measurement facilities. Permission to discharge shall be revocable by the District at any time and for any reason.
- 6.1.2. Water and other substances discharged into District Facilities shall meet all applicable federal, state and local water quality standards.
- 6.1.3. Filter station backflush water shall be allowed back into District facilities so long as chemical injection occurs downstream of backflush location, proper backflow prevention is in place and the Landowner is in compliance with the irrigated lands regulatory program.
- 6.1.4. The rate and quantity of discharge into any District Facility may be subject to limitations based on the capacity of the Facility and the quality of the water or other substance being discharged.
- 6.1.5. All discharge Facilities shall be constructed at the discharger's sole expense to and must meet the District's construction and engineering design standards.
- 6.1.6. All existing field drainage Facilities not currently covered by an agreement shall be subject to the District's current terms and conditions.
- 6.1.7. Gates and/or pumps from waste water lagoons that are connected to District Facilities, in any way, must have a District approved and functional backflow prevention device.

### **6.2. TRANSPORTATION:**

- 6.2.1. No person shall transport any water or other substance through District Facilities without the prior written agreement of the District. In granting permission to transport water or other substances, the District may impose reasonable conditions, including, without limitation, the right of the District to approve and monitor the transporter's measurement facilities. Permission to transport shall be revocable by the District at any time and for any reason.
- 6.2.2. Water and other substances transported through District Facilities shall meet all applicable federal, state and local water quality standards.

- 6.2.3. The rate and quantity of water and other substances transported through any District Facility may be subject to limitations based on the capacity of the Facility and the quality of the water and other substances being transported.
- 6.2.4. All transport Facilities shall be constructed at the transporter's sole expense and must meet the District's construction and engineering design standards.
- 6.2.5. All existing transportation Facilities not currently covered by an agreement shall be subject to the District's current rate, quantity, quality and other terms and conditions.

6.3. DRAINAGE AND TRANSPORTATION CHARGES

- 6.3.1. All costs of discharging into or transporting through District Facilities, as well as costs of associated carriage loss, shall be borne and paid by the discharger or transporter. A service charge will be assessed by the District for discharging or transporting through District Facilities. The amount of this service charge will be set from time to time by the Board of Directors.

## **SECTION 7: POLLUTION ABATEMENT**

### **7.1. POLLUTION:**

- 7.1.1. No Pollutant shall be placed, carried, transported, drained, dumped, pumped, siphoned, discharged, or otherwise allowed to enter into, onto, over, under or across any District Facility or associated Right-of-Way without the consent of the District.
- 7.1.2. Any person who violates this Rule may be subject to criminal prosecution and/or civil liability.

### **7.2. CLEANUP:**

- 7.2.1. Any person who willfully or negligently causes or permits any Pollutant to be placed, carried, transported, drained, dumped, pumped, siphoned, discharged, or otherwise allowed into, onto, over, under or across any District Facility or associated Right-of-Way without the prior written consent of the District shall immediately notify the District and take all action to mitigate the effects of such Pollutant. Such person shall, at that person's sole expense, unless otherwise directed by the District, perform or cause to be performed all necessary remediation to the District's satisfaction and in compliance with all applicable laws. Such person shall cooperate with the District to complete the remediation and shall reimburse the District for all costs and expenses incurred in connection with the remediation, including but not limited to administrative, investigative, and legal costs, fines and penalties.
- 7.2.2. No water shall be delivered to any parcel of land from which the pollutant originated or to any other parcel of land owned, rented, leased or irrigated by the person who caused or permitted any Pollutant into, onto, over, under or across any District Facility or associated Right-of-Way, until the remediation required in Section 7.2.1 is complete, all damages, costs and expenses, arising out of such event have been paid, and action satisfactory to the District has been taken to ensure that such event will not be repeated.

## **SECTION 8: ENFORCEMENT OF IRRIGATION RULES AND REGULATIONS**

### **8.1. FAILURE TO COMPLY WITH RULES OR REGULATIONS:**

- 8.1.1. Failure or refusal of any Landowner, Renter or Irrigator to comply with any of these Irrigation Rules or applicable regulations, or any part thereof, may be sufficient cause for curtailment or termination of delivery of District water to the lands of such Landowner, Renter or Irrigator.
- 8.1.2. Interference by any Landowner, Renter or Irrigator with a District employee, agent or official in the discharge of their duties may be sufficient cause for curtailing or terminating delivery of District water to the lands of such Landowner, Renter or Irrigator.
- 8.1.3. The District may immediately terminate the delivery of District water supplied to any parcel of land if the condition of the land or irrigation Facility present an immediate danger to any person, to the general public, or to any property, including but not limited to the flooding of property.
- 8.1.4. Compliance with each and all of these rules shall be a condition precedent to the delivery of water to any Irrigator. The Board retains the authority to make determinations regarding continued irrigation service in all instances that are not specifically contained in these rules and regulations.

### **8.2. RESTORATION OF SERVICE:**

- 8.2.1. Water delivery shall not be restored until full compliance with requirements established by these Irrigation Rules and Regulations is established and any other conditions for re-establishment of service as determined by the Board.

### **8.3. APPEAL OF A DECISION TO TERMINATE DELIVERY**

- 8.3.1. From a decision of the Ditchtender, an appeal may be made to the Irrigation Field Services Manager. From any decision of the Irrigation Field Services Manager, an appeal may be made to the Irrigation Operations Manager. From any decision of the Irrigation Operations Manager, an appeal may be made to the GM. From any decision of the GM, an appeal may be made to the Board. If an appeal from any decision is not made within fourteen (14) days of the date of the decision, the decision will be deemed final and the failure to appeal a decision in the manner and within the time period set forth above shall constitute a waiver of all rights to further protect, judicial or otherwise.

**PROCEDURES TO ORDER WATER:**

- A. Prepare your field to receive water.
- B. Contact your Ditchtender to place an order.
- C. Your Ditchtender will inform you of the time sequence, and other details regarding water delivery.

**IRRIGATION EQUATIONS:**

inches of water	=	$\frac{(\text{cfs flow}) \times (\text{hours irrigated})}{\text{acres served}}$
hours irrigated	=	$\frac{(\text{inches of water}) \times (\text{acres served})}{\text{cfs flow}}$
cfs flow	=	$\frac{(\text{inches of water}) \times (\text{acres served})}{\text{hours irrigated}}$
acre feet	=	$\text{cfs} (\text{hours irrigated} / 24) (1.983)$
number of acres	=	$\frac{(\text{cfs flow}) \times (\text{hours irrigated})}{\text{inches of water}}$

For example, a 20 acre parcel with a standard cfs irrigation flow will receive 6 inches of water in an 8 hour period.

6 inches	=	$\frac{(15 \text{ cfs flow}) \times (8 \text{ hours})}{20 \text{ acres}}$
----------	---	---

**COMMON CONVERSIONS:**

1 cubic foot per second (cfs) = 449 gallons per minute

1 cubic foot per second for 12 hours = 1 acre foot

1 acre foot = 325,900 gallons

1 acre foot = 43,560 cubic feet

An acre foot is the amount of water needed to cover 1 acre with 12 inches of water.

## APPENDIX “A”

Pertinent Provisions of law:

Water Code Section 22257 provides in part as follows:

“Each district shall establish equitable rules for the distribution and use of water, which shall be printed in convenient form for distribution in the district. A district may refuse to deliver water through a ditch which is not clean or not in suitable condition to prevent waste of water and may determine through which of two or more available ditches it will deliver water.”

“A district may close a defective gate in a community water distribution system used for irrigation purposes and may refuse to deliver water through the defective gate if the landowner fails to repair the gate or outlet to the satisfaction of the district within a reasonable time after receipt of notice from the Board through its authorized water superintendent, manager, or ditchtender to repair the gate outlet.”

Water Code Section 22282.1 provides that:

“A district may refuse service to any land if outstanding charges for services already rendered such land have not been paid within a reasonable time.”

Penal Code Section 592 provides that:

“Every person who shall, without authority of the owner or managing agent, and with intent to defraud, take water from any canal, ditch, flume or reservoir used for the purpose of holding or conveying water for manufacturing, agricultural, mining, irrigating or generation of power, or domestic uses, or who shall without like authority, raise, lower or otherwise disturb any gate or other apparatus thereof, used for the control or measurement of water, or who shall empty or place, or cause to be emptied or placed, into any such canal, ditch, flume or reservoir, a rubbish, filth or obstruction to the free flow of the water, is guilty of a misdemeanor.”

**MODESTO IRRIGATION DISTRICT**

**2015 AGRICULTURAL WATER MANAGEMENT PLAN**

**APPENDIX D**

**IRRIGATION RATE STRUCTURE**

## MODESTO IRRIGATION DISTRICT Irrigation Rate Structure (Pre-Volumetric Pricing)

Updated By: Carrie L  
Date: 8/27/15

Year	Base Allotment <sup>1</sup> (inches)	Groundwater Recharge Component <sup>2</sup> (inches)	Base Water Charge <sup>3</sup> (\$/acre)	Excess Water <sup>4</sup> Tier #1 (inches)	Excess Water Tier #1 (\$/AF)	Supplemental Groundwater <sup>5</sup> Tier #2 (inches)	Supplemental Groundwater Tier #2 (\$/AF)	Drought Surcharge <sup>6</sup> (\$/acre)
1988	Unlimited	-----	\$6.00	-----	-----	-----	-----	-----
1989	42"	-----	\$6.50	Over 42"	\$14.00	-----	-----	-----
1990	42"	-----	\$7.00	18"	\$3.50	-----	-----	-----
1991	33"	-----	\$7.50	12"	\$7.50	-----	-----	-----
1992	33"	-----	\$7.75	12"	\$7.50	-----	-----	-----
1993	42"	-----	\$8.00	12"	\$4.00	-----	-----	-----
1994	36"	-----	\$8.50	12"	\$4.25	-----	-----	-----
1995	42"	-----	\$9.00	18"	\$6.50	-----	-----	-----
1996	42"	6"	\$9.50	24"	\$8.25	72" & up	\$19.00	-----
1997	42"	6"	\$10.10	24"	\$8.75	72" & up	\$20.00	-----
1998	36"	6"	\$11.10	30"	\$9.00	72" & up	\$20.00	-----
1999	42"	12"	\$12.20	18"	\$10.10	72" & up	\$20.00	-----
2000	42"	6"	\$13.40	24"	\$6.70	72" & up	\$20.00	-----
2001	42"	-----	\$13.90	30"	\$7.35	72" & up	\$20.00	-----
2002	42"	-----	\$13.90	30"	\$7.35	72" & up	\$20.00	-----
2003	39"	6"	\$15.30	30"	\$7.65	66" & up	\$20.00	-----
2004	42"	-----	\$17.00	30"	\$8.50	72" & up	\$20.00	-----
2005	42"	6"	\$18.70	24"	\$9.35	72" & up	\$20.00	-----
2006	42"	6"	\$20.50	24"	\$10.25	72" & up	\$20.00	-----
2007	36"	6"	\$21.50	36"	\$10.75	72" & up	\$20.00	-----
2008	36"	-----	\$23.50	36"	\$11.75	72" & up	\$20.00	-----
2009	36"	-----	\$25.50	6"	\$12.75	42" & up	\$20.00	-----
2010	42"	6"	\$27.00	12"	\$13.50	60" & up	\$20.00	-----
2011	48"	6"	\$27.00	12"	\$13.50	60" & up	\$20.00	-----
2012	36"	-----	\$29.50	6"	\$14.75	42" & up	\$30.00	-----
2013	36"	-----	\$29.50	12"	\$14.75	42" & up	\$30.00	-----
2014	24"	-----	\$32.50	-----	-----	-----	-----	\$11.91

Notes:

- <sup>1</sup> City of Modesto (Domestic 1995 forward) receives equivalent allotment. Allotments started in 1989, before then water was unlimited.
- <sup>2</sup> Additional available water to encourage groundwater recharge (soft cap) at no cost
- <sup>3</sup> Facilities and Maintenance charge ½ of base water charge (\$/acre)
- <sup>4</sup> Water used in excess of base allotment
- <sup>5</sup> Water pumped above excess water
- <sup>6</sup> Only applies to irrigated acreage



# MODESTO IRRIGATION DISTRICT

## Irrigation Rate Structure (Volumetric Pricing)

Updated By: Carrie L  
Date: 8/27/15

2015			
Category	Cost \$/Acre (AC) or \$/Acre Foot (AF)	Available Water <sup>2</sup> (inches)	Drought Surcharge <sup>3</sup> (\$/acre)
Fixed Charge <sup>1</sup>	\$40.00/AC	16"	\$16.00
Volumetric - Tier 1 (up to 24")	\$1.00/AF		
Volumetric - Tier 2 (24" to 36")	\$2.00/AF		
Volumetric - Tier 3 (36" to 42")	\$3.00/AF		
Volumetric - Tier 4 (42" and up)	\$10.00/AF		

**Notes:**

<sup>1</sup> Facilities and Maintenance charge ½ of fixed water charge (\$/acre)

<sup>2</sup> City of Modesto (Domestic) receives equivalent allotment

<sup>3</sup> Only applicable to irrigated acreage

**MODESTO IRRIGATION DISTRICT**

**2015 AGRICULTURAL WATER MANAGEMENT PLAN**

**APPENDIX E**

**DROUGHT MANAGEMENT PLAN**



## **Drought Management Plan**

**September 9, 2015**

### Background

Modesto Irrigation District (MID) was formed on July 9, 1887 as the second irrigation district to be established in California under the California Irrigation Districts Act (Wright Act). During its early years, MID (along with the Turlock Irrigation District) acquired numerous water rights including pre-1914 rights and constructed conveyance facilities to meet the needs of its customers (agriculture, electric and domestic). Despite our efforts to procure and maximize our water resources, there has been and will continue to be times when the quantity of surface water available to MID is insufficient to meet the full demands of our customers. Surface water shortages can occur for a variety of reasons due both to single, multiple and consecutive events which may include, but aren't limited to; hydrology, increases in river flows, environmental regulation, sustainable groundwater management, reservoir operations and curtailment of water rights. In those instances and consistent with the direction provided by the Governor of the State of California in his 2015 Drought Proclamation, MID has prepared a Drought Management Plan in an effort to mitigate such surface water shortages while striving to provide the highest level of service to our customers.

The information set-forth herein isn't the solution, but it is intended to be used as a guide for MID staff, Board of Directors and the customers that we serve during periods of surface water shortage within MID's irrigation boundary. MID staff and the Board of Directors reserve the right to amend or otherwise modify the Drought Management Plan from time to time as may be necessary.

### Guiding Principles

The guiding principles presented below are intended to illustrate the basic assumptions that were used to develop the plan. The guiding principles are as follows:

1. MID's obligation under California Water Code is to manage and deliver surface water resources under its charge in a reasonable and beneficial manner.
2. All lands within MID irrigation boundaries have an equal right to availability of surface water, irrespective of crop(s) grown.
3. MID's options for allocating water are limited due to available supply, forecasted run-off and pre-releases from City and County of San Francisco.
4. MID strives to provide the highest level of service to our irrigation customers.

## Level of Surface Water Shortage and MID's Response:

Surface water shortage level may be assigned once the determination of current and potential surface water supplies have been established and the allocation has been set for the irrigation season.

Surface Water Shortage Level	Allocation (inches per acre)
Level One	36" --> 42"
Level Two	24" --> 36"
Level Three	< 24"

### Level One – MID allocation is 36" to 42" per acre

As soon as the shortage is known or discovered, MID may take any or all of the following actions:

- a. No run-off from on-farm irrigation
- b. Increased operation of well field in strategic locations to avoid conveyance losses
- c. Incentivize growers to remove/replant aging orchards for summer fallow
- d. Capped allotment
- e. No groundwater recharge component
- f. No out-of-district water will be provided
- g. No delivery of water to ponds not used for agricultural purposes
- h. No construction water will be provided
- i. Increased public outreach/grower meetings/education

### Level Two – MID allocation is 24" to 36" per acre

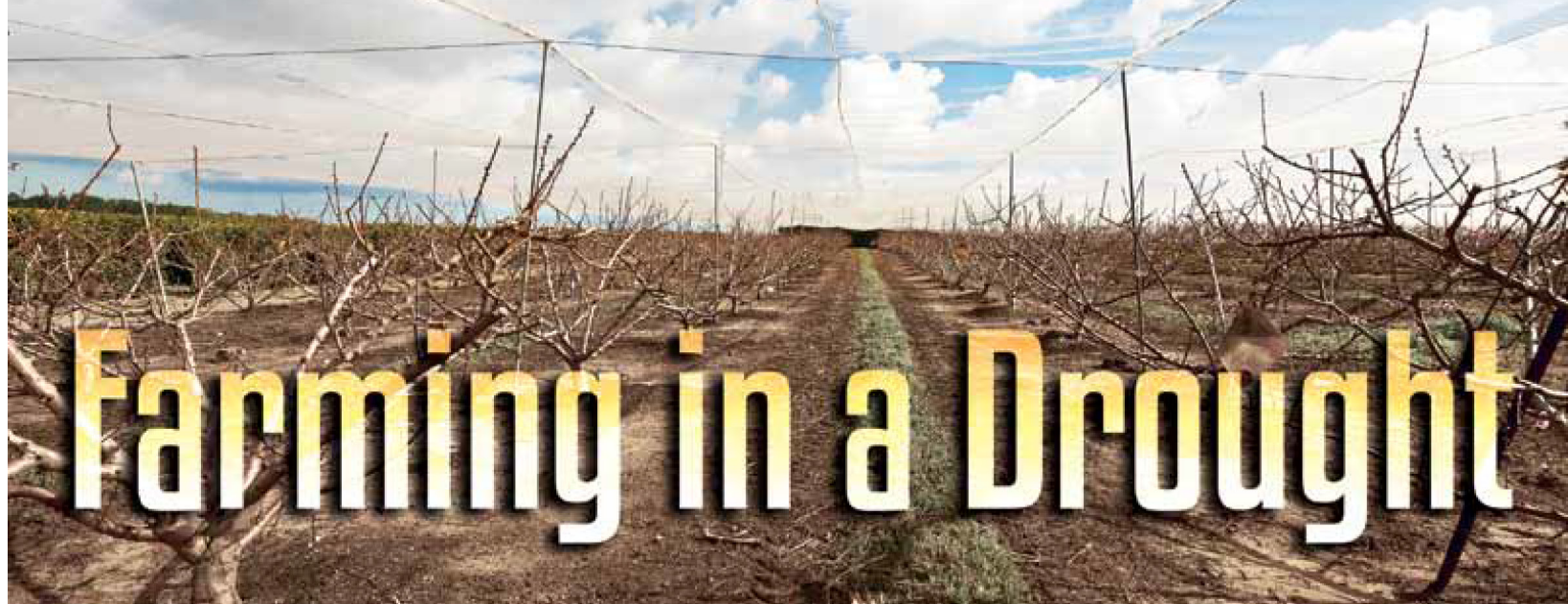
As soon as the shortage is known or discovered, MID may take any or all of the actions included in Level One in addition to any or all of the following actions:

- a. Maximize use of well field
- b. Implement Farmer to Farmer Delivery Program
- c. Implement Water Management Alternative Program
- d. Implement dry year rate schedule
- e. Operate Modesto Reservoir at a lower mean operating level to decrease seepage and evaporation losses

### Level Three – The District allocation is 24" or less

As soon as the shortage is known or discovered, MID may take any or all of the actions included in Level One and Two in addition to any or all of the following actions:

- a. Implement Allocation Return Program
- b. Implement Supply Augmentation Program



Sponsored by



## **AGENDA**

6:00 - 6:10	Welcome TID and MID
6:10 - 6:25	Milton O'Hare Stanislaus County Agriculture Commissioner
6:25 - 7:00	Roger Duncan Pomology and Viticulture Advisor University of California Cooperative Extension
7:00 - 7:15	Diana Waller District Conservationist USDA National Resources Conservation Service
7:15 - 8:00	Questions and Answers





**WATER & POWER**  
Serving Central California since 1887



**Modesto  
Irrigation  
District**

# Farming in a Drought

TID and MID invite you to learn how the drought is affecting Stanislaus County agriculture and hear about effective deficit irrigation strategies from industry experts.

Panelists will offer suggestions for coping with dry conditions as well as technical and financial assistance opportunities.

**Please join us for this free informational presentation:**

**> Thursday, May 29, 2014  
6:00 p.m. to 8:00 p.m.  
Harvest Hall  
3800 Cornucopia Way, Modesto**

**MODESTO IRRIGATION DISTRICT**

**2015 AGRICULTURAL WATER MANAGEMENT PLAN**

**APPENDIX F**

**GROUNDWATER MANAGEMENT PLAN –  
EXECUTIVE SUMMARY**

# Executive Summary

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## Introduction

The Modesto Groundwater Subbasin lies between the Stanislaus River on the north and the Tuolumne River on the south and between the San Joaquin River on the west and crystalline basement rock of the Sierra Nevada foothills on the east. The surface area of the subbasin is 247,000 acres.

The northern, western, and southern boundaries are shared with the Eastern San Joaquin, Delta-Mendota, and Turlock Groundwater Subbasins, respectively. The major water purveyors in the planning area include the Modesto Irrigation District (MID), the Oakdale Irrigation District (OID), and the Cities of Modesto, Riverbank, and Oakdale.

In April 1994, the five water purveyors were joined by a sixth agency, Stanislaus County, to form the Stanislaus and Tuolumne Rivers Groundwater Basin Association (Association). The Association provides a forum for the coordinated planning and management of the Modesto Groundwater Subbasin and encourages the development of projects and programs that will improve water supply reliability and water quality within the subbasin. Figure ES-1, a map of the subbasin, shows the boundaries of the six agencies.

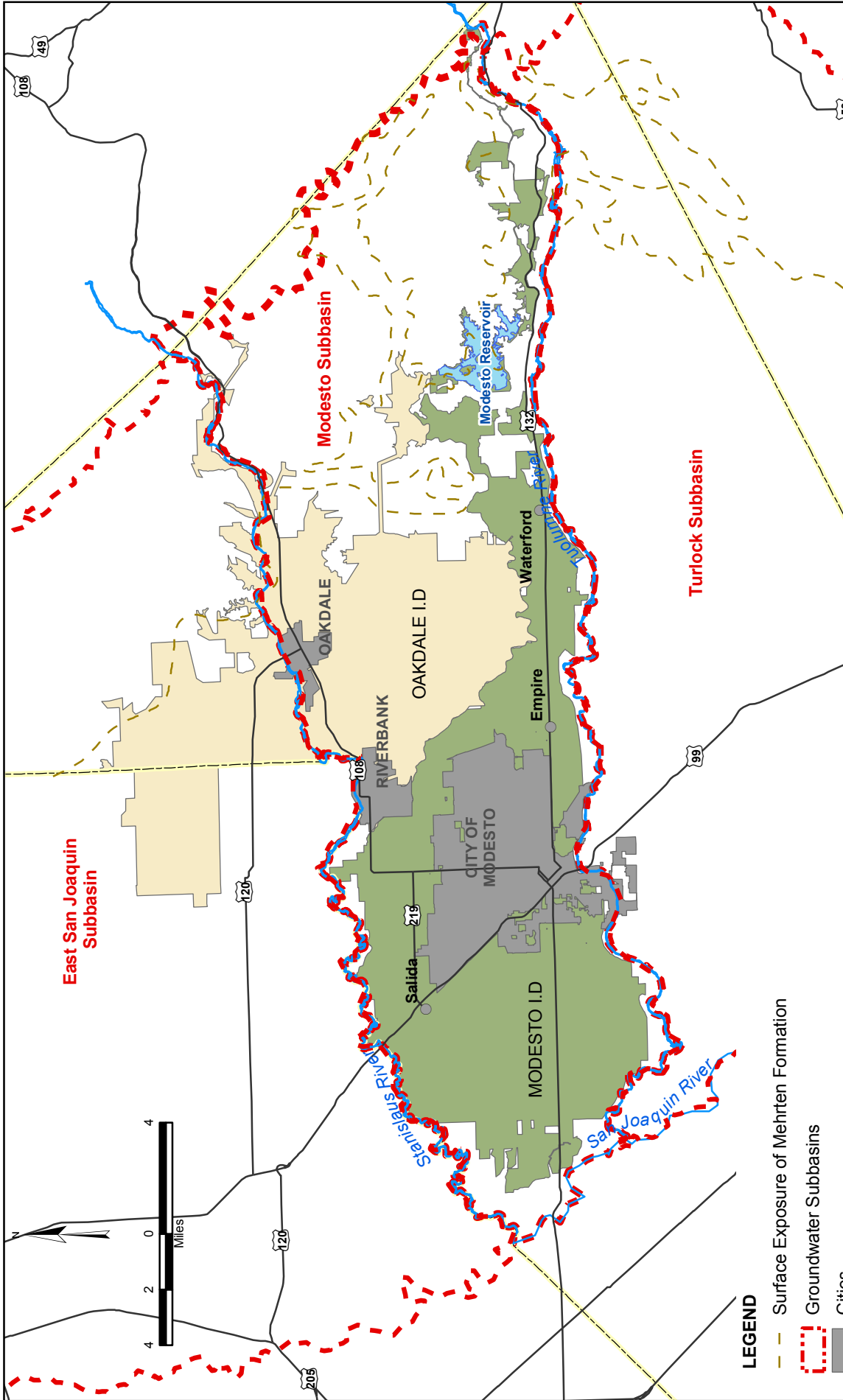
Since its formation, the Association has been actively engaged in the management of the subbasin. The Association provides its members a vehicle for coordinated planning to make the best use of groundwater and to satisfy the mutual interests of the member agencies.

Specific purposes of the Association are to:

- Determine and evaluate the subbasin's groundwater supply
- Promote the coordination of groundwater management planning
- Develop a hydrologic groundwater model of the groundwater basin
- Determine the subbasin's need for additional or improved water extraction, storage, delivery, conservation, and recharge facilities
- Provide information and guidance for the management, preservation, protection, and enhancement of groundwater quality and quantity in the subbasin

In late 2003, the Association began developing an Integrated Regional Groundwater Management Plan (IRGMP) in compliance with the Groundwater Management Planning Act of 2002 (SB 1938) and the Integrated Regional Water Management Planning Act of 2002





**LEGEND**

- Surface Exposure of Mehrten Formation
- Groundwater Subbasins
- Cities
- Modesto I.D.
- Oakdale I.D.

SOURCES: City of Modesto, Modesto Irrigation District, City of Oakdale, Oakdale Irrigation District, City of Riverbank, CA Dept of Water Resources Groundwater Basins, 2002, California Spatial Library.



**INTEGRATED REGIONAL GROUNDWATER  
MANAGEMENT PLAN FOR THE MODESTO BASIN  
Management Areas and Agencies**

JUNE 2005

FIGURE ES-1

(SB 1672). Throughout the planning process, other interested parties within the subbasin as well as state agencies have been encouraged to participate in the plan's development.

## Planning Area

Developed land uses within the Modesto Groundwater Subbasin are concentrated in two major categories: irrigated agricultural and urban land uses. The largest jurisdiction within the subbasin is MID with a service area of 101,700 acres and an irrigated area of approximately 62,000 acres. Nested within MID are the communities of Waterford, Empire, and Salida and parts of Del Rio and Riverbank. Also lying largely within MID is the city of Modesto, which occupies approximately 40 square miles or 25,600 acres. Modesto is in the southwestern portion of the subbasin, and a portion of the city is located south of the Tuolumne River in the Turlock Groundwater Subbasin.

The cities of Oakdale and Riverbank lie in the north-central portion of the subbasin. The southern 60 percent of OID is in the Modesto Groundwater Subbasin and the remaining 40 percent is in the Eastern San Joaquin Groundwater Subbasin.

The Modesto Groundwater Subbasin underlies all of MID, the City of Oakdale, and the City of Riverbank. However, a portion of OID overlies the Eastern San Joaquin Groundwater Subbasin, and a portion of the City of Modesto service area overlies the Turlock Groundwater Subbasin.

Because OID's jurisdictional boundaries reach beyond the boundaries of the Modesto Groundwater Subbasin, the study area has been extended to include OID's complete jurisdiction. A similar water planning effort is under way in the Turlock Groundwater Subbasin, and the portion of the City of Modesto service area within the Turlock Subbasin is covered in the Turlock groundwater planning process.

The entire subbasin and planning area lies within Stanislaus County.

## Description of the IRGMP

This IRGMP has been prepared in accordance with requirements of SB 1672 (California Water Code Section 10540 *et seq.*) and SB 1938 (California Water Code Section 10750 *et seq.*). As such, the plan includes components of AB 3030, SB 1938, and SB 1672.

The purpose of this IRGMP is to provide a framework for coordinating groundwater and surface water management activities into a cohesive set of management objectives and for implementing the actions necessary to meet those objectives.

The goal of the IRGMP is to integrate the use of groundwater and surface water within the Modesto subbasin to ensure the reliability of a long-term water supply that will meet current and future beneficial uses including agricultural, industrial, and municipal water

requirements while protecting the environment. Attaining this goal requires measures that enable the efficient use of groundwater and surface water and measures that protect water quality.

The overriding objective of the IRGMP is to improve the regional and local management of water resources through the formulation and implementation of Basin Management Objectives (BMOs).

## **Regional Priorities**

The IRGMP recognizes that the most effective approach to managing a basin's water resources is enlisting the cooperation of the agencies whose political boundaries match the basin's physical boundaries. For this reason, the IRWMP frames specific water management projects in the context of an integrated regional strategy. Although the plan emphasizes groundwater management, elements of the plan address the use of surface water supplies, water conservation, and water recycling and blending to meet demands that have previously been met with groundwater. This integration of surface water and groundwater resources leads to a more comprehensive management of water supplies and provides a lucid framework for complying with state and federal water quality standards. The primary regional objective is the preservation and protection of the basin's water resources for the benefit of inhabitants of the region. Specific regional objectives include:

- Improve local water supply reliability
- Protect the groundwater resources of the region
- Improve water quality
- Foster prudent stewardship of water resources
- Facilitate compliance with local, state, and federal water quality and public health regulations.

## **Local Priorities**

In addition to the statewide and regional priorities, the IRGMP addresses local issues by presenting BMOs that have been developed to meet the particular management needs of each of the participating agencies. Local BMOs are specific approaches to water management goals including groundwater supply, groundwater quality, and protection against inelastic land surface subsidence. Because they are presented within the context of a basin-wide plan, the local BMOs illustrate the degree to which many BMOs are common to more than one of the participating agencies. This suggests that in certain instances, implementation of local BMOs may best be achieved through cooperation among participating agencies. The most

prominent of the local priorities is protection of groundwater quality through monitoring and control of contaminant plumes.

## Statewide Priorities

Implementing the IRGMP will enable the Association and its member agencies to respond to a range of statewide water management initiatives. Key among these is the increasing emphasis placed on developing integrated regional solutions to water management problems and coordinating the conjunctive management of surface water and groundwater to improve water supply reliability and water quality.

In particular, by promoting effective water use in the Modesto Groundwater Subbasin, the implementation of the IRGMP will:

- Increase California's water supply reliability
- Reduce conflicts among water users
- Contribute to meeting Delta water quality objectives
- Assist in the implementation of Regional Water Quality Control Board Watershed Management Initiatives chapters, plans, and policies

## Regional BMOs

Specific water management strategies developed during the formulation of the IRGMP are expressed by the regional BMOs agreed upon by all of the participating agencies. The following specific regional BMOs are presented in the IRGMP:

- **Identification of Natural Recharge Areas:** Groundwater recharge has diminished because the expansion of urban areas and trends in agricultural irrigation practices have reduced the deep percolation of applied water. These trends underscore the need to identify and protect remaining natural recharge areas.
- **Development of a Basin-Wide Water Budget:** A basin-wide water budget will describe the pathways by which water enters and leaves the basin. This budget will offer a tool for comparing inflows, outflows, and changes in storage under historical and present conditions with flows and changes in storage that may exist after the implementation of specific BMOs.
- **Feasibility Evaluation of Artificial Recharge Projects:** The basin-wide water balance will reveal whether the basin is in overdraft and will illustrate trends in groundwater recharge and groundwater use. If the water balance demonstrates either that the basin is in overdraft or is likely to fall into overdraft in the near future, artificial recharge basins may be needed to supplement recharge from natural recharge areas.

- **Management and Optimization of Well Field Operation:** A component of improved groundwater management is the optimization of well operations to accomplish specified management objectives. For example, each well in a well field can be instrumented and controlled so that a group of wells can be operated to meet single- or multiple-objective functions.

In addition, well field optimization can support water quality objectives by reducing agricultural outflows to streams and by blending groundwater with surface deliveries. For example, agencies within the basin could evaluate an expansion of the blending program in order to control shallow groundwater and improve downstream water quality.

- **Identification and Feasibility Study of Conjunctive Use Projects:** Many of the management actions described above can be viewed as components of a broader conjunctive management program whose goal is an integrated approach that balances surface water and groundwater use. Implementation of a conjunctive management strategy may involve reduced groundwater pumping in some parts of the basin and broad controls on pumping to meet target groundwater levels. An important regional conjunctive use initiative is the Modesto Regional Water Treatment Plan, which has reduced demand for groundwater by storing and treating surface water. Because of its success, this project is being expanded.
- **Support of Public Health Programs:** Well construction and demolition standards are designed specifically to protect groundwater quality. Management actions to assist local agencies in complying with public health standards include the following components:
  - Installation of sanitary well seals on all new wells in accordance with the California Well Standards
  - Abandonment of wells in accordance with the California Well Standards

These actions will be particularly valuable in unincorporated areas not served by a water purveyor.

- **Water Quality Management:** The protection of groundwater quality is of increasing concern because the basin's population is growing. This management action would include a detailed geologic assessment of the basin that would focus on the areas with poor water quality and identify the sources of the contaminants. This assessment would result in coverage on a GIS system for mapping recharge areas and would be used to develop strategies to control the migration and movement of poor quality water into and throughout the basin.

- **Groundwater Monitoring and Subsidence Monitoring Program:** Groundwater monitoring and analysis and the archiving of collected data will be needed to implement several of the recommended management actions (e.g., conjunctive management and optimized operation of well fields) and to meet the reporting requirements of the plan. The Association is developing a database to facilitate the storage, retrieval, and archiving of groundwater data. Monitoring data will be important in the development and calibration of the basin-wide groundwater model that will be used to evaluate the effects of proposed projects and management actions.

The Association plans to monitor and measure the rate of inelastic land surface subsidence within the basin. Given the ongoing efforts by Association members to prevent groundwater overdraft and conditions that might lead to subsidence, it appears unlikely that the insignificant subsidence that has occurred historically within the basin will be accelerated. However, the Association plans to monitor and document any future changes in land surface elevations and, if inelastic subsidence is observed, may recommend necessary actions.

- **Policy Assessment:** Several of the technical management actions introduced above have clear policy requirements and implications. For example, effective protection of natural recharge areas will require coordination and communication with entities responsible for land use policies. Similarly, annexations to expand agencies' service areas as part of an in-lieu recharge program presume clear policies regarding annexation and a process to evaluate the impacts of annexation on groundwater levels and groundwater quality.

The development of consistent policies would be assisted by a regional groundwater forum such as the Association. The Association could promote interagency relationships that would foster coordination and cooperation among participating agencies to manage the Modesto Groundwater Subbasin and would provide a framework for the formulation of regional projects and programs for the protection and use of the subbasin's water resources.

For example, given the mutual concern of agencies within the basin regarding preserving natural recharge areas and protecting these areas from pollutants, local agencies could work together to inform one another about land use practices that may contribute to groundwater degradation and the importance of reducing the occurrence of these land use practices.

- **Promoting Cooperation and Coordination Between Water Entities:** The Association will continue to coordinate water management activities within the basin and to work cooperatively for the implementation of agreed-upon BMOs. It will also develop an outreach and educational program to engage other water interests in the

management of the basin. One example of such outreach will be working cooperatively with industrial water users to improve water levels and water quality in the basin and to reduce localized well interference.

## Water Management Strategies

The regional BMOs described above have been developed to support a comprehensive approach to managing water resources in the Modesto Groundwater Basin. In particular, these BMOs provide a framework for developing projects that will advance the following water management strategies:

- **Increase Local and Regional Water Supply Reliability and Water Use Efficiency:** BMOs supporting conjunctive management, policy assessment, and development of a basin-wide water budget will be key to the implementation of this strategy.
- **Promote Groundwater Recharge and Management:** BMOs encouraging the identification of natural recharge areas and the evaluation of artificial recharge areas will be used to implement this strategy.
- **Support Water Conservation:** Development of a basin-wide water budget will be used to identify water conservation opportunities, and the management and optimization of well field operations will be used to reduce spillage from irrigation distribution systems.
- **Implement Watershed Management Programs:** This strategy will be implemented through policy assessment, identification of natural recharge areas and evaluation of artificial recharge projects.
- **Promote Water Recycling:** Management and optimization of well field operations, groundwater monitoring, and development of artificial recharge projects offer opportunities for the management and use of recycled water generated by municipalities and industries in the planning area.
- **Foster Conjunctive Use:** The BMO dedicated to the identification and study of conjunctive use projects focuses on developing conjunctive management in the Modesto Groundwater Subbasin. Other BMOs addressing natural and artificial recharge, groundwater monitoring, well field optimization, and policy assessment will also contribute to planning and implementation of conjunctive use.
- **Improve Water Quality:** The water quality management BMO, groundwater monitoring, and the management and optimization of well field operations will all be important BMOs for improving water quality.

- **Improve Storm Water Capture and Management:** BMOs that support public health programs and that call for capturing storm water in dry wells and in natural and artificial recharge facilities will reduce storm water discharges.

Other regional water management elements such as provisions for recreation and environmental and habitat protection are addressed in other planning documents prepared by the participating agencies.

## Public Involvement

The six agencies forming the Association share groundwater and surface water resources and worked together to formulate this management plan. Throughout this planning process, other interested agencies and entities within the subbasin were encouraged to participate. The Association will work with its member agencies and other entities to implement the components of this plan. The County of Stanislaus, as a member of the Association, represented other self-supplied groundwater producers. An extensive public involvement process was also followed during the IRGMP's development to enable stakeholder participation in the planning process.

In addition to public stakeholders, key local, state, and federal government agencies have contributed to the IRGMP. In mid-2004, the Association engaged in discussions with the Department of Water Resources to initiate a cooperative relationship for the conjunctive management of the basin. As a result of these discussions, the Association and the Department of Water Resources signed a Memorandum of Understanding to work together to develop conjunctive use projects.

For the last several years, the Association has been working cooperatively with the U.S. Geological Survey to study the geology and aquifers of the Modesto Groundwater Subbasin. The Association and the U.S. Geological Survey have entered into an agreement, under the National Water-Quality Assessment Program, to map the subsurface geology of the basin and to develop a data network and three-dimensional model of the basin.

The Association's member cities are also working with the Department of Health Services on issues related to compliance with Title 22, Drinking Water Quality Standards.

## Plan Implementation

A key feature of the IRGMP implementation is the establishment of linkages among program actions. These linkages transform individual implementation activities into a coherent program where the whole is greater than the sum of the parts with respect to achieving regional water management objectives.

Implementation of the actions recommended in the IRGMP is scheduled in three phases:



- **Phase I—Near Term Projects:** These projects are intended to be implemented within the next three years and include:
  - Management of the well fields: A decision support system to assist the districts to optimize groundwater production from their well fields, based on a set of established objectives
  - Additional water blending projects: To help agencies meet their water quality objectives while increasing the beneficial use of groundwater
  - Water conservation projects, including agricultural and urban water conservation projects
  - Identification of conjunctive use project concepts
  - Increase treatment capacity for the City of Modesto
  - Development of a three-dimensional groundwater model
- **Phase II—Mid-Term Projects:** These projects are planned for implementation in four to seven years:
  - Identification of groundwater recharge areas
  - Rock well monitoring
  - Development of conjunctive use projects
  - Development of the in-lieu recharge projects, including evaluation of annexation options to reduce groundwater pumping
  - Development of a basin-wide database
- **Phase III—Long-Term Projects:** These projects are scheduled for implementation beyond seven years in the future and include:
  - Installation of subsidence monitoring station if needed
  - Water exchange program
  - Update water budget
  - Feasibility evaluation of artificial recharge projects

Other water management actions may continue throughout the planning horizon, including:

- Monthly Association meetings
- Preparation of annual progress reports
- Groundwater monitoring and data sharing
- Coordination and cooperation with water entities, neighboring basins, and state and federal agencies
- Periodic review of groundwater monitoring and groundwater management

Progress toward the implementation of the IRGMP is contingent upon securing funding to complete the program. Two available avenues are grant funding and funds generated internally by the Association members.

**MODESTO IRRIGATION DISTRICT**

**2015 AGRICULTURAL WATER MANAGEMENT PLAN**

**APPENDIX G**

**CONSERVATION PROGRAM GUIDELINES**

# MODESTO IRRIGATION DISTRICT

## *CONSERVATION PROGRAM GUIDELINES*

AUGUST 2015



TABLE OF CONTENTS

**INTRODUCTION..... 1**  
**ELIGIBLE PROJECTS ..... 1**  
**ELIGIBILITY ..... 3**  
**AVAILABLE FUNDING ..... 3**  
**APPLICATION PROCESS AND PAYMENT PROCEDURES ..... 3**  
**PROJECT RANKING CRITERIA..... 4**  
**CONTRACTUAL OBLIGATIONS ..... 4**  
**ANTICIPATED ANNUAL SCHEDULE ..... 5**

**Attachments**

A - Suggested Design Requirements

B - Landowner Application for Conservation Program Funding

## **INTRODUCTION**

### Background

The Modesto Irrigation District (MID or District) Conservation Program (Program) provides partial funding to qualifying MID landowners for projects that conserve water and improve water management after the eligible project is completed. Projects must meet certain eligibility criteria and be pre-approved by MID. These guidelines provide information on eligible projects, applicant eligibility, available funding, the application process, project ranking criteria, contractual obligations, and the anticipated annual schedule. The Program will be an annual program that is subject to funding and approval by the Board of Directors (Board) on an annual basis.

### Objectives

The objective of the Program is to encourage landowners, through financial incentives, to invest in physical improvements and management practices that conserve water and improve water management. The long-term goal of the Conservation Program is to improve water management within the District.

### Role of Modesto Irrigation District

The role of MID in the Program includes, but isn't limited to:

- Review of applications for conformance with guidelines
- Ranking of applications and selection of projects for funding
- Review adequacy of project designs
- Recommendation and approval of contractors, consultants and equipment suppliers
- Monitoring during construction
- Performing final inspection
- Making reimbursement payment in accordance with funding agreement after completion of work and approval by MID
- Verification of project performance one-year after construction

If you have any questions about the Program please contact the MID Irrigation Operations office at (209) 526-7373.

## **ELIGIBLE PROJECTS**

### Physical Improvements

Physical improvements that are eligible for funding include:

- New pipelines, sidegates, control gates and control boxes from District conveyance system to pump intake of private landowner facilities (components related to conversion from flood irrigation to pressurized irrigation system)

- Replacement of leaky cast-in-place concrete pipelines
- Conversion of canals to pipelines
- Flow measurement
- Land leveling for improved flood irrigation
- Unlined regulating reservoirs in areas where groundwater recharge is desired and practical

### Management Practices

Management practices that are eligible for funding include:

- Scientific irrigation scheduling (using approved consultant)
- Soil moisture monitoring (using approved equipment manufacturer)

### Landowner Proposed Projects

Landowners may propose other conservation measures that are not listed above. These conservation measures must have quantifiable benefit in terms of water conservation or water management. MID will review applications for other proposed conservation measures and determine eligibility for funding on a case-by-case basis.

### Ineligible Projects

Most on-farm irrigation system improvements aren't eligible for District funding as these projects may be funded by other programs such as the Natural Resources Conservation Service (NRCS) Environmental Quality Incentives Program (EQIP).

Projects not eligible for funding under the Program include the following:

- Irrigation system components for drip, micro-spray, sprinkler or flood irrigation. These include, but are not limited to: power source, electrical devices, chemical injection tanks and equipment, on-farm irrigation control valves, prescreening (i.e. trashracks), pumps, pump intake piping, piping after pumps, filters, distribution system, emitters, dripline or drip tape
- Engineering design
- Easements
- Land acquisition
- Deep ripping/tillage
- Components related to a conversion from sprinkler to drip irrigation
- Repairs to, or modification of, existing irrigation system not listed herein or under eligible physical improvements
- Pipe relocations and/or improvements to facilitate development or for more efficient on-farm practices.

## ELIGIBILITY

Applications can be submitted by an individual landowner or a group of landowners (i.e., Improvement District). Applicants must satisfy the following eligibility criteria to be considered for funding:

- Must have control of the land as a landowner (lessees cannot apply)
- Landowner must have an active irrigation account with MID in good standing
- Must be in good standing with the Irrigated Lands Regulatory Program
- Eligible lands must be entirely or partially within MID
- Benefitting land must have irrigation history in at least 2 of the last 5 years

## AVAILABLE FUNDING

The total amount of funding available for the Program will vary each year based on MID’s approved budget. The table below shows the anticipated project funding that may be available annually.

**Table 1 – Available Funding**

Fundable Projects	Percent Funded	Maximum Funding <sup>1</sup>
Physical Improvements	50%	\$60,000
Management Practices	50%	\$5,000

<sup>1</sup> Maximum funding may vary based on Board approval

If a project serves property that is only partially located within MID’s irrigation boundary, then project funding will be proportionate to the acreage within MID’s irrigation boundaries. For example, if 80% of the acreage, as determined by MID, is within MID’s irrigation boundary, then only 80% of the total project cost would be eligible for 50% funding.

## APPLICATION PROCESS AND PAYMENT PROCEDURES

The application and payment procedures are summarized as follows:

1. Landowner submits application package for review (including attached application form, design plans and information, cost estimate, contractor/consultant/manufacture information, 5 year average water use per acre, and estimate of water savings and operational benefits)
2. MID considers project for funding based on ranking all submitted applications that year



3. MID prepares reimbursement agreement if project is approved
4. Landowner executes agreement
5. MID monitors during construction
6. Landowner completes project
7. MID performs final inspection
8. Landowner submits final pay request, record drawings, and itemized invoices for qualifying project expenses
9. MID makes reimbursement payment in accordance with agreement
10. MID may perform post-project monitoring 1 year after construction to verify performance as intended

## **PROJECT RANKING CRITERIA**

MID Irrigation Operations staff will rank projects based on the information submitted and projects will be funded up to the amount allocated by the Board for that year. Selection won't be on a first come – first served basis. Rather, all projects submitted by the annual deadline will be considered. Preference will be given to projects that meet the following criteria:

1. High water conservation value relative to the project cost (cost/acre-foot conserved)
2. Projects benefitting multiple landowners versus a single landowner
3. Applicants that have historically high water use as determined by MID
4. Applicants that haven't participated before in the Program
5. Replacement of existing infrastructure versus installation of new infrastructure
6. Lands entirely within versus partially within MID's irrigation boundary

## **CONTRACTUAL OBLIGATIONS**

### General Obligations

- Landowner must agree to maintain an active irrigation account for at least 5 years following project completion
- Minimum field size of 10 acres for overall project (smaller fields considered on a case-by-case basis)
- MID must pre-approve consultants, contractors and equipment manufacturers
- Project design shall be reviewed by MID prior to construction, and when relevant, prepared by a California Registered Civil Engineer or certified by an Irrigation Association Certified Irrigation Designer
- All projects shall adhere to the attached Conservation Program Design Requirements as applicable.
- Design and construction must meet applicable District standards in accordance with current MID Irrigation Rules and Regulations (Section 2.6)

- MID shall inspect project during construction to ensure conformance with current MID Irrigation Rules and Regulations

Flow Measurement Requirements

- Flow measurement is required for all conveyance projects
- Flowmeters must be Seametrics AG2000 Irrigation Magmeter or McCrometer Mag 3000. Meter with 4-20mA output is required.
- Landowner agrees to provide District permanent access to flow measurement device
- MID shall have the right to install telemetry in the future, if desired
- Landowner agrees to repair, modify, calibrate or replace flow measurement device to ensure accuracy in accordance with MID Irrigation Rules and Regulation then in effect as required by the District. District shall also have the right to repair, modify, calibrate or replace flow measurement device at landowners expense.

**ANTICIPATED ANNUAL SCHEDULE**

The anticipated annual schedule for implementing the Program is shown below. The schedule may vary from year to year based on available funding and the availability of District staff to administer the Program.

**Table 2 – Anticipated Annual Schedule**

Description	Date
Applications Released	May 1
Applications Due	August 1
Project Rankings Released	September 1
Deadline for Submitting for Reimbursement	March 1 of following year

*ATTACHMENT A*

SUGGESTED DESIGN REQUIREMENTS

# **MODESTO IRRIGATION DISTRICT CONSERVATION PROGRAM SUGGESTED DESIGN REQUIREMENTS**

*FOR USE BY SYSTEM DESIGNERS OR ENGINEERS*

## **A. General Project Requirements**

1. See Design and Construction of Private and Improvement District Facilities (Section 2.6) of MID Rules and Regulations Governing the Distribution of Irrigation Water with the MID.

## **B. Project Drawing Requirements**

1. Scalable drawing with a scale not to exceed 1" = 60'.
2. General project vicinity map, north arrow, and legend.
3. Minimum paper size of 11" X 17" for irrigation projects having a pipe length greater than 500 feet.
4. Plan view depicting existing roads, property lines, MID irrigation facilities, and Improvement District facilities, as each facility is known by the public or District.
5. All proposed project facilities to be labeled based on type, size, and distances from existing facilities if applicable.
6. Legible copies of MID original standard engineering details related to project shall be incorporated into plan view drawing or additional drawing pages as needed. MID engineering standard detail number to be referenced on plan view drawing(s) at all locations where the MID engineering standard detail is utilized.
7. For irrigation pipelines having a length greater than 500 feet the project designer must provide a profile drawing view of pipeline and related irrigation facilities.

## **C. Project Design Requirements (To be provided on drawing)**

1. Water elevation datum with respect to existing District irrigation facility as determined by project designer.
2. Total required flow (15 cfs minimum required for flood irrigation systems only).
3. For pressurized systems determine maximum flow, time, duration, and frequency of irrigation events during maximum crop evapotranspiration.
4. Provide minimum pipeline cover of two (2) feet at all times
5. Minimum Polyvinyl chloride pipe rating of 100 psi.
6. For all pipelines having a length greater than 500 feet, the project design shall graphically show the static water line from the existing District distribution facility, as determined by the designer.
7. For all pipelines having a length greater than 500 feet, the project designer shall graphically show the hydraulic grade line based on the maximum required design flow with respect to the water elevation datum from the existing District distribution facility, as determined by the designer.

*ATTACHMENT B*

LANDOWNER APPLICATION FOR  
CONSERVATION PROGRAM FUNDING



For District Use Only  
Date Received: \_\_\_\_\_

## LANDOWNER APPLICATION FOR CONSERVATION PROGRAM FUNDING

(applications due by August 1 of each year)

### Instructions

The Modesto Irrigation District (MID) Conservation Program provides partial funding to MID landowners for projects that conserve water and improve water management after the eligible project is completed. Projects must meet certain eligibility criteria and be pre-approved by MID. Please carefully read the MID Conservation Program Guidelines before submitting an application. Application must be submitted by the deadline to be considered for funding. All applications will be reviewed and ranked by MID. Funding is not guaranteed for all applications. If you have any questions about the Conservation Program please contact the MID Irrigation Operations office at (209) 526-7373.

### General Information

Landowner Name: \_\_\_\_\_

Farm Name (if applicable): \_\_\_\_\_

Email: \_\_\_\_\_

Mailing Address: \_\_\_\_\_

Telephone: \_\_\_\_\_

Assessor's Parcel Number(s) (APN): \_\_\_\_\_

Design Engineer: \_\_\_\_\_

Contractor(s): \_\_\_\_\_

MID Delivery Location (Lateral, Sidegate or Turnout No., Improvement District, etc.):  
\_\_\_\_\_

MID Customer ID: \_\_\_\_\_

Parcel Size (acres): \_\_\_\_\_

Crop: \_\_\_\_\_

Future Design Flow: \_\_\_\_\_

Future Irrigation Schedule: \_\_\_\_\_

Have you received MID Conservation Program Funding for any projects in the past 5 years?

Yes     No

Have you applied for funding for these conservation measures, or a portion of related conservation measures, under any other program, such as NRCS EQIP?

Yes     No

If yes, what portion of project? \_\_\_\_\_

### Eligibility

- Must have control of the land as a landowner (lessees cannot apply)
- Must have an active irrigation account with MID in good standing

- Must be in good standing with the Irrigated Lands Regulatory Program
- Eligible lands must be entirely or partially within MID
- Benefitting land must have irrigation history in at least 2 of the last 5 years
- Minimum field size of 10 acres for overall project (smaller fields considered on a case-by-case basis)
- Project must be on approved list of eligible project types (see Conservation Program Guidelines)

**Proposed Physical Improvements (Check all that apply)**

- New pipeline, sidegate, control gates and/or control boxes from District conveyance system to pump intake of private landowner facilities (components related to conversion from flood irrigation to pressurized irrigation system)
  - Conversion from flood to drip/micro system
  - Replacement of leaky cast-in-place concrete pipeline
  - Conversion of canal to pipeline
  - Flow measurement
  - Land leveling for improved flood irrigation
  - Unlined regulation reservoir
  - Other (provide brief description below, see section titled "Other Conservation Measures")
- 

**Proposed Management Improvements (Check all that apply)**

- Scientific irrigation scheduling
  - Soil moisture monitoring
  - Other (provide brief description below, see section titled "Other Conservation Measures")
- 

**Other Conservation Measures**

Modesto Irrigation District will consider other conservation measures that can result in water conservation or improved water management. If you are proposing a conservation measure that isn't listed above then please attach the following:

1. Description of conservation measures to be implemented, including physical changes to the field and/or irrigation management changes
2. Sketch showing field and project location, and physical changes to the field
3. Description of how the proposed conservation measure will result in water conservation or better water management

**Other Relevant Notes Regarding Project** (please add any other relevant information below)

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**5-Year Avg. Water Use:** \_\_\_\_\_ AF/AC

**Estimated Water Savings:** \_\_\_\_\_ AF/AC

**Identified Operational Benefits:** \_\_\_\_\_  
\_\_\_\_\_  
\_\_\_\_\_

**Requested Funding<sup>1</sup>**

Total Project Cost: \$ \_\_\_\_\_

Total Requested Funding \$ \_\_\_\_\_

<sup>1</sup>MID will fund up to 50% of project costs with normal maximum funding of \$60,000 for physical improvements and \$5,000 for water management practices

**Attachments**

Please attach the following to your application:

1. Design drawings (for physical improvements)
2. Cost estimate (be sure to separate eligible and ineligible costs)
3. Calculation of estimated water savings and operational benefits
4. Information on proposed irrigation consultant (scientific irrigation scheduling)
5. Information on proposed contractors
6. Information on equipment manufacturer (flowmeters and soil moisture monitoring)

\_\_\_\_\_  
Please Print Name

\_\_\_\_\_  
Landowner Signature

\_\_\_\_\_  
Date

Please Submit Application to:

Modesto Irrigation District  
Irrigation Operations Division  
P.O. Box 4060  
Modesto, CA 95352-4060

*FOR OFFICE USE ONLY*

Received by Civil Engineering Dept.	Initials: _____	Date: _____
Application Deemed Complete:	Initials: _____	Date: _____
Approved by Irrigation Field Services Manager:	Initials: _____	Date: _____
Approved by Irrigation Operations Manager:	Initials: _____	Date: _____



**MODESTO IRRIGATION DISTRICT**

**2015 AGRICULTURAL WATER MANAGEMENT PLAN**

**APPENDIX H**

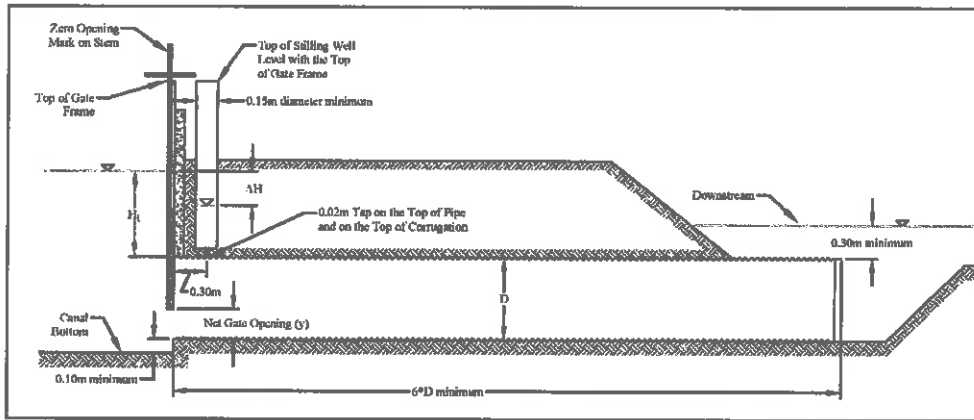
**ITRC WATER MEASUREMENT REPORTS**

- **IMPROVING FLOW MEASUREMENT ACCURACY AT FARM DELIVERY GATES IN CALIFORNIA – JULY 2015**
- **FLOW MEASUREMENT OPTIONS FOR CANAL TURNOUTS – DECEMBER 2014**
- **SBx7 FLOW RATE MEASUREMENT COMPLIANCE FOR AGRICULTURAL IRRIGATION DISTRICTS – AUGUST 2012**



IRRIGATION  
TRAINING &  
RESEARCH  
CENTER

## Improving Flow Measurement Accuracy at Farm Delivery Gates in California



**Technical Report**

California State University  
Agricultural Research Institute

July 2015

**IRRIGATION  
TRAINING &  
RESEARCH  
CENTER**

***Prepared by***

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## DISSEMINATION

The information developed in this report has led to several peer-reviewed journal articles, thesis project, and conference papers and presentations listed below. In addition, the BRAE 533 (Irrigation Project Design) and the ITRC Flow Measurement Books have been updated to include the recommendations outlined in this report. This information is being taught in industry short courses (2 per year) and Cal Poly undergraduate and graduate courses (BRAE 236 and BRAE 533).

### **Publication of Final Results:**

- Howes, D. J. and C. M. Burt (2015a). "Accuracy of Round Meter Gates for On-Farm Deliveries." ASCE Journal of Irrigation and Drainage Engineering, in press.
- Howes, D. J. and C. M. Burt (2015b). "Rating Rectangular Farm Delivery Meter Gates for Flow Measurement." ASCE Journal of Irrigation and Drainage Engineering, in press.
- Howes, D. J., C. M. Burt, and J.M. Thorburn (2015). "Supply Channel Velocity Influence on Farm Delivery Meter Gate Flow Measurement." In progress
- Thorburn, J.M. (2015). *Hydraulic Effects of Water Approach Velocity on Meter Gate Flow Measurement* (Master's thesis). In progress
- Burt, C.M. and D.J. Howes (2015). "Practical Guide for Meter Gates". ITRC Report Number: 15-001. Irrigation Training and Research Center, San Luis Obispo, CA.
- Feist, K. and C.M. Burt. (2014). Flow Measurement Options for Canal Turnouts. Paper presented at USCID Water Management Conference, Dec. 2-5, 2014. Denver, CO.

### **Presentations and Publication of Preliminary Results:**

- Howes, D.J. 2014. "Calibrating Orifice Flow Turnout Gates for SBx7-7 Compliance." California Irrigation Institute Conference on Building a Water and Energy Efficient California, January 23-24, 2014. Sacramento, CA.
- Howes, D.J. and R. Fulton. (2013). "ITRC metergate calibration testing for farm turnout delivery." Paper presented at USCID Water Management Conference, Oct. 22-25 2013. Denver, CO.
- Fulton, R. (2013). *Meter Gate Calibration and Accuracy Analysis* (Senior Project). California Polytechnic State University, San Luis Obispo.

## EXECUTIVE SUMMARY

Recent California legislation requires irrigation water agencies larger than 25,000 acres to measure volumetric water deliveries within specified levels of relative uncertainty. Although the meter gate is one of the most widely used flow measurement devices in California, little investigation has been conducted into the accuracy, limitations, and uncertainties of the rating tables developed over 60 years ago.

The Cal Poly Irrigation Training and Research Center (ITRC), through a CSU ARI grant, constructed a meter gate testing facility (see *Attachment A*) and tested five gates of various sizes and designs. The gate testing was conducted by varying the multiple parameters including upstream and downstream head, supply channel velocity, gate opening, and head above the gate. Data was also collected at various locations on the downstream side of the gate. In total, ITRC staff collected over 10,000 points of data during this evaluation.

Using the new rating tables for the three gates examined, the relative uncertainty is less than  $\pm 5$  to  $\pm 7\%$  at the 95% confidence level with the new rating tables, as compared to less than  $\pm 10\%$  at a 95% confidence level using common published tables. Uncertainties are lower than the required estimated 10.7% instantaneous flow rate uncertainty that will be needed to meet current SB X7-7 requirements. However, in order to ensure accurate flow measurement using these devices, they must be designed and operated within a certain set of recommended conditions. The remainder of this *Executive Summary* will describe the recommendations developed from this study. These recommendations and final rating tables can be found in *Attachment B – Practical Guide for Meter Gates*.

### ***Recommendations for Design and Operation of Meter Gates***

As with any flow measurement device, there are constraints and recommendations that must be followed to obtain accurate results. The following guidelines combine some current installation standards, authors' experience, and results found in this study.

1. Traditionally, the upstream head above the top of the turnout pipe ( $H_1$ ) was recommended to be equivalent to one pipe diameter. However, results in this study show that  $H_1 \geq 0.5 \times D$  provide accurate results. This will increase the number of sites that could potentially utilize meter gates accurately.
2. The range of gate openings should be maintained between 25% and 75% open (the relationship between gate opening and  $A_o/A_p$  can be found in Table 4). If the stilling well is in the correct location, higher gate openings can be used but should always remain below fully open. For smaller 0.30 m (12-inch) gates, the minimum opening should be increased to 30% to 40%. If smaller gate openings are used for only a portion of the season and larger openings for the remainder, the volumetric uncertainty (accuracy) over the season may not be greatly impacted. It is likely that more significant volume will be delivered with the larger gate openings because of the higher flow rates. The volumetric uncertainty caused by the flow rate uncertainty will be proportional to the volume

delivered at a specific gate opening. Therefore, the overall instantaneous flow rate uncertainty can be taken as the weighted average uncertainty at gate openings used weighted by the volumes delivered.

3. Sufficient upstream submergence is needed on the downstream end of the turnout pipe. The pipe downstream of the meter gate needs to be full. The water level needs to rise to some measurable level in the downstream stilling well. The downstream submergence should be at least 0.30 m (12 inches). However, more submergence may be needed so that a  $\Delta H$  of approximately 0.76 m (30 inches) is not exceeded. Previous recommendations limit the head loss to 0.46 m (18 inches) but head losses greater than this performed well in this study.
4. All rating tables and  $C_d$  values presented here require knowledge of the net gate opening, as measured by the shaft opening. The “zero” gate opening must be properly determined and marked on the gate shaft. This is not a trivial detail. Specific points are:
  - a) All measurements of gate opening, as well as the initial marking, must be made after the gate stem has been opened (on the upswing). This is because there is some slack or movement between the shaft and the gate itself.
  - b) The gate stem will move up some distance before the gate plate itself reaches the bottom of the pipe. The  $C_d$  values developed in this study and traditional rating tables depend on knowing the gate opening, not the movement from the gate seating position. The gate must be closed beyond the bottom of the pipe to seal off completely. That sealed position is not the “zero” position.
  - c) There must be some specific way to measure the shaft position when the bottom of the gate just barely clears the bottom of the pipe – in other words, when there is a “zero opening”. This is fairly easy to set and measure if the canal is empty or if a new gate is being installed. The gate is opened until a narrow strip of paper can be inserted between the bottom of the gate and the bottom of the pipe (zero position). If the canal is full, special calipers can be used to determine the actual net gate opening and the zero point on the gate stem can be identified from that.
  - d) The gate stem needs to be marked in a clear manner so that operators know where the “zero” opening is for the gate when they open the gate. In the field it is often easiest to cut into the stem about 1 cm (0.5 inches) with a grinder at the top of the gate lift nut. Then the operator should always measure the gate opening on the upswing from the top of the lift nut to the bottom of the notch.
5. The stilling well needs to have sufficient diameter to dampen the turbulence, and so that operators can see into it. The authors recommends a stilling well of 0.15 m – 0.21 m (6 inches – 8 inches) diameter, with a tap hole of about 0.016 m or 0.019 m (5/8 inch or 3/4 inch) diameter. The stilling well to tap hole diameters should be greater than 7:1.
6. The tap hole must be on the top of the pipe and should be 0.305 m (12 inches) downstream of the downstream face of the gate. However, the stilling well does not need to be centered over the access hole in the top of the discharge pipe. In general, it is good to have the stilling well close to the gate frame/bulkhead, so that it can be supported.

7. If the stilling well is less than 0.30 m (12 inches) from the face of the gate for larger gates, the error in measurement will be low if the gates remain less than 75% open. For 0.30 m gates and probably smaller, there is a high likelihood of substantial error with different tap locations. It is recommended that the tap location at these sites be moved to the correct location or the correction factor ( $F_{tap}$ ) should be multiplied by the flow rate obtained from tables based on the 0.305 m (12-inch) tap location as described.
8. The tap hole should also be on the top of a corrugation if corrugated pipe is used. The closest peak to the 0.305 m (12-inch) ideal tap location will be sufficient.
9. To simplify the measurement for head difference ( $\Delta H$ ) use the same datum (elevation) for both measurements. See Figure 1 in the body of this report for a stilling well with the top correctly placed at the same elevation as the gate frame, and with a proper diameter. The top of the stilling well should be at the same elevation as the top of the gate frame (where the bottom of the lift nut rests). Then the upstream measurement should be taken from the top of the gate frame to the water level. The downstream measurement should be taken from the top of the stilling well to the water level in the well. The  $\Delta H$  is the difference between the upstream and downstream measurements from the datum (reference) to the water levels.
10. In many cases having the stilling well top at the same elevation as the top of the gate frame will prevent debris and soil from falling into the well and plugging the tap hole. This can occur during maintenance of the canal bank and road. If the top of the gate frame is still low enough that debris can fall in, a cap should be placed over the well when measurements are not being taken.
11. Volumetric accuracy can be improved if:
  - a) Additional instantaneous flow measurements are taken during the irrigation event. An example would be taking flow measurements every 24 hours at open turnouts even if adjustments are not being made. This will reduce  $U_{Hu}$  and  $U_{Hd}$ .
  - b) The time the delivery starts and stops is properly recorded. If operators open and close turnout gates this can be done without additional work.

The new  $C_d$  values from this study for the five gates examined presented in Table 4 in the body of this report should be used for creating new rating tables for these gates. While a best-fit polynomial can be created for each gate, it is more appropriate to interpolate between these values to estimate  $C_d$  values for other gate openings. Utilizing variables outside of those tested in a regression equation can lead to significant error in the computed  $C_d$  ( $\hat{C}_d$ ). Linear interpolation or a more advanced interpolation method can be used. If an advanced interpolation is used the values should be plotted with those reported in this report to ensure that the results conform.

## TABLE OF CONTENTS

<b>Executive Summary</b> .....	<b>i</b>
Recommendations for Design and Operation of Meter Gates .....	<b>i</b>
<b>Introduction</b> .....	<b>1</b>
<b>Background</b> .....	<b>3</b>
Flow Measurement Errors and Uncertainty .....	<b>5</b>
<b>Procedures</b> .....	<b>7</b>
Meter Gate Testing Facility .....	<b>7</b>
Flow Rate through the Meter Gate (Q) .....	<b>9</b>
Net Gate Opening Area ( $A_0$ ) .....	<b>10</b>
<i>Round Gate Opening Area</i> .....	<b>10</b>
<i>Rectangular Gate Opening Area</i> .....	<b>11</b>
Pressure Head Testing Design .....	<b>12</b>
Meter Gate Testing Scenarios .....	<b>14</b>
<i>Evaluation of Flow Computed from Different Calibration Sources</i> .....	<b>17</b>
<b>Results and Discussion</b> .....	<b>18</b>
Correlation between $C_d$ and Testing Variables .....	<b>18</b>
ITRC New Coefficient of Discharge ( $C_d$ ) Values .....	<b>24</b>
Error Using Original Rating Tables or Charts .....	<b>26</b>
Downstream Pressure Tap Location Influence .....	<b>28</b>
Supply Channel Velocity Influence .....	<b>30</b>
<b>Conclusion</b> .....	<b>32</b>
<b>References</b> .....	<b>33</b>

## LIST OF ATTACHMENTS

<b>Attachment A</b>	Meter Gate Testing Facility
<b>Attachment B</b>	Practical Guide for Meter Gates
<b>Attachment C</b>	ITRC Water Measurement Tables



## LIST OF FIGURES

Figure 1. Recommended meter gate installation (Howes and Burt, 2015a).....	4
Figure 2. Layout of the testing facility constructed at the Cal Poly Water Resources Facility. ....	8
Figure 3. Round gate (white) on a round pipe showing variables used to compute the net gate open area (gray) for a round gate on a round pipe (Howes and Burt 2015a).....	11
Figure 4. Rectangular gate on a round pipe showing variables used to compute net gate opening area (shaded gray) (Howes and Burt 2015b).....	12
Figure 5. Side view of meter gate testing facility showing the pressure tap locations where measurements were taken.....	13
Figure 6. Coefficient of discharge ( $C_d$ ) for the three round gates (a-c) and the two rectangular gates (d-e) for the low supply channel velocity testing with the downstream head measurement taken 12-inches (0.305 m) downstream of the face of the gate. ....	19
Figure 7. $C_d$ related to the fraction of upstream head to turnout pipe diameter for the low supply channel velocity scenarios.....	21
Figure 8. $C_d$ related to the relative change in head ( $\Delta H/H_i$ ) for all gates tested for the low supply channel velocity with the downstream head measured at the 0.30 m (12-inch) pressure tap location.....	22
Figure 9. Relationship between $C_d$ and Reynolds Number in the discharge pipe for the low velocity testing conditions and the downstream head measurement at the 0.305 m (12-inch) pressure tap location.....	23
Figure 10. Average discharge coefficient by gate opening for the different gate sizes and types investigated based on the 0.305 m (12-inch) pressure tap location for downstream head measurements.....	24
Figure 11. Comparison of percent error and uncertainty of discharge (Q) derived from the original Armco Meter Gate Rated Table to those measured in this study (low supply channel velocity tests).....	26
Figure 12. Rectangular gate percent error and uncertainty from using $C_d$ values based on the USBR Water Measurement Manual Figure 9-10 to compute discharge compared to the actual flow measured in this study (low supply channel velocity tests).....	27
Figure 13. Influence of pressure tap location (6-inches (0.15 m) and 8 inches (0.20 m) downstream from the back face of the gate) on $C_d$ , compared to the $C_d$ computed using the standard 12 inches (0.30 m) pressure tap location.....	29
Figure 14. Percent flow rate error and uncertainty using $C_d$ values from the low channel velocity tests for all supply channel velocities.....	31

## LIST OF TABLES

Table 1. Tests conducted under low supply channel velocity for the meter gate testing. ....	15
Table 2. Range of supply channel depths, velocity, and Froude numbers evaluated for each gate type and size. The round gate type refers to the Armco-Type gate. ....	16
Table 3. Multiple regression coefficients and corresponding P-values for each gate size tested .....	20
Table 4. Recommended new $C_d$ values from this study by net gate opening ( $y$ ) and fraction of net opening area ( $A_o/A_p$ ). .....	25

## INTRODUCTION

Accurate flow measurement of water from irrigation projects delivered to farms is important for a number of reasons. Farmers use the flow measurement and volumes delivered to know how much water was applied to fields; the amount applied must be known for irrigation scheduling and management. Irrigation projects have been shifting from assessment-based fees to volumetric billing (often there is some combination of both). Irrigation district operators also need good turnout flow measurement to properly operate canals.

On November 9, 2009, the California Senate enacted Senate Bill (SB) X7-7 mandating water conservation and water use efficiency targets for urban and agricultural water suppliers. As part of this legislation, agricultural water suppliers serving areas greater than 25,000 acres were required to have a tentative plan in place for how irrigation districts will measure water deliveries volumetrically within mandated levels of accuracy by July 31, 2012. The districts' plans are to be officially updated in 2015. Over the last year of this process the lead agency, the California Department of Water Resources (DWR), held a number of public hearings and meetings to clarify issues to water users about flow measurement from open channels (through turnouts). Dr. Charles Burt and Dr. Stuart Styles, Chairman and Director, respectively, of the Irrigation Training and Research Center (ITRC), California Polytechnic State University, San Luis Obispo have been active participants in this process. Dr. Burt is a member of the Agricultural Stakeholders Committee (ASC) and has provided his expertise on turnout flow measurement through a number of presentations and documents ([www.water.ca.gov/wateruseefficiency/sb7/committees/ag/a2/](http://www.water.ca.gov/wateruseefficiency/sb7/committees/ag/a2/)).

Specific regulation for agricultural irrigation water agency turnout flow measurement includes (DWR 2011):

- If there is an existing flow measurement device, the volumetric accuracy must be within  $\pm 12\%$ .
- For new flow measurement devices, the volumetric accuracy must be within a laboratory rated  $\pm 5\%$  or  $\pm 10\%$  in the field if laboratory ratings are not available.

Because of the vast array of conditions in the field, there is no single hardware solution that will economically meet the SB X7-7 requirements in all agricultural water delivery locations throughout California. In most cases, the regions that can use simple solutions already utilize potentially accurate volumetric flow measurement devices such as propeller meters. The challenge is finding solutions for the difficult situations. These include areas with high sediment loads, aquatic weeds, little available head loss (where the water levels upstream and downstream of a turnout gate are similar), and high flow rates.

“Volumetric accuracy” is defined in the SB X7-7 regulations as the percent error between the measured volume and the actual or true volume. The measurement device provides the measured volume (volumes may be computed from a measured flow rate and the duration of delivery) and the actual volume is determined through laboratory or field testing (DWR 2011).

*A second term used in this report is “uncertainty”, which is the proper term to use when describing the range of values within which the actual value lies for a stated confidence level. In other words, “on the average” a measured number may equal 0.625, which is identical to the “true” value. However, any single measured value may be different.*

In some cases volumetric measurements are made directly by the meter (e.g., propeller meter with totalizing capability). However, instantaneous flow rate is often measured and volumes are estimated based on the duration of the delivery. Since the instantaneous flow rate ( $Q$ ) may have only been measured at one or more instances during the duration of the delivery, there is some uncertainty beyond the flow meter uncertainty of the volume computed from the device that will influence the volumetric measurement accuracy.

There are several factors that will influence the combined uncertainty of the volumetric measurement from devices such as meter gates, where volumes are computed based on instantaneous flow measurement and the duration of the water delivery. Flow measurement accuracy is a major component of the volumetric accuracy and was investigated for meter gates (special submerged orifice) in this work. The change in supply channel water level between flow measurement reading events will influence the head on the turnout gate and therefore influence the flow rate. Water level variation downstream of the orifice will influence the head loss across the gate, which can change the flow rate. Finally, inaccuracy in determining the correct duration of the irrigation event will influence the computed volumetric accuracy. These are discussed in the *Flow Measurement Errors and Uncertainty* section.

One of the most commonly used farm delivery (i.e., turnout) flow measurement devices in California is a meter gate (ITRC 2000; ITRC 2002). Meter gates provide a number of advantages if these devices can meet the volumetric accuracy requirements. A major advantage is that thousands of these devices are already installed; water agencies may not need to invest in new devices. Water quality issues including high sediment loads and aquatic weeds do not cause significant problems, and annual maintenance and calibration costs are low with meter gates.

As will be discussed, rating tables exist for common meter gates. One purpose of this work was to compare existing rating table values for several gate sizes against laboratory evaluations. Another was to provide improved gate discharge equations, if found, and to expand the equations to cover a wider range of configurations. Additionally, there was a need to provide clear rules on the installation and operation of these devices to meet the accuracy requirements in SB X7-7. Finally, new rating tables were developed for two rectangular gate sizes commonly installed as new or replacement gates for irrigation turnout delivery.

## BACKGROUND

The meter gate is a special type of rated orifice (sluice gate) that generally uses a round gate to control water flowing into a round pipeline. Meter gates are submerged and the downstream head is typically measured 0.30 m (12 inches) downstream of the back face of the gate through an access hole in the top of the pipeline connected to a stilling well. Irrigation agency operators use rating tables for a particular gate size, with measurements of the head loss ( $\Delta H$ ) between the supply channel and the downstream water level, and the net gate opening ( $y$ ) to obtain a flow rate through the gate. Rating table development started around 1918 when Modesto Irrigation District began an investigation into calibrating standard gate designs and installations. Modesto ID selected the Calco Slide Headgate Model 101 as its standard gate.

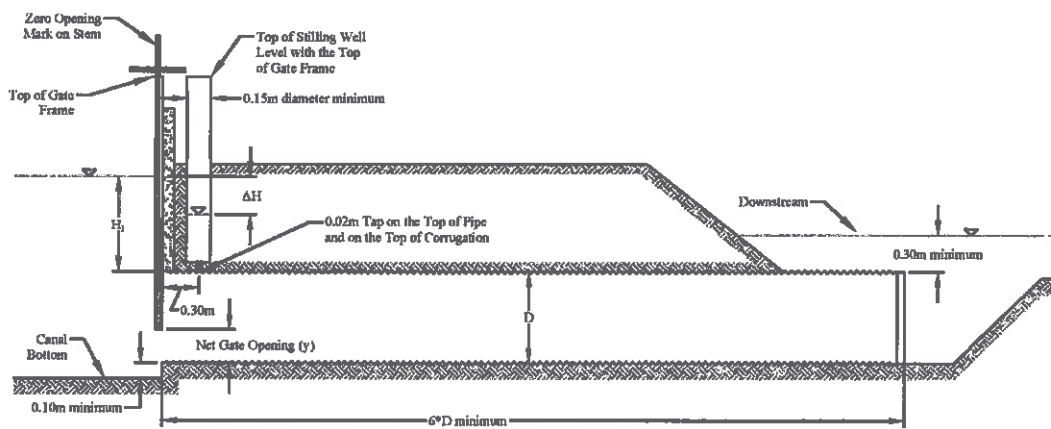
Calco (California Corrugated Culvert Company, Berkeley, CA) was a division of Armco (American Rolling Mill Company); the gates became known as the Armco Model 101. The basic design of the round gate on a round pipe is generally referred to as the Armco-type gate. The Armco Model 101 was acquired by Fresno Valves and Casting, Inc. (Selma, CA) and is still being manufactured as the Series 6600 Model 101C. Other, similar round canal turnout gates by other manufacturers include the Waterman Industries (Exeter, CA) C-10 canal gate and XCAD (Paul, ID) X-GATE™ W-type. It should be noted that the gates by themselves are not meter gates. It is necessary to properly install the tap and stilling well downstream of the gate as well as identify the zero openings to measure flow rates, as will be discussed.

The original Modesto ID ratings were based on submerged gates with different lengths of pipe downstream (Armco 1949). Since the pipe lengths can vary depending on installation, in the mid-1920's Fresno Irrigation District constructed a facility and began developing rating tables using a standard downstream head measurement of 0.30 m (12 inches) behind the face of the gate, which was also a Calco (Armco) Model 101 (Fresno Irrigation District 1928). Fresno ID conducted tests for gate sizes from 0.20 m to 0.61 m (8-inch to 24-inch). These rating tables were published by Armco for the Model 101 meter gate until approximately 1951, when the U.S. Bureau of Reclamation (USBR) completed another set of meter gate ratings for gate sizes ranging from 0.20 m to 1.22 m (8 inches to 48 inches) (Summers 1951). The reason for the USBR rating table development was that the USBR found errors in the Fresno ID ratings of up to 18% (Summers 1951).

Since the USBR rating development (Summers 1951), very little work has been conducted to examine the accuracy of Armco-type meter gates. Other researchers have used the data collected during the USBR investigation without examining the accuracy of the original data (Cadena and Magallanez 2005).

Prior recommendations on installation of meter gates can be found in the USBR Water Measurement Manual (USBR 1997) and in the Armco Rating Table booklet (Armco Steel Corporation 1975). In the field, there can be a variety of installations that do not conform to either set of recommendations and may have been a result of confusion from alternative instructions or mistakes. The issues seen in the field may be attributed to differing recommendations. For example, the Armco Rating Table booklet and Summers (1951) recommend that the stilling well tap for the downstream head measurement be placed 0.305 m (12 inches) behind the face of the turnout gate. However, the USBR Water Measurement Manual and Ball (1961; 1962) state a preferred distance of one-third of the turnout pipe diameter downstream. The result is a variety of downstream head measurement locations.

Figure 1 shows the recommended installation of a meter gate with some modifications to the stilling well and pressure tap recommendations based on the authors' experiences with these devices. The stilling well in Figure 1 is taller than those shown in the USBR Water Measurement Manual and the Armco Rating Table booklet, which show the top of the well nearly level with the top of the channel bank. Raising the well above the bank prevents debris from depositing in the well when the channel bank road is being graded.



**Figure 1. Recommended meter gate installation (Howes and Burt, 2015a)**

In addition, the top of the stilling well should be level with the top of the gate frame. This allows the operator to measure down from the top of the gate frame and the top of the stilling well to the water surfaces to obtain the head loss ( $\Delta H$ ). This is an alternative to the typical meter gate well assemblies that can be purchased from manufacturers and those shown in the literature discussed. It is common to see two wells on the downstream side of the gate with the same top elevations. One well is connected to the top of the pipe as shown in Figure 1 and the other is connected with a horizontal pipe to the upstream canal. The authors have found that this horizontal pipe plugs easily and is very difficult to clean out with water in the upstream canal. In most cases the upstream water level does not fluctuate significantly so the stilling well for the upstream reading is not necessary.

The stilling well and tap sizes are usually not specified in published meter gate installation recommendations. A stilling well (inside diameter) to (tap diameter) ratio of no less than 8:1 is recommended to dampen the downstream water level fluctuations.

The typical recommendation for the upstream head above the top of the pipe ( $H_I$ ) is a minimum of one pipe diameter ( $D$ ). The USBR manual and Armco booklet recommend not having a head loss ( $\Delta H$ ) greater than 0.46 m (18 inches) and the Armco tables do not show values for  $\Delta H$  greater than this. An updated set of recommendations will be presented in the *Results and Discussion* and *Application* sections.

The original USBR testing setup for the meter gate ratings was oriented so that the supply water entered the meter gate straight on (parallel to the meter gate discharge pipe) (Summers 1951). The testing conducted for the work presented here had the gates oriented perpendicular to the supply channel flow, which is common in field installations.

### ***Flow Measurement Errors and Uncertainty***

Several primary factors influence the combined uncertainty of the volumetric measurement from meter gates: flow measurement accuracy, the change in supply channel water level between flow measurement readings, water level variation downstream of the orifice, and inaccuracy in determining the correct duration of the irrigation event. A good discussion on each of these components can be found in Burt and Geer (2012). Since the accuracies of each component are independent, they were combined (Burt and Geer 2012) using the root-sum-of-squares method (Taylor and Kuyatt 1994) to compute the volumetric uncertainty as:

$$U_v = 100 \times \sqrt{\left(\frac{U_Q}{100}\right)^2 + \left(\frac{U_{Hu}}{100}\right)^2 + \left(\frac{U_{Hd}}{100}\right)^2 + \left(\frac{U_T}{100}\right)^2} \quad (1)$$

Where  $U_v$  is the percent (relative) volumetric expanded uncertainty where the resulting value describes the range within which true values lie both in the positive and negative around the measured value with a 95% confidence level or within two standard deviations (i.e., expanded uncertainty of the volumetric measurement is  $\pm U_v$ );  $U_Q$  is the instantaneous flow measurement accuracy;  $U_{Hu}$  is the accuracy in flow rate estimated due to variable upstream supply canal water levels;  $U_{Hd}$  is the accuracy in flow rate estimated due to variable downstream water levels; and  $U_T$  is the accuracy of the delivery duration estimate. SB X7-7 does not provide a standard coverage factor (number of standard deviations) or confidence level for uncertainty. It should be noted that in the U.S., some organizations base uncertainty and standard error reporting on one standard deviation (67% confidence level). In this report, two standard deviations (i.e.,  $k = 2$  and 95% level of confidence) will be used based on international recommended standards (Taylor and Kuyatt 1994).

An evaluation of upstream supply channel variability for operating turnouts was conducted at San Luis Canal Company (Los Banos, CA) during the summer of 2012 (Burt and Geer 2012). Canal water levels were recorded on an hourly basis at 22 sites, collecting data for approximately 90 irrigation deliveries. The channel conditions and structures are typical of many upstream channel distribution systems in the western U.S. with flashboard weir check structures for water level control and submerged orifice turnouts. The results of this evaluation showed that under submerged flow conditions, the uncertainty of flow measurement due to supply channel water level variation ( $U_{Hu}$ ) was within  $\pm 2\%$  with a 95% level of confidence.

$U_{Hd}$  and  $U_T$  are influenced by farming practices and irrigation water agency operational rules. Burt and Geer conservatively estimated the expanded  $U_{Hd}$ , or the uncertainty due to change in backpressure on the gate, as  $\pm 3\%$  based on field experience. Additional research is needed to evaluate this uncertainty parameter, and it would depend upon the average elevation change between the supply canal and the farm ditch. The  $U_T$  of  $\pm 4\%$  was based on a conservative estimate that the difference between actual and recorded duration would be within  $\pm 1$  hour within a 24-hour delivery period. In many cases irrigation delivery durations are longer than 24 hours, which would result in a smaller  $U_T$  if a  $\pm 1$  hour error is recorded versus actual duration.

Rearranging Eq. 1 and solving for  $U_Q$  based on  $U_{Hu} = 2\%$ ,  $U_{Hd} = 3\%$ ,  $U_T = 4\%$ , and the SB X7-7 requirement of  $U_v = 12\%$ , the relative instantaneous flow measurement uncertainty ( $U_Q$ ) that can be tolerated is computed to be  $\pm 10.7\%$ . The uncertainty of instantaneous flow measurement ( $U_Q$ ) for meter gates was the focus of the work presented here.

The overall objectives of the study were to check the accuracy of the existing Armco rating tables, provide corrected or more accurate rating tables if necessary, provide laboratory-verified accuracy under a clearly defined set of installation and operational standards, expand the operational range of meter gate rating tables if possible (so that these can be used in a wider range of sites), examine how supply channel velocities influence accuracy, and, when installations do not conform to standards, determine what if any influence this will have on accuracy.



## PROCEDURES

The standard discharge equation for a submerged orifice is:

$$Q = C_d A_o \sqrt{2g\Delta H} \quad (2)$$

Where  $Q$  is the flow rate (cubic meters per second (CMS)),  $C_d$  is the coefficient of discharge,  $A_o$  is the net gate opened area ( $m^2$ ),  $g$  is the gravitational acceleration ( $9.81 \text{ m/s}^2$ ), and  $\Delta H$  is the head loss across the gate (meters). The coefficient of velocity ( $C_v$ ) has been neglected since the velocity of approach is close to zero because these gates are typically installed perpendicular to the supply channel velocity streamlines.

The  $C_d$  value can be computed from Eq. 2 as:

$$C_d = \frac{Q}{A_o \sqrt{2g\Delta H}} \quad (3)$$

As will be discussed, a new meter gate testing facility was constructed. Measurements for  $Q$ ,  $A$ , and  $H$  will be discussed in the following sections.

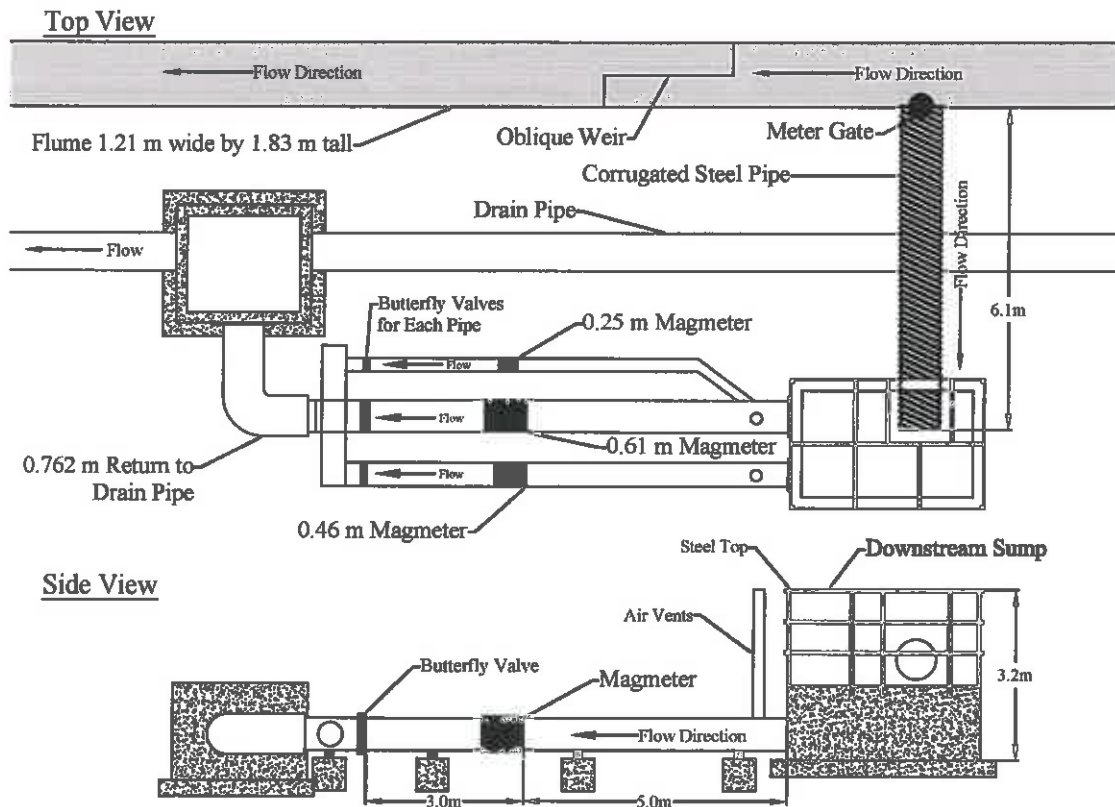
### *Meter Gate Testing Facility*

A meter gate testing facility was constructed at the Cal Poly Irrigation Training and Research Center (ITRC) Water Resources Facility. Photos of the construction can be found in *Attachment A*. The new testing facility was added to an existing elevated flume near its upstream end (Figure 2). Prior to modification, the rectangular flume was 1.21 m wide by 1.21 m in height on a 0.002 slope. A portion of the flume wall was raised from 1.21 m to 1.83 m for this testing. If the recirculation pump is used, 0.85 cms can be supplied through the flume. The recirculation pump has a variable frequency drive (VFD) on a 100 horsepower (HP) motor. Flow is measured exiting the pump by a calibrated 0.762 m McCrometer UltraMag magnetic meter on the supply pipeline (not shown in Figure 2). This water enters a basin at the head of the flume through a 0.762 m steel pipeline. Flow rates into the flume are controlled by adjusting the VFD to match the target. The flow through the 0.762 m UltraMag ( $Q_i$ ), the supply channel dimensions, and upstream water level ( $d_i$ ) were used to compute the supply channel Froude number ( $F_1$ ) was computed as:

$$F_1 = \frac{V_1}{\sqrt{gd_1}} \quad (4)$$

As shown in Figure 2, the meter gates were attached to the flume perpendicular to the flume flow. The meter gate was connected to the side of the flume with a removable steel bulkhead so that the gate frame was attached flush to the side of the steel. The frame and gate protruded slightly into the flow the width of the gate frame as can be found in many field installations. The bottom of the gate was set at least 0.10 m (4 inches) above the bottom of

the channel as recommended by the USBR. A corrugated discharge pipe, sized to match the gate diameter, connected the gate to the downstream sump as it would be in a typical field installation. The sump on the downstream end of the corrugated pipe had a top elevation equivalent to the top of the flume walls so that a full range of head differentials could be tested.



**Figure 2. Layout of the testing facility constructed at the Cal Poly Water Resources Facility. (Howes and Burt, 2015a)**

The Armco and other gate rating tables require the user to measure the net gate stem opening from the zero opening. The zero opening is the point at which the bottom of the gate is level with the bottom of the pipe. To prevent leakage when the gate is closed, the gate plate diameter is larger than the pipe diameter, and the bottom of the gate seats closed in a position below the inside bottom of the pipe. The gate must open some distance, which varies by gate size and manufacturer, before the zero opening is reached. When a new gate was installed for testing, the stem on each gate was marked to identify the zero opening. A procedure for marking the zero opening is described in the *Application* section.

Three steel discharge pipes were connected to the bottom of the downstream sump (Figure 2). Each pipe had a calibrated magnetic flow meter (mag meter) with the same inside diameter as the steel pipes; these flow meters were used to determine the flow rate ( $Q$ ) in Eq. 3. The nominal magnetic (mag) meter sizes are shown in Figure 2. A discussion on the calibration and operation of these mag meters will be discussed in the next section.

Butterfly valves were installed at the downstream end of each of the mag meter pipes to control the water level in the sump and ensure that the pipe flowed full. These valves were operated manually. The flow leaving the mag meter pipelines entered a manifold where it was discharged into a drain line to be recirculated back to the head of the flume. The drain line ran from an emergency spill at the upper supply sump (to the right of the meter gate in Figure 2 approximately 18 meters) down to a sump at the tail end of the flume where the recirculation pump is located.

In the flume, a 3.7 m long oblique weir was used to control the water level (head) upstream of the meter gate. The weir crest elevation was manually adjusted by adding or removing wood boards (flashboards). All flow passing through the VFD and the 0.762 m mag meter entered the flume and either passed through the meter gate or went over the weir. The 0.762 m mag meter was used to measure the flow rate entering the flume so that the velocity of the water in the flume could be known. The results presented in this report utilize very low velocities in the flume to negate the impacts of supply channel velocity on the results and to provide a baseline. Since supply channel velocity will depend on entrance conditions and the channel, it is anticipated that if adjustments are necessary, they would be applied to the baseline ratings developed here.

### ***Flow Rate through the Meter Gate (Q)***

Three magnetic meters were installed downstream of the meter gate to determine the flow rate standard (Q) shown in Eq. 3. The 0.61 m (24-inch) and 0.46 m (18-inch) McCrometer UltraMag mag meters and the 0.25 m (10-inch) Seametrics AG2000 were installed to provide a range of flow testing capabilities. For the results that will be presented here, only one mag meter was used for one test.

Calibration of each meter involved installing it in a pipeline within and parallel to the flume (at different times) prior to the meter gate testing. The meter readings were compared against the flow rate computed from a National Institute of Standards and Technology (NIST) traceable weigh tank at the downstream end. At least nine different flow rates were tested for each meter and the weigh tank flow rate was compared to the readings from the mag meter. A best-fit linear regression was developed for each gate and used to compute the calibrated flow. The r-squared value for all three calibration equations was greater than 0.999.

The pre-calibration average percent error of the 0.61 m mag meter was -4.43%. Post-calibration the error was 0.14% with a root mean squared error (RMSE) of 0.0029 cms and a coefficient of variation of the RMSE (CVRMSE) of 0.014. The pre-calibration average percent error for the 0.46 m mag meter was -0.67%. Post-calibration the average percent error was 0.07% with a RMSE of 0.0012 cms and a CVRMSE of 0.007. Pre-calibration average percent error was for the 0.25 m mag meter was 3.37%. Post-calibration for the 0.25 m mag meter was -0.12% with a RMSE of 0.0017 cms and a CVRMSE of 0.043.

Each mag meter had a digital display showing flow rate. Readings were recorded manually during the testing four times for each test after steady state conditions were reached. The calibration equations for each meter gate were applied to the raw flow rates recorded from the digital displays during post-processing of the data.

### ***Net Gate Opening Area ( $A_o$ )***

In this study, the actual gate opening area ( $A_o$ ) was used to compute the  $C_d$ . The original USBR calibration computed  $C_d$  based on the full pipe area ( $A_p$ ), not the actual opening area. Therefore, the  $C_d$  values from this study and the USBR work are not directly comparable. The full pipe  $C_d$  incorporates the loss across the gate, resulting in  $C_d$  values that approach zero as the gate opening becomes smaller. The actual gate opening was used here so that differences in actual  $C_d$  values could be compared between gate openings and different gate sizes. It should be noted that Cadena and Magallanez (2005) computed  $C_d$  values from the USBR meter gate tests based on an area approximation presented by Hager (Hager 1987). However, that area approximation performs poorly at gate openings less than 25% and greater than 55%, so the  $C_d$  values computed by Cadena and Magallanez will also not be comparable to those presented in this report.

### **Round Gate Opening Area**

An (opening area) to (gate opening position) relationship was derived for a circular gate on a circular pipe. To ensure that the gate seats completely over the pipe, the radius of the gate ( $R_g$ ) is larger than the radius of the pipe ( $R_p$ ). The relationship will depend on the gate manufacturer and the gate size. Figure 3 shows key measurements used to compute the gate opening area.

The following is the relationship between net gate open area ( $A_o$ ) and net gate opening ( $y$ ):

$$A_o = A_i - A_{\text{subtracted}} \quad (5)$$

Where:

$$A_i = R_p^2 \times \cos^{-1}\left(\frac{O}{R_p}\right) + O \times \sqrt{R_p^2 - O^2} \quad (6)$$

$$A_{\text{subtracted}} = R_g^2 \times \cos^{-1}\left(\frac{P-O}{R_g}\right) + (O-P) \times \sqrt{R_g^2 - O^2} \quad (7)$$

$$P = y + R_g - R_p \quad (8)$$

$$\text{Offset} = y + R_g - R_p \quad (12) \quad (9)$$

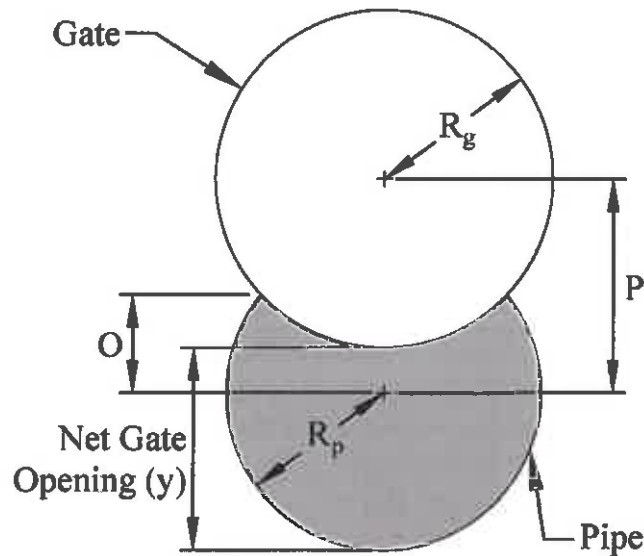


Figure 3. Round gate (white) on a round pipe showing variables used to compute the net gate open area (gray) for a round gate on a round pipe (Howes and Burt 2015a).

From Figure 3,  $y$  is the net gate opening,  $R_g$  is the outside radius of the gate,  $R_p$  is the inside radius of the pipe,  $O$  is the distance from the centerline of the pipe to the intersection of the pipe and gate, and  $P$  is the distance from the center of the pipe to the center of the gate. In Eq. 4-6,  $A_i$  is the pipe area at the gate opening if the gate bottom was flat along a geometric chord where the gate intersects the pipe on both sides, and  $A_{subtracted}$  is the area of the circular portion of the gate protruding into the pipe area below this chord. The net gate opening (also referred to as the stem height) is a critical measurement from the bottom of the inside diameter of the pipe to the bottom of the gate. This measurement is often mistaken in the field and as will be discussed, care must be taken to identify the correct zero opening just as the gate breaches the bottom of the pipe.

The previous equations for round gate opening area are equivalent to those used by Skogerboe and Merkley (1996). However, users of that text should be aware of an error in one of the equations presented (Equation 10.37) and refer to an example on the following pages of that chapter to determine the correct equation.

### Rectangular Gate Opening Area

The following is the relationship between net gate open area ( $A_o$ ), pipe radius ( $R_p$ ), and net gate opening ( $y$ ) from Skogerboe and Merkley (1996):

$$A_o = \frac{R_p^2}{2} \times \left[ 2 \times \cos^{-1} \left( 1 - \frac{2 \times y}{R_p} \right) - \sin \left( 2 \times \cos^{-1} \left( 1 - \frac{2 \times y}{R_p} \right) \right) \right] \quad (10)$$

Where  $y$  is the net gate opening and  $R_p$  is the pipe inside radius shown in Figure 3. Since the ratings are based on net gate opening (also referred to as the stem height), correct measurement is critical. The correct procedure for this measurement is from the bottom of the inside diameter of the pipe to the bottom of the gate. The gate stem (above the frame) must be marked to indicate the correct zero opening, which occurs just as the gate breaches the bottom of the pipe, while the gate is being opened (not closed). The distinction between measurement during the action of opening or closing the gate is necessary because the stem-gate connection almost always has free movement.

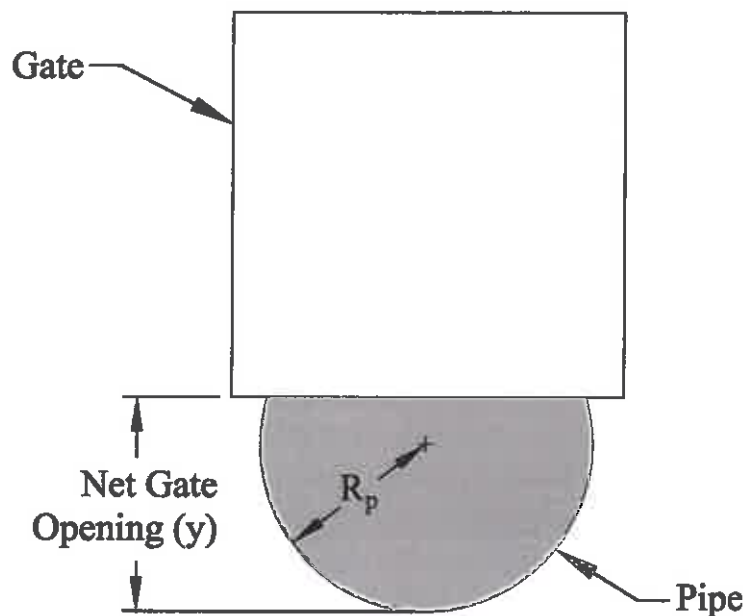


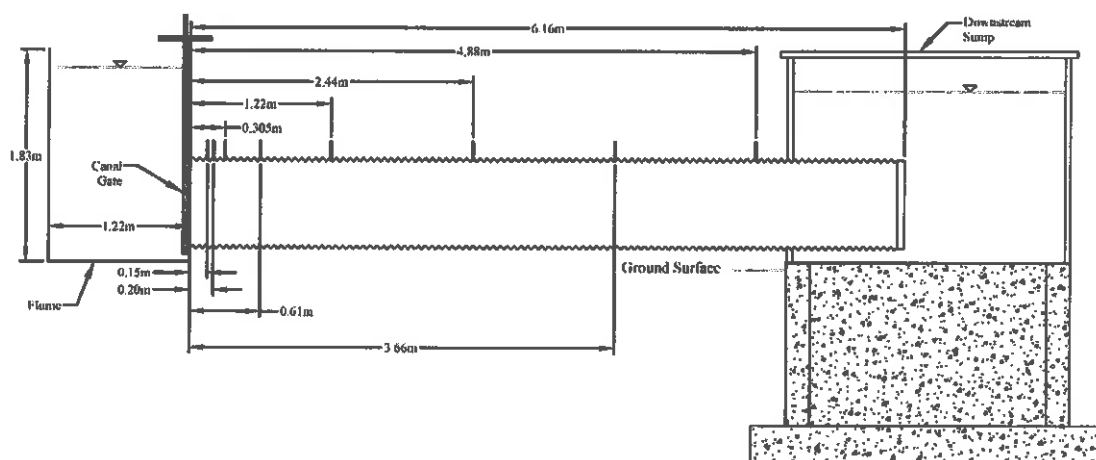
Figure 4. Rectangular gate on a round pipe showing variables used to compute net gate opening area (shaded gray) (Howes and Burt 2015b).

### ***Pressure Head Testing Design***

In order to determine the  $\Delta H$  from Eq. 3, the difference in head upstream and downstream of the gate is needed. For this evaluation, multiple locations downstream from the back face of the gate were examined to determine the effect that location of the downstream measurement had on the estimated flow. For a standard meter gate design, the Armco water measurement tables state that the  $\Delta H$  is the difference in head between the upstream water level and the water level measured in a stilling well that is connected to the turnout pipeline tapped 0.305 m (12 inches) downstream from the face of the gate (Armco Steel Corporation 1975). This standard location was used for the baseline rating (to compare existing Armco Rating Tables). Figure 5 shows the locations where downstream head was measured for various readings, including at the 0.305 m location.

At the top of the closest corrugation to the locations shown in Figure 5 (0.15, 0.20, 0.305, 1.22, 2.44, 4.88 m), 0.019 m (3/4") holes were tapped for the head measurements along the pipe. In addition to these locations, head measurements were made for the upstream water level through a 0.019 m hole in the bulkhead at the same elevation as the top of the corrugated pipe.

Stilling wells were required because of the fluctuation in pressure head in the pipeline; proper sizing is critical, as will be noted in the *Discussion* section. The stilling wells were grouped together for ease of leveling and measurement and were located on the side of the downstream sump wall. The holes were connected to the 0.152 m (6-inch) PVC stilling wells using 0.016 m clear flexible plastic hose that was sloped slightly upward from the head measurement location to the stilling wells. Clear plastic hose was used so that air bubbles were visible and could be removed.



**Figure 5. Side view of meter gate testing facility showing the pressure tap locations where measurements were taken.**

The  $\Delta H$  was measured directly by using a SMAR-LD301 pressure differential transmitter/transducer (SMAR, Houston, TX). The LD301 is temperature compensating and the differential range was modified to decrease the output uncertainty to within  $\pm 1$  mm. The stilling well from the tap connected to the water level in the flume (upstream head) on one side of the differential pressure transducer. The other side of the transducer was connected to a manifold. The manifold connected stilling wells for each downstream head location so that one head measurement location could be isolated with valves and compared to the upstream head for each measurement. The testing personnel would sequentially turn valves on and off and manually take  $\Delta H$  readings for each head measurement location.

As redundancy, staff gauges were connected to each stilling well and referenced to the same datum. The staff gauge increments were approximately 1.5 mm. These visual readings were used to manually compute  $\Delta H$  to check that the pressure differential transducer was functioning properly. If the  $\Delta H$  from the LD301 differed from manual readings by more than 1%, the pressure transducer was zeroed and the test was repeated.

## ***Meter Gate Testing Scenarios***

Testing involved examining a wide array of conditions to examine effects that these conditions have on  $C_d$ . Conditions that were varied included upstream water level in the flume, water level in the downstream sump, gate opening, supply channel velocity, gate size, and gate type. Additionally, as discussed, downstream head measurements were taken at multiple locations downstream of the gate. In total, over 10,000 individual points of data were collected in this study

Armco-type (round) gates examined were manufactured and provided by Fresno Valves and Casting, Inc. Three common gate sizes were examined, nominally 0.30 m (12-inch), 0.46 m (18-inch), and a 0.61 m (24-inch) Model 101C. These are the same designs as the Armco gates used in the original Fresno Irrigation District and USBR studies. Two commonly used rectangular canal gate sizes (0.46 m (18-inch) and 0.61 m (24-inch)) were examined under various conditions. These rectangular gates were manufactured by Mechanical Associates (Visalia, CA) and were donated for testing by San Luis Canal Company (Dos Palos, CA).

Table 1 shows the testing range for the different tests conducted as part of the meter gate evaluation under low supply channel velocity. The low supply channel velocity runs were used as a baseline. Other supply channel velocities were also tested as will be discussed. The low supply channel testing had velocity in the supply channel, downstream of the meter gate was always less than 0.2 meters per second (m/s).

The goal of this evaluation was to collect data over a wide range of scenarios for each gate size. In large-scale testing it was not feasible or important to match a predetermined water level or head loss target exactly. Therefore, a target range was attempted for each scenario (e.g., an actual  $\Delta H$  of 0.11-0.19 m would be satisfactory for a test with a target  $\Delta H$  of 0.15 m) and the results are presented for the actual measured variables. As Table 1 indicates, a variety of head differences were evaluated. The actual head difference (head loss) was varied depending on the upstream head available. With Low upstream head, the limited head available typically resulted in a small head loss.

Upstream head was varied from a classification of Very Low, upwards. Very Low would be less than the recommended 1 pipe diameter (corrugated turnout pipe) head above the pipe. Typically this was about 0.5 pipe diameter. The water level for the Low target was typically 1 pipe diameter above the top of the corrugated pipe. The Middle through Very High upstream head ratings were incrementally increased up to the maximum water level that could be safely obtained with the flume wall heights. For the 0.61 m gate there is no Very High upstream head since the large gate size limited the maximum water level that could be achieved in the flume.

During each of the tests shown in Table 1, the gate openings were changed. For the 0.30 m gate, gate openings in increments of 0.025 m were used from 0.025 m to fully open. For the 0.46 m and 0.61 m gates, gate openings in increments of 0.05 m (2 inches) were used from 0.05 m open to fully open.



**Table 1. Tests conducted under low supply channel velocity for the meter gate testing.**

Gate Type	Nominal Gate Size (m)	Relative Upstream Head	Relative Head Loss	Upstream Head ( $H_1$ ) Range (m)	$\Delta H$ Range (m)
Armco	0.30	Very Low	Very Small	0.251 - 0.111	0.062 - 0.041
Armco	0.30	Very Low	Small	0.251 - 0.111	0.108 - 0.086
Armco	0.30	Low	Medium	0.383 - 0.359	0.182 - 0.131
Armco	0.30	Middle	Small	0.643 - 0.595	0.351 - 0.305
Armco	0.30	Middle	Medium	0.643 - 0.595	0.427 - 0.378
Armco	0.30	Middle	Large	0.643 - 0.595	0.181 - 0.107
Armco	0.30	High	Small	0.97 - 0.845	0.446 - 0.369
Armco	0.30	High	Medium	0.97 - 0.845	0.661 - 0.613
Armco	0.30	High	Large	0.97 - 0.845	0.347 - 0.310
Armco	0.30	Very High	Medium	1.235 - 1.194	0.599 - 0.563
Armco	0.30	Very High	Large	1.235 - 1.194	0.863 - 0.736
Armco	0.46	Very Low	Small	0.241 - 0.203	0.070 - 0.011
Armco	0.46	Low	Medium	0.489 - 0.457	0.216 - 0.156
Armco	0.46	Middle	Small	0.692 - 0.597	0.048 - 0.032
Armco	0.46	Middle	Medium	0.692 - 0.597	0.310 - 0.263
Armco	0.46	Very High	Small	0.953 - 0.806	0.185 - 0.116
Armco	0.46	Very High	Large	0.953 - 0.806	0.401 - 0.358
Armco	0.46	Very High	Very Large	0.953 - 0.806	0.589 - 0.538
Armco	0.61	Very Low	Small	0.359 - 0.283	0.051 - 0.025
Armco	0.61	Low	Medium	0.448 - 0.427	0.212 - 0.172
Armco	0.61	Middle	Small	0.694 - 0.594	0.054 - 0.029
Armco	0.61	Middle	Medium	0.694 - 0.594	0.348 - 0.260
Armco	0.61	Middle	Large	0.694 - 0.594	0.435 - 0.396
Armco	0.61	High	Small	0.953 - 0.841	0.147 - 0.112
Armco	0.61	High	Large	0.953 - 0.841	0.435 - 0.357
Armco	0.61	High	Very Large	0.953 - 0.841	0.666 - 0.540
Rectangular	0.46	Very Low	Small	0.246 - 0.232	0.059 - 0.027
Rectangular	0.46	Low	Small	0.416 - 0.322	0.151 - 0.062
Rectangular	0.46	Standard	Small	0.73 - 0.457	0.191 - 0.143
Rectangular	0.46	Standard	Large	0.73 - 0.457	0.262 - 0.19
Rectangular	0.46	High	Small	0.66 - 0.584	0.319 - 0.184
Rectangular	0.46	High	Large	0.66 - 0.584	0.353 - 0.266
Rectangular	0.46	Very High	Small	0.819 - 0.775	0.323 - 0.22
Rectangular	0.46	Very High	Large	0.819 - 0.775	0.573 - 0.305
Rectangular	0.61	Very Low	Small	0.449 - 0.249	0.054 - 0.038
Rectangular	0.61	Very Low	Medium	0.449 - 0.249	0.263 - 0.151
Rectangular	0.61	Low	Medium	0.529 - 0.379	0.263 - 0.151
Rectangular	0.61	Low	Large	0.529 - 0.379	0.382 - 0.309
Rectangular	0.61	Standard	Small	0.7 - 0.667	0.051 - 0.03
Rectangular	0.61	Standard	Medium	0.7 - 0.667	0.221 - 0.171
Rectangular	0.61	Standard	Large	0.7 - 0.667	0.407 - 0.305
Rectangular	0.61	High	Small	0.798 - 0.745	0.049 - 0.032
Rectangular	0.61	High	Medium	0.798 - 0.745	0.215 - 0.167
Rectangular	0.61	High	Large	0.798 - 0.745	0.438 - 0.329

In addition to the tests shown in Table 1, Table 2 lists the different velocities and Froude Numbers ( $F_1$ ) tested. In general there were 3 sets of tests conducted for each scenario listed in Table 1 (Low, Medium, and High supply channel flow). Table 2 lists the range of the velocities and  $F_1$  (computed based on Eq. 4) tested for each of the upstream head conditions.

The supply channel velocity upstream of the meter gate was varied between 0.071 m/s and 0.941 m/s (0.23 ft/s and 3.09 ft/s, respectively). The maximum flow possible in the Cal Poly flume was 0.85 m<sup>3</sup>/s (30 cfs), so the maximum velocity was limited for the larger gate sizes because of minimum depths that could be tested. Thereby, the highest velocities and Froude numbers occurred at the lowest upstream depth scenarios for the smaller gate sizes.

Most irrigation distribution canals in California are earthen, and typically have velocities less than 0.91 m/s (3 ft/s). Concrete (or other lined) canals can have velocities greater than this, although many used for irrigation deliveries remain at 0.91 m/s (3 ft/s) or less (Scobey 1939). Therefore, even with the limited testing velocities, the results presented here will be applicable for many (if not most) meter gate installations.

**Table 2. Range of supply channel depths, velocity, and Froude numbers evaluated for each gate type and size. The round gate type refers to the Armco-Type gate.**

Gate Type	Nominal Gate Size (m)	Relative Upstream Head	Upstream Channel Depth ( $d$ ) Range (m)	Upstream Channel Velocity ( $V$ ) Range (m/s)	Upstream Channel $F_1$ Range
Round	0.3	Low	0.789 - 0.865	0.078 - 0.941	0.027 - 0.309
Round	0.3	Middle	1.078 - 1.133	0.120 - 0.633	0.036 - 0.193
Round	0.3	High	1.318 - 1.453	0.075 - 0.520	0.020 - 0.145
Round	0.3	Very High	1.670 - 1.721	0.075 - 0.404	0.018 - 0.099
Round	0.46	Very Low	0.813 - 0.902	0.122 - 0.769	0.043 - 0.259
Round	0.46	Low	1.014 - 1.340	0.255 - 0.693	0.078 - 0.220
Round	0.46	Middle	0.597 - 1.305	0.071 - 0.558	0.020 - 0.160
Round	0.46	Very High	1.416 - 1.562	0.081 - 0.461	0.022 - 0.121
Round	0.61	Very Low	1.019 - 1.114	0.157 - 0.658	0.050 - 0.201
Round	0.61	Low	1.164 - 1.284	0.164 - 0.561	0.048 - 0.165
Round	0.61	Middle	1.343 - 1.467	0.083 - 0.509	0.022 - 0.138
Round	0.61	High	1.513 - 1.743	0.086 - 0.454	0.022 - 0.118
Rectangular	0.46	Very Low	0.841 - 0.857	0.090 - 0.739	0.025 - 0.256
Rectangular	0.46	Low	0.932 - 1.030	0.105 - 0.581	0.033 - 0.184
Rectangular	0.46	Middle	1.067 - 1.340	0.134 - 0.474	0.039 - 0.142
Rectangular	0.46	High	1.194 - 1.311	0.127 - 0.498	0.036 - 0.140
Rectangular	0.46	Very High	1.384 - 1.545	0.085 - 0.417	0.023 - 0.108
Rectangular	0.61	Very Low	1.013 - 1.199	0.090 - 0.657	0.025 - 0.208
Rectangular	0.61	Low	1.116 - 1.321	0.107 - 0.481	0.030 - 0.138
Rectangular	0.61	Middle	1.373 - 1.437	0.079 - 0.521	0.021 - 0.142
Rectangular	0.61	High	1.481 - 1.641	0.083 - 0.471	0.022 - 0.123

### Evaluation of Flow Computed from Different Calibration Sources

Coefficient of discharge ( $C_d$ ) values were computed for each net gate opening under each scenario in Table 1. Relationships between various testing parameters and the  $C_d$  will be discussed. The uncertainty of the new  $C_d$  values as well as the original Armco tables were evaluated by examining the percent error between the actual flow rate measured and that determined using the new rating or Armco table values. Percent error is computed as:

$$E_{Q_i} = \frac{Q_i - Q}{Q} \times 100 \quad (11)$$

Where  $E_{Q_i}$  is the percent error between the estimated flow ( $Q_i$ ) and the actual flow ( $Q$ ). The estimated flow ( $Q_i$ ) was based on the Armco Rating Table ( $Q_{Armco}$ ) for the round gates and the  $C_d$  from the USBR Flow Measurement Manual for the rectangular gates ( $Q_{Rect}$ ). The  $Q_{Improved}$  was computed from the new  $C_d$  values developed from this work. The instantaneous flow measurement relative expanded uncertainty (95% confidence level) was developed based on multiple independent tests with the same gate at each gate opening for the existing Armco tables and the flow rate using  $C_d$  values from this study. Standard uncertainty of the meter gate ( $U_Q$ ) was computed as the standard deviation of the error ( $Q_i - Q$ ) at each gate opening. A coverage factor of  $k = 2$  (i.e.,  $\pm 2$  standard deviations) was applied for the expanded uncertainty to the 95% confidence level ( $U_{Q_{95}}$ ) as:

$$U_{Q_{95}} = 2U \quad (12)$$

The relative expanded uncertainty ( $RU_{95}$ ) was computed as:

$$RU_{95} = \frac{U_{Q_{95}}}{Q_{mean}} \quad (13)$$

Where  $Q_{mean}$  is the mean flow rate for the tests for that gate opening. More discussion on the methods used can be found in a number of references (Taylor and Kuyatt 1994; USBR 1997; Lozano et al. 2009).

Values from hardcopy Armco tables (Armco Steel Corporation 1975) provided by Fresno Valves and Casting, Inc. were entered into a spreadsheet.  $Q_{Armco}$  was determined for each net gate opening and  $\Delta H$  by linear interpolation between the two closest  $\Delta H$  values for each net gate opening.

The USBR Flow Measurement Manual contains a graph showing the recommended  $C_d$  value (based on the full pipe area) for rectangular gates. This  $C_d$  value was used to compute the flow rate ( $Q_{rect}$ ) to compare with the actual flow. This will also be compared to the new  $C_d$  values obtained from this work.

## RESULTS AND DISCUSSION

One of the primary reasons for testing the same gates under a variety of conditions, even those outside of the ranges shown in the Armco Flow Measurement Tables, is to examine the limitations for the accurate use of meter gates. Potential relationships exist between  $\Delta H$ ,  $H_1$ , Reynolds number in the turnout pipe ( $Re_{pipe}$ ), fraction of net gate opening ( $A_o/A_p$ ) and supply channel Froude number ( $F_1$ ).

### *Correlation between $C_d$ and Testing Variables*

The  $C_d$  computed for all scenarios is shown in Figure 6 for each of the three Armco-type gate sizes examined related to gate opening area. All gate sizes show variability in  $C_d$  values at the low gate openings. This was also found by Summers (1951) with round gates, and others have reported greater uncertainty at smaller gate openings with rectangular orifice experiments (Lozano et al. 2009).

Figure 6a (0.30 m gate) also shows significant variability in  $C_d$  values at different fractions of gate opening areas. This variability can be attributed to the Low and Very Low upstream head ( $H_1$ ) conditions where the upstream head was less than 0.5 times the pipe diameter. However, the 0.46 m and 0.61 m gates performed well for upstream heads as low as 0.5 times the pipe diameter. The “Tests Not Excluded” in Figure 6a, b, and c represent  $C_d$  values without upstream head values below 0.5 times the pipe diameter above the top of the pipe for the 0.30 m gate and gate openings below 20% for all gates. Additionally, several of the  $C_d$  values in Figure 6a (0.30 m gate) at gates openings of 30% not associated with Very Low upstream head were above 1.0. While this is theoretically not possible since there must be energy loss, there are several possible reasons for the inconsistency. Gate leakage is one possibility since the leakage would be a higher percentage of the total flow at the lower gate opening. Measurement error is another possibility. Finally, it should be noted that Eq. 2 and 3 assume hydrostatic conditions at the upstream and downstream measurement locations. However, these conditions may not necessarily be assumed at the 0.305 m pressure tap location, specifically at the lower gate opening when the velocity in the vertical direction may be significant close to the gate. This will be investigated in future work. At this point measurement error will be assumed and the values above 1.0 have been removed from further analysis.

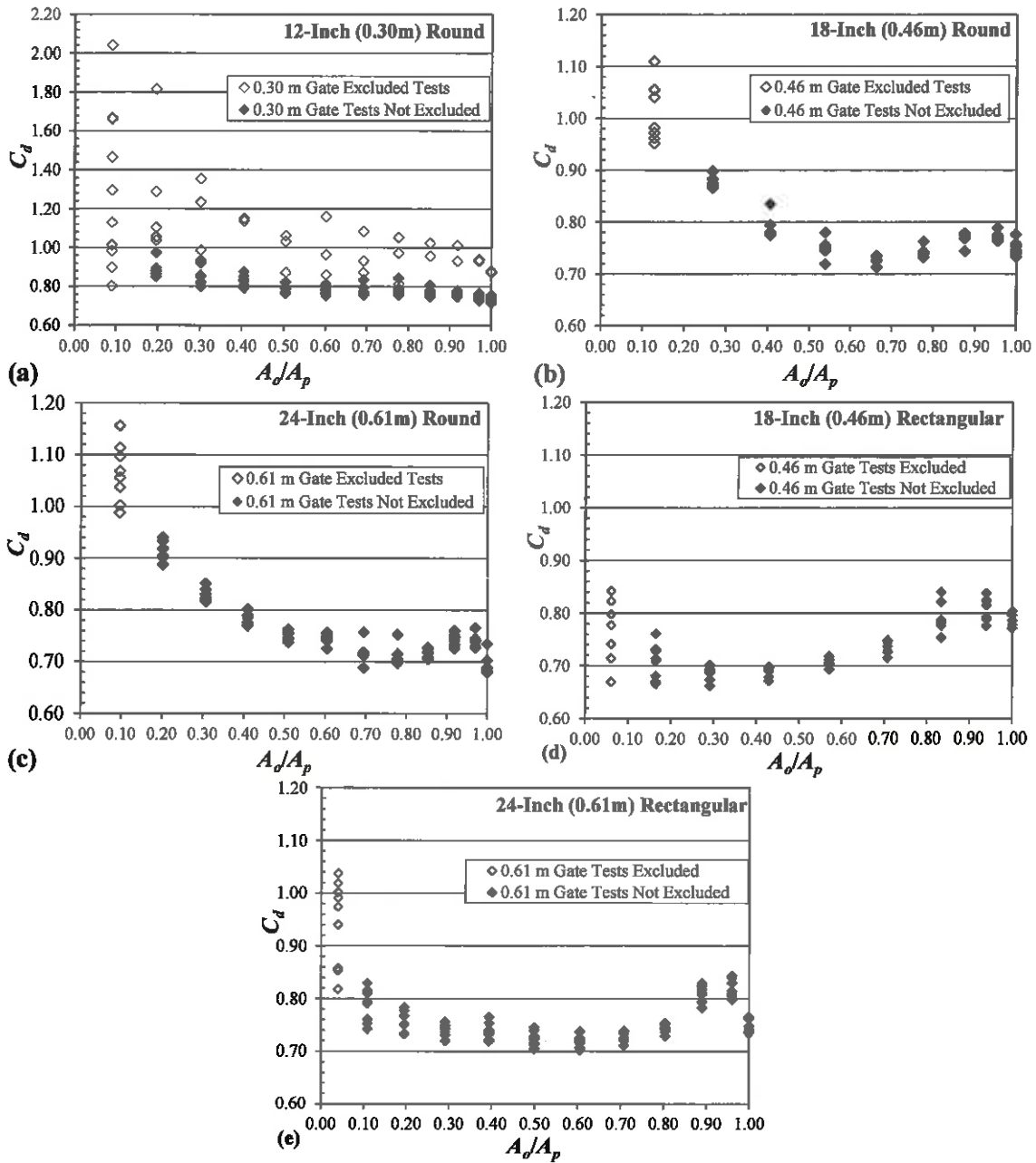


Figure 6. Coefficient of discharge ( $C_d$ ) for the three round gates (a-c) and the two rectangular gates (d-e) for the low supply channel velocity testing with the downstream head measurement taken 12-inches (0.305 m) downstream of the face of the gate.

Multiple regression analysis was examined on the non-excluded data relating  $A_o/A_p$ ,  $H_1/D$  and  $\Delta H/H_1$  supply channel Froude number ( $F_1$ ) to  $C_d$  using the model:

$$\hat{C}_d = \beta_6 \left( \frac{A_o}{A_p} \right)^3 + \beta_5 \left( \frac{A_o}{A_p} \right)^2 + \beta_4 \left( \frac{A_o}{A_p} \right) + \beta_3 \left( \frac{H_1}{D} \right) + \beta_2 \left( \frac{\Delta H}{H_1} \right) + \beta_1 (F_1) + \beta_0 \quad (14)$$

where,  $\hat{C}_d$  is the predicted discharge coefficient,  $\beta_0$  through  $\beta_6$  are the regression coefficients,  $A_o/A_p$  is the relative gate opening,  $H_1/D$  is relative upstream head,  $\Delta H/H_1$  is relative change headloss, and  $F_1$  the supply channel Froude number. Residual analysis was used to confirm the assumptions (normality, homoscedasticity, and independence of the errors) required for the multiple regression. The multiple regression coefficients and corresponding P-values for each gate size tested are shown in Table 3.

**Table 3. Multiple regression coefficients and corresponding P-values for each gate size tested**

Predictor	Coefficient	0.30 m Round <sup>a</sup>		0.46 m Round <sup>b</sup>		0.61 m Round <sup>c</sup>		0.46 m Rectangular <sup>d</sup>		0.61 m Rectangular <sup>e</sup>	
		Coefficient	P-value	Coefficient	P-value	Coefficient	P-value	Coefficient	P-value	Coefficient	P-value
$(A_o/A_p)^3$	$\beta_6$	-1.324	0.000	-1.041	0.000	-0.589	0.001	-1.484	0.000	-0.299	0.049
$(A_o/A_p)^2$	$\beta_5$	2.745	0.000	2.555	0.000	1.536	0.000	3.014	0.000	0.881	0.002
$(A_o/A_p)$	$\beta_4$	-1.911	0.000	-2.031	0.000	-1.359	0.000	-1.733	0.000	-0.679	0.000
$H_1/D$	$\beta_3$	-0.001	0.686	-0.016	0.000	-0.022	0.000	-0.005	0.327	-0.002	0.817
$\Delta H/H_1$	$\beta_2$	0.023	0.008	0.003	0.721	0.007	0.306	0.006	0.657	-0.013	0.213
$F_1$	$\beta_1$	0.054	0.031	-0.086	0.022	-0.108	0.023	-0.047	0.255	-0.051	0.371
Constant	$\beta_0$	1.213	0.000	1.293	0.000	1.155	0.000	0.995	0.000	0.890	0.000

Note: P-values >0.01 indicate the variable does not influence  $C_d$  at an  $\alpha$ -level = 0.01

<sup>a</sup>  $R^2 = 76.7\%$

<sup>b</sup>  $R^2 = 86.1\%$

<sup>c</sup>  $R^2 = 77.9\%$

<sup>d</sup>  $R^2 = 78.5\%$

<sup>e</sup>  $R^2 = 36.4\%$

It can be concluded that  $A_o/A_p$  (for all gates) and  $H_1/D$  (for the two larger round gates) have some influence on  $C_d$ , while statistically,  $\Delta H/H_1$  and  $F_1$  do not affect  $C_d$  at an  $\alpha$ -level of 0.01. We do not recommend using this multiple regression to compute the  $C_d$  values because the  $H_1/D$  and  $A_o/A_p$  values that may be used could be outside of the values used in the multiple regression. Alternative recommendations for determining  $C_d$  values will be discussed.

Relative upstream head ( $H_1/D$  or upstream head above the pipe divided by the pipe diameter) did have some influence on  $C_d$  (Table 3) for the two larger round gates although the coefficients are low, indicating the influence is relatively small (2-3% when  $H_1/D$  is included using Eq. 12 compared to without). This is represented visually in Figure 6, which shows a relatively constant  $C_d$  for the same relative gate opening. As indicated in Figure 5, the lower gate openings generally had higher  $C_d$  values. Figure 7 shows that the upstream head above the top of the pipe ( $H_1$ ) as low as 0.5 times the pipe diameter performed similar to higher heads at and above the recommended minimum head of 1 pipe diameter above the pipe. This indicates that it should be possible to obtain accurate flow measurement in situations where upstream head is less than the recommended 1 pipe diameter, but it should be greater than 0.5 times the pipe diameter.

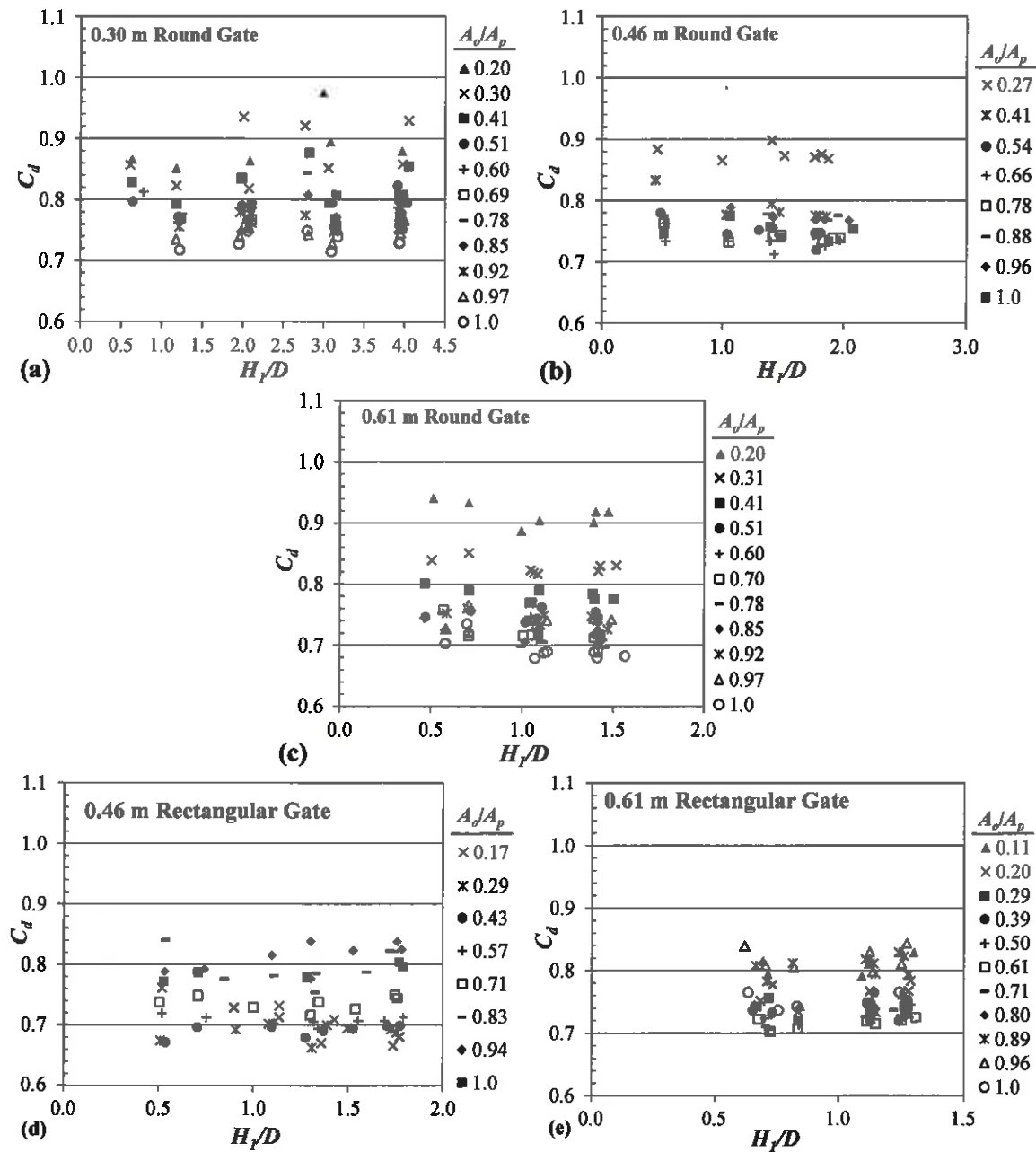


Figure 7.  $C_d$  related to the fraction of upstream head to turnout pipe diameter for the low supply channel velocity scenarios.

Figure 8 shows the relation between relative head loss ( $\Delta H/H_1$ ) to  $C_d$ . From this data, there is no indication that discharge coefficient is negatively influenced by increased headloss. Therefore, the current recommendation of a maximum  $\Delta H$  of 0.46 m (18 inches) can be exceeded at least with the gates tested in this study. For the 0.30 m gate (12-inch), 0.46 m gate (18-inch), and 0.61 m (24-inch) gate, maximum  $\Delta H$  values of 0.86 m, 0.59 m, and 0.67 m were tested, respectively.

During testing  $\Delta H$  values were attempted at  $\Delta H/H_1$  greater than 0.75-0.8. However, at this point the downstream head in the stilling well would typically drop below the level of the pipe and readings could not be made at the 0.305 m stilling well (or the closer wells). In field applications, this issue would lead to improper measurements or an inability to take the downstream head measurement.

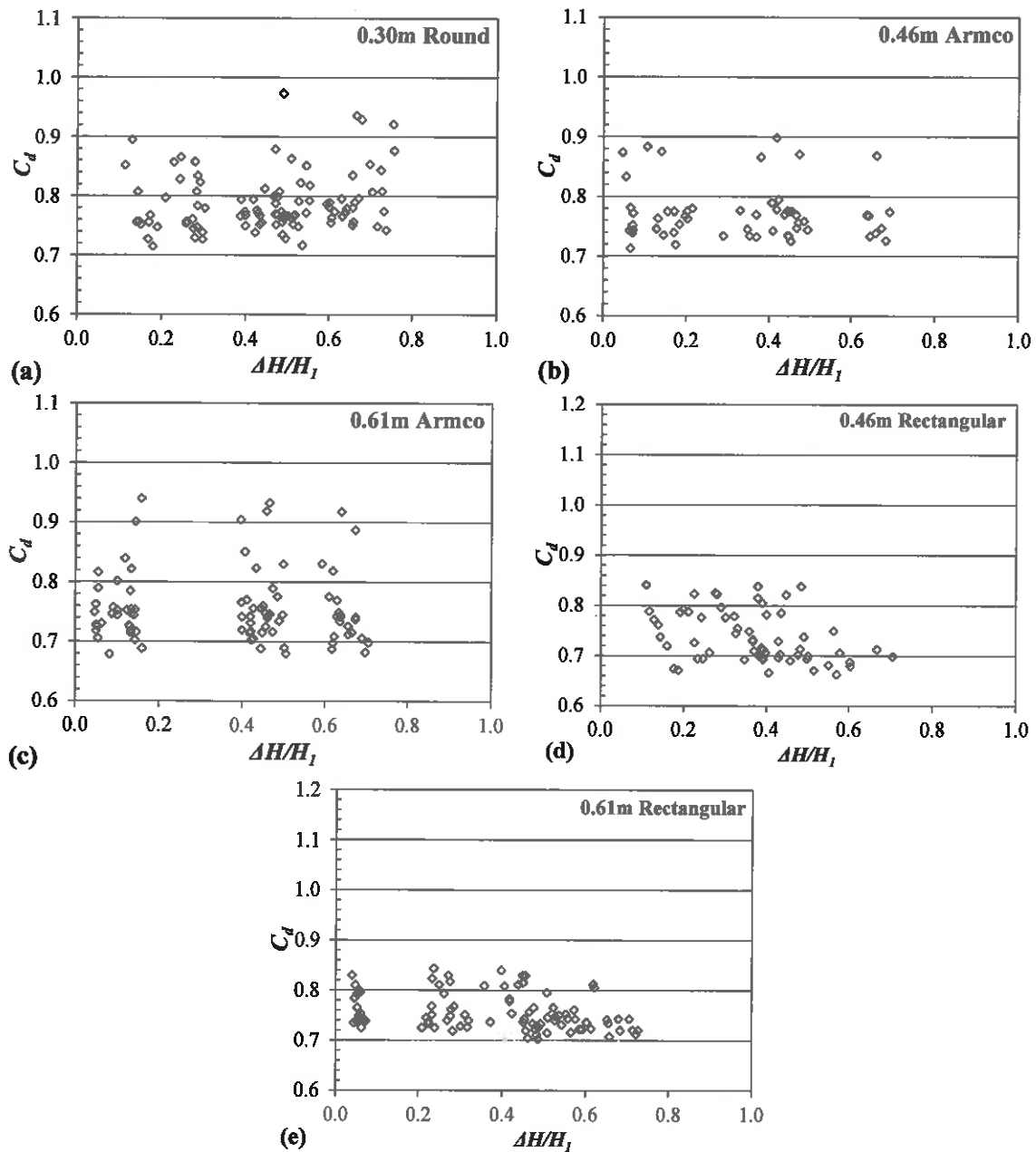


Figure 8.  $C_d$  related to the relative change in head ( $\Delta H/H_1$ ) for all gates tested for the low supply channel velocity with the downstream head measured at the 0.30 m (12-inch) pressure tap location.



Figure 9 shows the relationship between Reynolds numbers in the corrugated pipe ( $Re_{pipe}$ ) to  $C_d$ . The correlation between  $C_d$  values and Reynolds numbers can be attributed to the high correlation between  $C_d$  and relative gate openings (Figure 6). Therefore, this variable was not investigated independently (because it is not independent of gate opening). The data is shown only for general information.

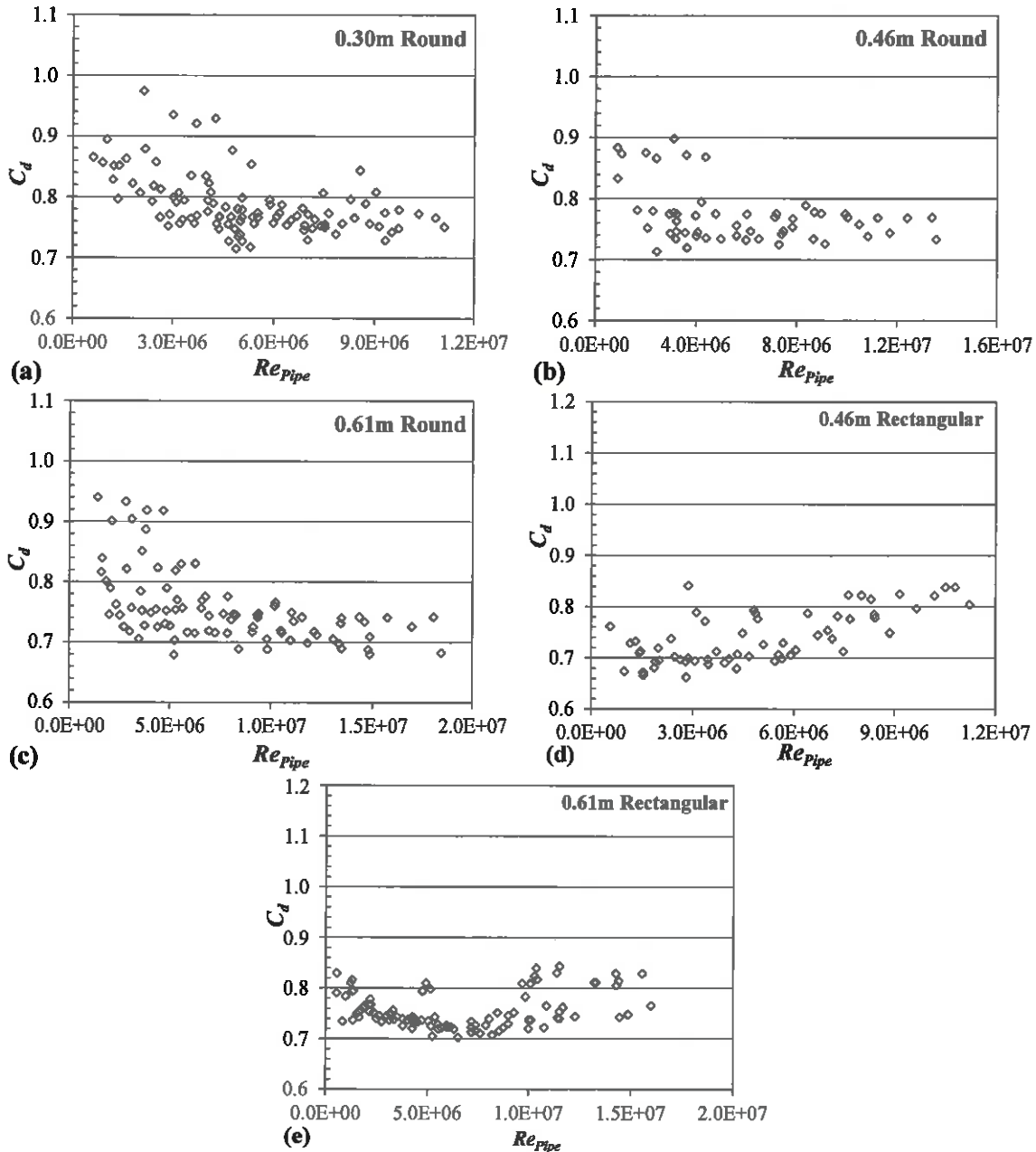
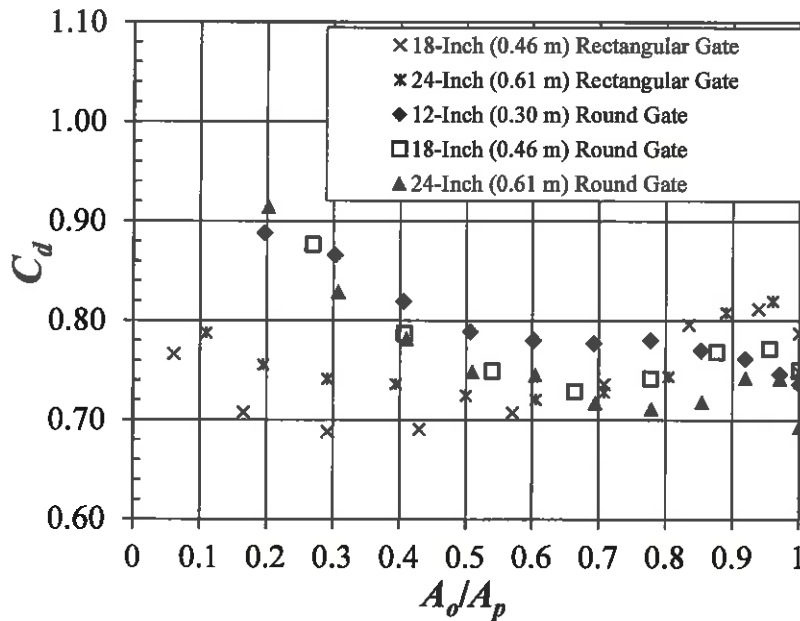


Figure 9. Relationship between  $C_d$  and Reynolds Number in the discharge pipe for the low velocity testing conditions and the downstream head measurement at the 0.305 m (12-inch) pressure tap location.

### ***ITRC New Coefficient of Discharge ( $C_d$ ) Values***

The average  $C_d$  for each gate, and each gate opening fraction, was averaged to develop the final  $C_d$  recommended for using. Only the low supply channel velocity tests were used for the recommended  $C_d$  values. The higher supply channel velocities will be investigated in a subsequent section. Figure 10 and Table 4 contain the final recommended  $C_d$  values by fraction of gate opening ( $A_o/A_p$ ).

Figure 10 shows the average  $C_d$  by fraction of gate opening area for each gate. Interestingly, the  $C_d$  values for the 0.46 m and 0.61 m gates are similar when open areas are less than 0.75 (75%). The  $C_d$  for the 0.30 m gate is consistently higher than the larger gates when the net open areas are less than 0.75 (75%). At net open areas greater than 0.75, the  $C_d$  values seem to consistently stay at approximately 0.75. For most of the gates there is a dip in  $C_d$  when the gate reaches full open ( $A_o/A_p = 1.0$ ). This is likely due to hydraulic effects as water enters the pipe, with and without gate obstruction, influencing the pressure at the 0.305 m stilling well.



**Figure 10. Average discharge coefficient by gate opening for the different gate sizes and types investigated based on the 0.305 m (12-inch) pressure tap location for downstream head measurements.**

Table 4 shows  $C_d$  by actual gate opening ( $y$ ) in meters and inches, fraction of gate opening ( $y/y_p$ ) and fraction of opening area ( $A_o/A_p$ ). Since it is common to have tables showing the net opening (as opposed to fraction of open area), this table presents information to help users implement this information. As previously mentioned, using a regression equation to “fit” the  $C_d$  by gate opening or fraction of open area (or any other variable) is NOT recommended. A more appropriate method would be to simply interpolate (linear interpolation) between the  $C_d$  values in Table 4.

**Table 4. Recommended new  $C_d$  values from this study by net gate opening ( $y$ ) and fraction of net opening area ( $A_o/A_p$ ).**

Gate Size	$y$ (m)	$y$ (inches)	$y/y_o$	$A_o/A_p$	ITRC $C_d$
0.30 m 12-inch Round	0.051	2	0.167	0.197	0.958
	0.076	3	0.250	0.303	0.878
	0.102	4	0.333	0.406	0.819
	0.127	5	0.417	0.506	0.789
	0.152	6	0.500	0.602	0.780
	0.178	7	0.583	0.693	0.770
	0.203	8	0.667	0.777	0.780
	0.229	9	0.750	0.853	0.770
	0.254	10	0.833	0.919	0.762
	0.279	11	0.917	0.971	0.746
	0.305	12	1.000	1.000	0.736
	0.46 m 18-inch Round	0.102	4	0.222	0.269
0.152		6	0.333	0.407	0.787
0.203		8	0.444	0.540	0.749
0.254		10	0.556	0.664	0.729
0.305		12	0.667	0.778	0.742
0.356		14	0.778	0.877	0.769
0.406		16	0.889	0.956	0.772
0.457		18	1.000	1.000	0.750
0.61 m 24-inch Round	0.102	4	0.167	0.202	0.915
	0.152	6	0.250	0.307	0.829
	0.203	8	0.333	0.410	0.782
	0.254	10	0.417	0.510	0.749
	0.305	12	0.500	0.605	0.745
	0.356	14	0.583	0.695	0.717
	0.406	16	0.667	0.779	0.711
	0.457	18	0.750	0.854	0.718
	0.508	20	0.833	0.920	0.743
	0.559	22	0.917	0.971	0.741
0.610	24	1.000	1.000	0.692	
0.46 m 18-inch Rectangular	0.102	4	0.222	0.165	0.708
	0.152	6	0.333	0.292	0.688
	0.203	8	0.444	0.429	0.690
	0.254	10	0.556	0.571	0.707
	0.305	12	0.667	0.708	0.736
	0.356	14	0.778	0.835	0.796
	0.406	16	0.889	0.939	0.812
	0.457	18	1.000	1.000	0.788
0.61 m 24-inch Rectangular	0.102	4	0.167	0.110	0.788
	0.152	6	0.250	0.196	0.756
	0.203	8	0.333	0.292	0.741
	0.254	10	0.417	0.394	0.736
	0.305	12	0.500	0.500	0.725
	0.356	14	0.583	0.606	0.721
	0.406	16	0.667	0.708	0.728
	0.457	18	0.750	0.804	0.744
	0.508	20	0.833	0.890	0.808
	0.559	22	0.917	0.960	0.820
0.610	24	1.000	1.000	0.748	

### Error Using Original Rating Tables or Charts

Of importance to meter gate users is the uncertainty (accuracy) of the existing rating tables (specifically the Armco Rating Table Booklet) and the  $C_d$  values computed in this study shown in Figure 10. Only the three most commonly used gates were investigated. There is a variety of other sizes from 0.20 m to 1.22 m that have rating tables. If good agreement exists between the 0.30 m, 0.46 m, and 0.61 m Armco tables then it might be inferred that similar agreement exists for the other size gates.

Figure 11 shows the average relative error at each gate opening with the relative expanded (95% confidence level) uncertainty shown bounding the relative error.

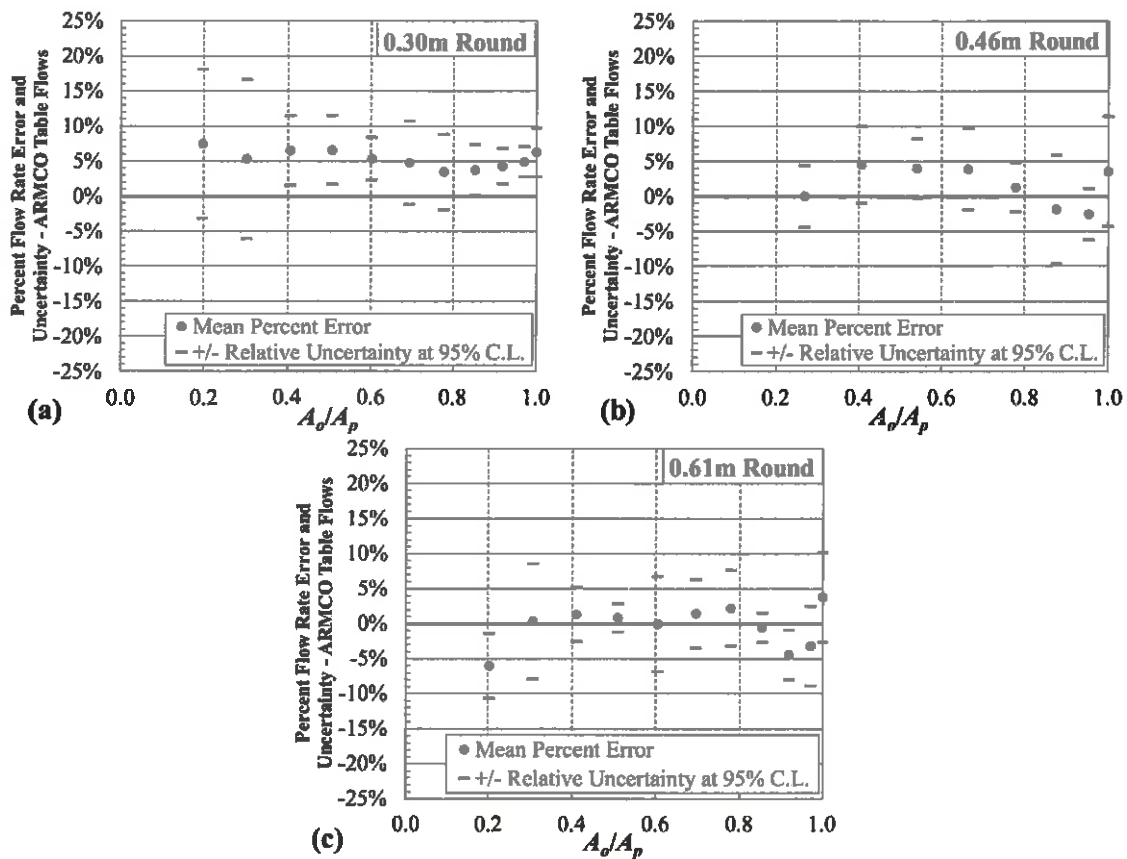


Figure 11. Comparison of percent error and uncertainty of discharge (Q) derived from the original Armco Meter Gate Rated Table to those measured in this study (low supply channel velocity tests).

As expected, the 0.30 m gate showed a high level of uncertainty at fractions of gate opening areas ( $A_o/A_p$ ) of less than 40% (gate opening of 0.10 m or 4"). This high variability in  $C_d$  values shown in Figure 6 indicates that there was significant variability in the measurements at these low gate openings. At  $A_o/A_p$  of greater than 40% for the 0.30 m gate the average relative error was within 7%. The Armco flow tables tended to over-estimate the flow rate for the 0.30 m and 0.46 m gates. This could be caused by the gate arrangement perpendicular to the supply channel flow instead of straight on as they were for the original tests used for the Armco tables. The biased error combined with the expanded uncertainty for the 0.30 m gate exceeds +10% for  $A_o/A_i$  of 50% and lower. It should be noted that the actual relative errors for  $A_o/A_i$  at 41% and 51% did not exceed  $\pm 10\%$ .

Armco table flow uncertainties ranged from -9% to +10% for  $A_o/A_p$  greater than 0.25 for the 0.46 m and 0.61 m gates. This is a good indication that these original Armco tables have been providing and will continue to provide good accuracy if the net gate opening area fractions remain greater than 0.25 – 0.35 and less than 1.0 (100% open). In most applications this is the case. No evaluation of other round meter gate tables has been conducted.

Figure 12 shows the percent discharge measurement error and expanded uncertainty from rectangular gates if the discharged was computed based the discharge coefficients from Figure 9-10 in the USBR Water Measurement Manual (USBR, 1997). For these figures, the downstream head for the  $\Delta H$  in Eq. 2 was taken at D/3 (1/3<sup>rd</sup> of the pipe diameter downstream from the face of the gate) as recommended in the manual. There is significant error using the USBR Water Measurement Manual (WMM)  $C_d$  for rectangular gates. While the 18-inch (0.46 m) gate was better than the 24-inch (0.61 m) gate, we still do not recommend using the USBR (1997)  $C_d$  values.

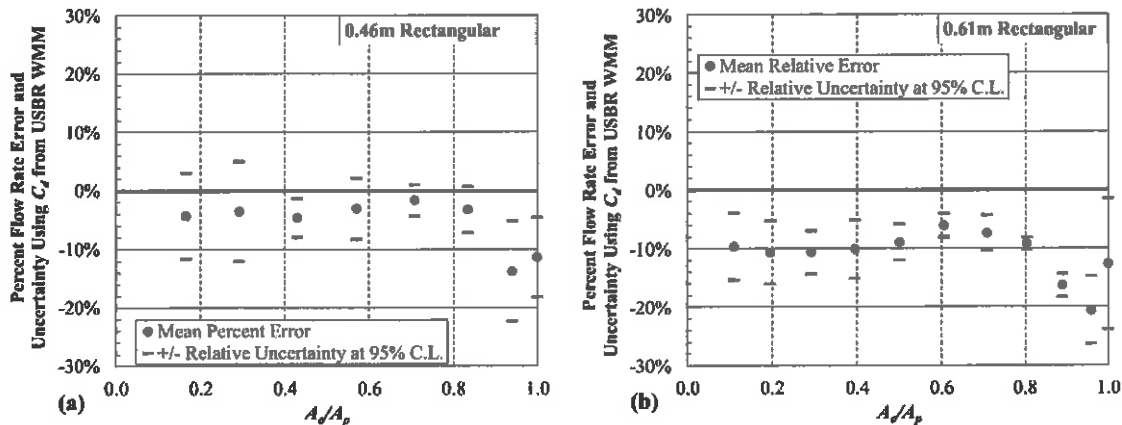


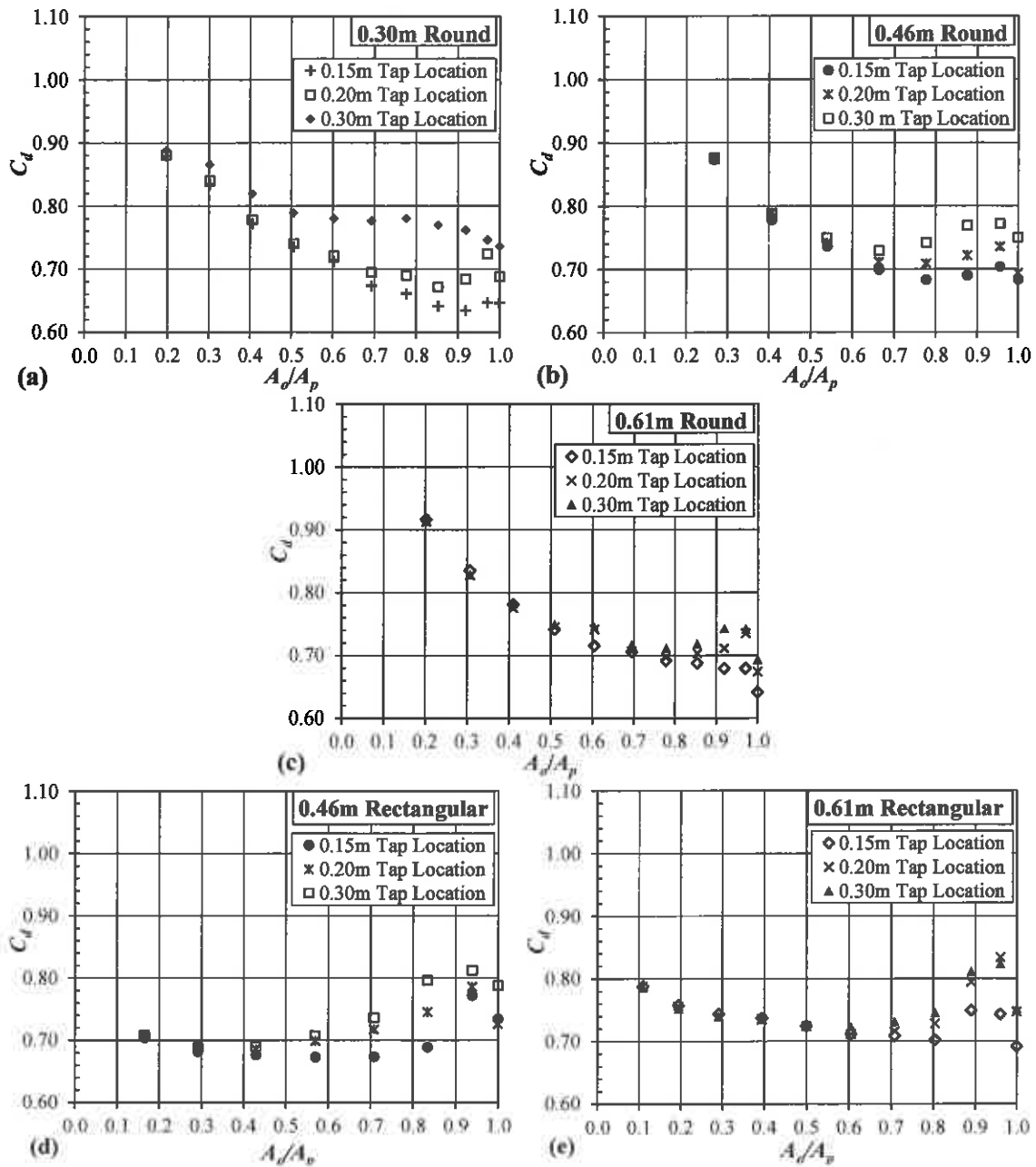
Figure 12. Rectangular gate percent error and uncertainty from using  $C_d$  values based on the USBR Water Measurement Manual Figure 9-10 to compute discharge compared to the actual flow measured in this study (low supply channel velocity tests)

The percent error is significantly reduced with the  $C_d$  values developed in this study (Table 4). This is to be expected since these  $C_d$  values were developed from this dataset. Of greater importance is the relative uncertainty, which remains within approximately  $\pm 7\%$  for  $A_o/A_p$  above 0.4 for the 0.30 m gate and typically within a  $\pm 5\%$  for  $A_o/A_p$  above 0.2 for other gates. It is recommended that users utilize the new  $C_d$  values developed in this study for the gates tested. For gates that have not been tested by ITRC, users should utilize the original Armco tables for all round gates on round pipes. For rectangular gates on round pipes, if gate sizes have not been tested by ITRC, onsite calibration or alternative flow measurement may be needed.

### ***Downstream Pressure Tap Location Influence***

Some impact of downstream pressure tap placement will be discussed since improper placement of the tap relative to the face of the gate is a common occurrence. Figure 5 shows the downstream pressure tap locations that were investigated. It is common for pressure taps to be placed closer than the recommended 0.305 m location. Figure 13 shows the  $C_d$  computed based on the 0.15 m (6 inches) and 0.20 m (8 inches) locations compared to the  $C_d$  computed from the recommended 0.30 m (12 inches) pressure tap location from Figure 10 for the five gates. It is interesting that for the 0.46 m and 0.61 m gates (both types) the  $C_d$  values are similar for relative fractions of net gate openings less than 0.75. For the 0.46 m and 0.61 m gates, improper placement should not cause a significant error unless gates are open more than 75%.

In contrast, the  $C_d$  values vary significantly for  $A_o/A_p$  greater than 0.4 for the 12-inch (0.30 m) round gate. The significantly lower  $C_d$  values are a result of greater  $\Delta H$  measured at the 0.15 m and 0.2 m wells. If a rating table based on the 0.305 m stilling well location were used with a tap location at 0.15 m or 0.2 m, the resulting flow rate would be overestimated. This indicates that if  $C_d$  values or Armco tables are used for the 0.30 m gate, the pressure tap location should be moved or a correction factor should be applied to correct for the difference. For the 12-inch (0.30 m) round gate ONLY; if stilling wells are located closer than 0.2 m to the face of the gate, the flow rates from tables should be multiplied by a correction factor ( $F_{tap}$ ) of 0.95 for gates openings less than or equal to 0.13 m (5 inches), by  $F_{tap} = 0.89$  for gate openings between 0.13 m and 0.23 m (9 inches), and  $F_{tap} = 0.86$  for gate openings greater than 0.23 m (although openings more than 75% are not recommended).



**Figure 13. Influence of pressure tap location (6-inches (0.15 m) and 8 inches (0.20 m) downstream from the back face of the gate) on  $C_d$ , compared to the  $C_d$  computed using the standard 12 inches (0.30 m) pressure tap location.**

### ***Supply Channel Velocity Influence***

Results from the multiple regression analysis indicated that supply channel Froude number did not seem have a significant influence on meter gate flow for the velocities tested (Table 3). To investigate how these additional tests and variable supply channel velocities influenced the uncertainty, the additional tests at the higher channel velocities were combined with the original low channel velocity tests shown previously.

The percent error was computed by comparing the computed flow through the meter gate using  $C_d$  values from Figure 10 to the measured flow through the meter gates with different supply channel velocities. Figure 14 (a-e) shows the results of this evaluation. The uncertainty is similar to those for recommended meter gate operation (within a  $\pm 5\%$  with gate openings typically between 25% and 75%). However, the uncertainty increased for the 0.46 m and 0.61 m round and rectangular gates at gate openings above 75%. In general the relative error also increased slightly at these gate openings, indicating that the recommended  $C_d$  values resulted in a slight overestimation of the flow rate (0-2%). Although, overall, the higher supply channel velocity did not have a significant influence, at larger gate openings (above 75%) there may be greater impacts. This could be a result of increased variability in measurements due to hydraulics at the entrance of the pipe (which is why it is recommended to design meter gates to operate between 25% and 75% open).

Overall, we do not believe that any adjustments or corrections are needed based on supply channel velocity (up to say 3 ft/s).



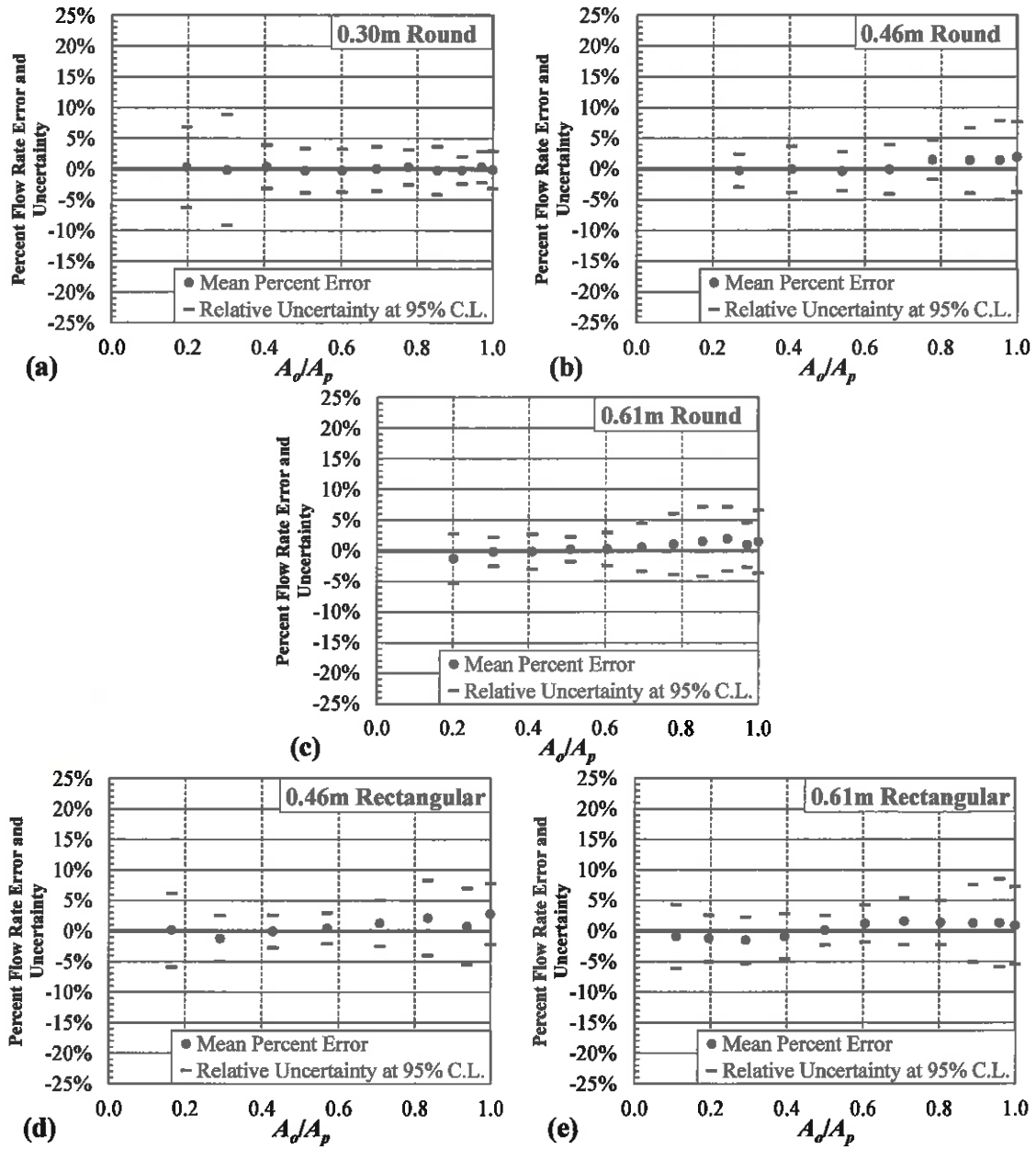


Figure 14. Percent flow rate error and uncertainty using  $C_d$  values from the low channel velocity tests for all supply channel velocities.

## CONCLUSION

The meter gate can be an accurate flow measurement device if installed and operated correctly. If the recommendations in the *Recommendations* section and *Attachment B* are followed, users of improved rating tables (*Attachment C*) for the 0.30 m (12-inch), 0.46 m (18-inch), and 0.61 m (24-inch) gates can expect the relative uncertainty ( $U_{Q\_new}$ ) to be better than  $\pm 5$  to  $\pm 7\%$  at the 95% confidence level. For other gate sizes the continued use of the Armco Flow Measurement Tables (Armco Steel Corporation 1975) is recommended with an uncertainty ( $U_{Q\_Armco}$ ) of better than  $\pm 10\%$  at a 95% confidence level if recommendations are followed. Uncertainties are lower than the required 10.7% instantaneous flow rate uncertainty required for SBx7-7.

The uncertainty is significantly less using the  $C_d$  values from this study compared to the original Armco tables. Therefore, new rating tables from  $C_d$  values developed in this study will be provided to users in digital format by the Irrigation Training and Research Center at California Polytechnic State University, San Luis Obispo ([www.itrc.org](http://www.itrc.org)). Until the remaining gates can be tested and improved  $C_d$  values developed, Armco Flow Measurement Tables will be made available in digital format, and will be replaced as gates are tested in the future and new tables are developed.

Future evaluations are necessary to develop  $C_d$  values and expanded tables for other Armco-type gates and gate sizes. Since the Waterman C-10 and XCAD X-Gate have very similar designs as the Fresno Valves and Casting, Inc. 101C gate, it is anticipated that the same rating tables can be used for these gates as well. Additional work is needed to confirm this. The Cal Poly ITRC meter gate testing facility is currently capable of testing gates up to 0.762 m (30 inches) in size.

Additionally, research is needed to investigate other uncertainties used to develop the overall volumetric uncertainty. Namely, the change in backpressure or downstream water level variations ( $U_{Hd}$ ) and potential uncertainty related to durations ( $U_T$ ) should be examined.

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**ATTACHMENT A**  
***Photos of Meter Gate Testing Facility***  
***Construction***

## **Attachment A**

### **Photos of Meter Gate Testing Facility Construction**

Modifications were made to the Cal Poly Water Resources Facility Irrigation Training and Research Facility flume to test and calibrate the meter gates with perpendicular side flow. The facilities include:

- A new pipeline connection from the upstream reservoir to increase the inlet capacity (Figure A-1)
- A drain connection box to the drain pipe that returns the flow to the recirculation facility so that water can be recirculated at high flow (Figure A-1).
- A main spill box where water flows out of the pipe downstream of the meter gate. Water levels in this box will be varied as part of the study so that we can test different downstream conditions (Figure A-2).
- One 24-inch pipeline and one 18-inch pipeline have been constructed and installed with calibrated magnetic meters between the main spill box and the drain connection box to provide accurate flow measurement readings for gate calibration (Figure A-2).
- Manifold connection from the two magmeter pipelines into a 30-inch steel pipe that connects to the drain pipe connection box (Figure A-2).



**Figure A-1. Adding a 30" pipe to the head of the flume to increase the capacity into the flume (left)**



**Figure A-2. Construction of the drain pipe connection box (right). Students in BRAE 433 (Fall Quarter) helped design and construct the bottom concrete slab shown in the top photo.**



**Figure A-3. Initial construction of main spill box that the meter gate pipeline drains into.**





**Figure A-4. Construction of the main spill box and 24” and 18” pipelines with magmeters**



**Figure A-5. Weir in flume to maintain the water level upstream of the gate. The photos show a longer weir than was actually used. Only the two bays on the downstream end were used in the actual testing.**



**Figure A-6. Completed testing setup after construction.**

**ATTACHMENT B**  
*Practical Guide for Meter Gates*

## Attachment B



*moving water in new directions*

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### **Practical Guide for Meter Gates**

by

**Dr. Charles Burt and Dr. Daniel Howes**

**Rev June 30, 2015**

#### **Background**

This document contains brief instructions on the use of special round canal gates called “meter gates” for flow measurement. A meter gate differs from a traditional canal gate turnout because it has a hole in the top of the pipe attached to a stilling well downstream of the gate so that the downstream water level can be measured.

Meter gates have been used since the early 1900’s for flow measurement in addition to on-off control. Recent research conducted by the authors at the Irrigation Training and Research Center has shown that the existing tables for “Armco”-type meter gates, published after the 1950’s, provide good accuracy for flow measurement (if measurements are made correctly).

Armco-type meter gates include round gates from Fresno Valve and Casting (101), Waterman (C-10), and X-CAD (model unknown) gates. In order to properly use these gates, a hole (5/8 to 3/4 inch in diameter) must be drilled in the pipe 12 inches downstream of the back face of the gate (or at the top of a corrugation as close to 12 inches as possible). This hole must be attached to a stilling well at least 6 inches in diameter that protrudes up to the elevation of the top of the gate frame.

**Figure B-1** shows a common meter gate design drawing.

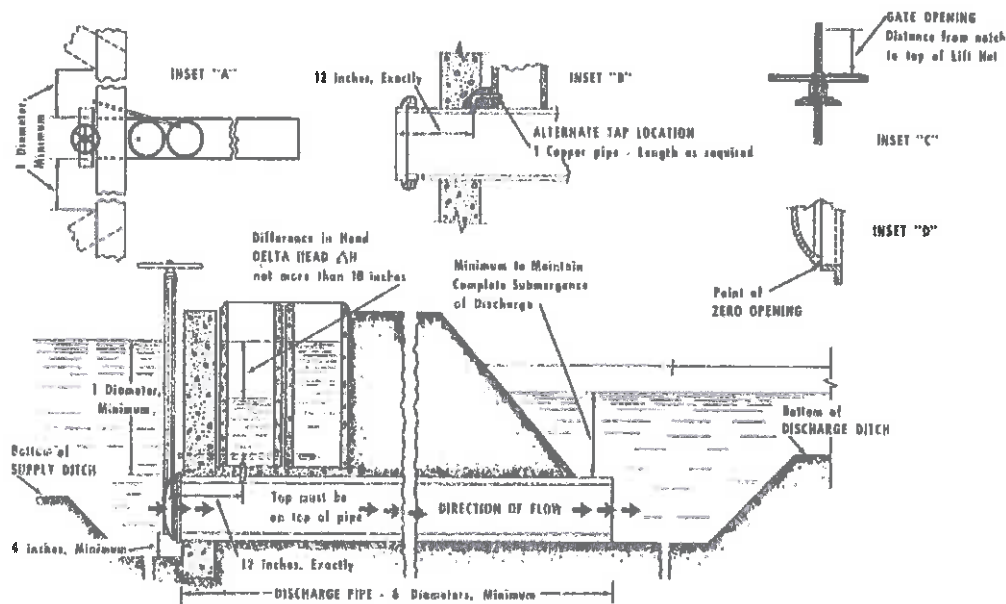


Figure B-1. Meter gate drawing used by various manufacturers, USBR, etc.

### ITRC Research

ITRC evaluated the calibration of a variety of Armco-type round and square gates to determine if published “meter gate” calibration tables are accurate. These gates were installed at the ITRC gate calibration facility (Figure B-2). The gate calibration facility is set up so that the turnout gate is perpendicular to the main supply channel flow, which is typical in field installations.



Figure B-2. ITRC gate calibration facility

### *Summary of ITRC Research Results*

- 1) A high level of accuracy (+/-5%) was found if all of the following conditions are met:
  - a. Gate opening range:  $20\% < \text{Gate Opening} < 75\%$
  - b. Upstream submergence  $> 0.5D$  (where D is the gate diameter)
  - c. Stilling well location was 4” to 12” downstream of the face of the gate
- 2) The distance downstream of the gate at which the stilling well is located (as long as it is within the 4” to 12” range) does not have a significant effect on the flow rate obtained using the tables **unless** the gate is **open** more than 70-75% (percent of fully open).

- 3) The preliminary evaluation of tangential supply channel flow velocity did not seem to have a significant impact on the flow through the turnout gates. Supply channel velocities up to 1.9 feet per second (fps) were examined in this evaluation.
- 4) Higher uncertainty (error) occurred at smaller gate openings.
- 5) Optimum range of operation for the highest accuracy was an opening between 20% and 75% under most conditions. Smaller gate openings seemed to be more problematic than larger gate openings.
- 6) One issue that is not discussed here but was apparent was the submergence (water level) in the supply canal above the turnout pipeline. Care should be taken to ensure that the water level upstream of the top of the turnout pipe remains above  $(0.5 \times \text{gate diameter})$ . The USBR standard is  $(1 \times \text{gate diameter})$ .

### Correction for Stilling Well 4" from Gate

Standard flow tables are based on a stilling well located 12" downstream of the back of the gate. Stilling well measurements were made at multiple locations downstream of the gate to analyze the effects of stilling well location. It was found that, at gate openings less than 70% open, there was **minimal** impact on the change in head from any stilling well closer than 12" to the gate. Once the gate reached an opening of 70% or greater, the  $\Delta H$  measurement measured at the closer stilling wells (e.g., at 4") began to vary depending on gate size resulting in more significant error.

On average, at gate openings above 75%, the flow rate for a 4" stilling well was 8%-10% greater than the value shown on a 12" stilling well-based table. This adjustment could be applied in the case where gates must be opened more than 75%.

### Practical Details

Figure B-3 shows one recommended configuration for a meter gate. There are some significant differences between Figures B-1 and B-3. With meter gates, "the devil is in the details". These are discussed on the next few pages.

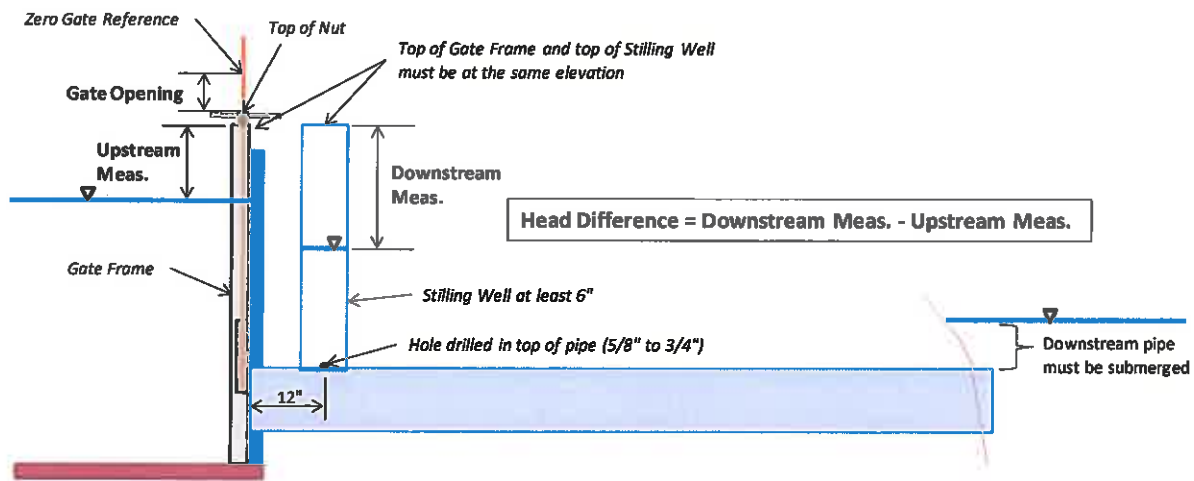
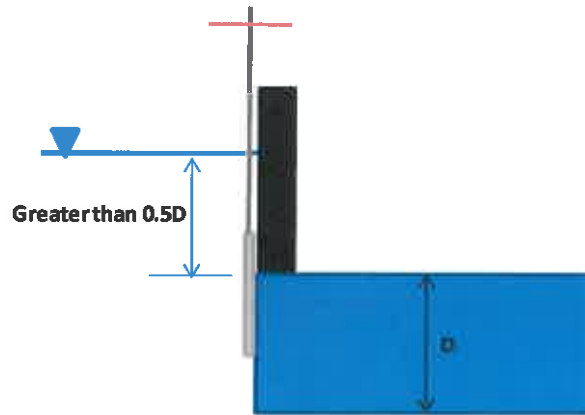


Figure B-3. ITRC recommendation for proper meter gate installation. These have been improved by Glenn Colusa ID with pre-cast concrete structures.

*Practical Detail #1* – The pipe downstream of the meter gate needs to be full. The water level needs to rise to some measurable level in the downstream stilling well.

*Practical Detail #2* – Sufficient upstream submergence is needed. The required water level in the canal, above the top of the pipe, must be at least  $\frac{1}{2}$  of the gate (or pipe) diameter. In other words, if there is a 12" pipe, the water level in the supply canal needs to be at least 6" above the top of the pipe.



**Figure B-4. Recommended upstream submergence above the gate to ensure accurate flow measurement**

*Practical Detail #3* – All of the calibration charts require knowledge of the gate opening, as measured by the shaft opening. The “zero” gate opening must be properly determined and marked on the gate shaft. This is not a trivial detail. Specific points are:

1. All measurements of gate opening, as well as the initial marking, must be made after the gate stem has been lifted (opened). This is because there is some “slop” or movement between the shaft and the gate itself.
2. The gate stem will move up some distance before the gate plate itself reaches the bottom of the pipe. The charts depend on knowing the gate opening, not the movement from the gate seating position. The gate must be closed beyond the bottom of the pipe to seal off completely. That sealed position is not the “zero” position.
3. There must be some specific way to measure the shaft position when the bottom of the gate just barely clears the bottom of the pipe – in other words, when there is a “zero opening”. This is fairly easy to set and measure if the canal is full. The gate is opened until a narrow strip of paper can be inserted into the crack. **Figure B-5** shows photos taken at San Luis Canal Company of a customized tool that is used to detect the actual gate opening, but a similar device can be used to detect the initial “cracking (zero) open” position..





**Figure B-5. Special tool to detect actual gate opening.**

4. The shaft needs to be marked in a clear manner so that operators know where the “zero” opening is for the gate when they open the gate. **Figure B-6** shows a properly cut notch. It has a sharp bottom edge that was cut with a grinding wheel so that the bottom of the cut is at the same elevation as the top of the bushing. Notice from the color on the shaft that the shaft can be lowered from this position to properly seat the gate.

The operator will measure from the bottom of cut to the top of the bushing, when the gate is open, to determine the gate opening. This is always measured after an “uplift” action.

*Practical Detail #4* – The stilling well needs to have sufficient diameter to dampen the turbulence, and so that operators can see into it. ITRC recommends a stilling well of 6” – 8” diameter, with an access hole of about 5/8” or 3/4” diameter.



**Figure B-6. Proper cut in shaft to mark the “zero” opening**



**Figure B-7. Stilling well is located the correct distance downstream of the gate, but is so small that there will be tremendous surging (up/down movement), and operators cannot see the water surface**

*Practical Detail #5* – The stilling well does not need to be centered over the access hole in the top of the discharge pipe. In general, it is good to have the stilling well close to the gate frame/bulkhead, so that it can be supported.

*Practical Detail #6* – Make it easy to measure the difference in head (between the water level in the canal, and the water level in the stilling well). In other words, use the same datum (elevation) for both measurements. **Figure B-8** shows a stilling well with the top correctly placed at the same elevation as the gate frame, and with a proper diameter. **The top of the stilling well should be at the same elevation as the top of the gate frame (where the bottom of the nut rests), or have the same elevation as another reference point.** Then the upstream measurement should be taken from the top of the gate frame to the water level. The downstream measurement should be taken from the top of the stilling well to the water level. The head difference is the difference between the upstream and downstream water levels.



**Figure B-8. Stilling well installed on an existing discharge pipe. It is constructed of PVC pipe that is too thin for long life, but it serves as an example of the correct diameter, position, and height.**



**Figure B-9.** An old type of dual-stilling well commonly found in Central California irrigation districts. One stilling well was connected to the canal, and the second was directly over the discharge pipe. The idea of measuring down into both stilling wells from the same center point was good, but the top of the stilling well was so close to the ground surface that road maintenance quickly filled these stilling wells with dirt. Also, the side connection between the canal stilling well and the canal itself was too difficult to clean.



**Figure B-10.** This stilling well is properly located, but it has too small a diameter. The operator also needs to know the elevation difference between the top of the stilling well and the gate frame, which requires an extra computation to determine the difference in head across the gate.



**Figure B-11. Correct height of stilling well to match top of gate frame. However, the diameter is too small. Steel pipe material is good**



**Figure B-12. Large diameter stilling well, with cover to minimize having it fill with dirt from the road. Strong concrete, with the rim of the stilling well at the same elevation as the bulkhead top.**

The tables on the next few pages show the key measurements needed to properly use a meter gate. The gate opening should be measured from the top of the gate opening nut to a zero gate opening reference. **As mentioned previously, the zero gate opening reference should be marked with a grinder at the gate opening nut on the shaft when the gate is just open enough to breach the bottom of the pipe.**

### **The Glenn-Colusa ID Configuration**

Glenn-Colusa ID (GCID) worked with Briggs (a local pre-cast concrete structure company near Willows, CA) to incorporate the ITRC recommendations into a pre-cast structure. The following figures illustrate their solution, which appears to be excellent.



**Figure B-13. GCID meter gate at Briggs**



**Figure B-14. Pre-cast metergate ready for transport**



**Figure B-15. Installation of GCID meter gate**



**Figure B-16. Final concrete for GCID meter gate, showing downstream pre-cast outlet box.**

**Table B-1. Approximate cost for GCID meter gate installation**

**18" X 6' H-Metergate with precast concrete tailbox**

1). MATERIAL COST	QUANTITY		UNIT	COST/UNIT	COST
STILLING WELL 12" X 6' H W/ LID	1		EA	\$340	\$340
PRE/FAB 6' H BRIGG'S (metergate box)	1		EA	\$470	\$470
PLYWOOD			EA	\$1	\$0
SNAP TIES			YD	\$1	\$0
PIPE (18" PLASTIC)	25		FL.	\$11	\$278
GATE 18" 5' FRAME	1		EA	\$1,270	\$1,270
CONCRETE	3		YD	\$105	\$315
METER BOX 5' H (tailbox)	1		EA	\$550	\$550
TOTAL COST =					\$3,223

2). LABOR COST	QUANTITY	HRS/JOB	COST/HR	UNIT	COST/UNIT	COST
TOTAL COST =						\$650

3). EQUIPMENT COST	QUANTITY	HRS/JOB	COST/HR*	UNIT	COST/UNIT	COST
BACKHOE			\$25.00	P/H		\$0
EXCAVATOR	1	\$1.00	\$50.00	P/H		\$50
LONG REACH			\$50.00	P/H		\$0
TRUCK	1	\$12.00	\$25.00	P/H		\$300
TRANSPORT	1	\$1.00	\$44.00	P/H		\$44
CRANE			\$50.00	P/H		\$0
PICKUPS	1	\$4.00	\$5.50	P/H		\$22
D-6 DOZER			\$35.00	P/H		\$0
D-4 DOZER			\$25.00	P/H		\$0
MISC.(WELDERS,PUMPS,GENERATORS )	1	\$4.00	\$8.00	P/H		\$32
TOTAL COST =						\$448
TOTAL HOURS =						22
<b>TOTAL =</b>						<b>\$4,321</b>

# **ATTACHMENT C**

## ***ITRC Water Measurement Tables***



ITRC Water Measurement Tables for  
**ROUND (Armco-Type) Gates**  
**on Round Pipes**  
Discharge Values in CFS







Preliminary Tables for  
**Round Gates on Round Pipes**  
Discharge Values in CFS

*These tables are from the original ARMCO  
Flow Measurement Tables and will be  
replaced as these gate sizes are tested by  
ITRC*

Armco-Type Meter Gate Tables - Preliminary

8-inch Round Gate

Head Difference (feet)	Net Gate Opening (feet)												
	0.17	0.21	0.25	0.29	0.33	0.38	0.42	0.46	0.50	0.54	0.58	0.63	0.67
0.08	0.27	0.32	0.38	0.42	0.46	0.51	0.55	0.57	0.59	0.61	0.62	0.63	0.64
0.10	0.30	0.36	0.42	0.46	0.51	0.56	0.60	0.63	0.65	0.68	0.70	0.71	0.71
0.13	0.32	0.39	0.46	0.50	0.56	0.61	0.67	0.69	0.72	0.75	0.77	0.78	0.78
0.15	0.35	0.42	0.49	0.54	0.60	0.66	0.72	0.75	0.78	0.81	0.83	0.84	0.85
0.17	0.37	0.44	0.52	0.58	0.64	0.70	0.76	0.80	0.83	0.86	0.89	0.90	0.91
0.19	0.39	0.46	0.54	0.61	0.67	0.74	0.80	0.84	0.88	0.92	0.94	0.96	0.97
0.21	0.41	0.49	0.57	0.64	0.70	0.77	0.85	0.89	0.93	0.96	1.00	1.01	1.02
0.23	0.42	0.51	0.60	0.66	0.74	0.81	0.88	0.93	0.97	1.01	1.04	1.06	1.07
0.25	0.44	0.53	0.62	0.70	0.76	0.84	0.92	0.97	1.02	1.06	1.09	1.11	1.12
0.27	0.46	0.55	0.64	0.72	0.79	0.87	0.95	1.01	1.06	1.10	1.13	1.15	1.16
0.29	0.47	0.57	0.67	0.74	0.82	0.90	0.99	1.05	1.10	1.14	1.18	1.20	1.21
0.31	0.49	0.59	0.69	0.77	0.85	0.93	1.02	1.08	1.14	1.18	1.22	1.24	1.26
0.33	0.50	0.60	0.71	0.79	0.88	0.96	1.05	1.12	1.18	1.22	1.26	1.28	1.30
0.35	0.52	0.62	0.73	0.82	0.90	0.99	1.08	1.15	1.22	1.26	1.30	1.33	1.34
0.38	0.53	0.64	0.75	0.84	0.92	1.02	1.11	1.19	1.25	1.30	1.34	1.37	1.38
0.40	0.54	0.65	0.76	0.86	0.95	1.04	1.14	1.22	1.29	1.34	1.38	1.41	1.42
0.42	0.56	0.67	0.78	0.88	0.97	1.07	1.17	1.25	1.32	1.37	1.42	1.44	1.46
0.46	0.58	0.70	0.81	0.91	1.01	1.12	1.22	1.31	1.38	1.44	1.49	1.52	1.54
0.50	0.60	0.72	0.84	0.95	1.06	1.17	1.27	1.36	1.44	1.50	1.55	1.58	1.60
0.54	0.62	0.75	0.87	0.99	1.10	1.22	1.32	1.42	1.50	1.56	1.61	1.65	1.67
0.58	0.64	0.77	0.90	1.03	1.15	1.26	1.37	1.47	1.55	1.62	1.67	1.71	1.74
0.63	0.66	0.80	0.94	1.06	1.19	1.31	1.42	1.53	1.61	1.68	1.73	1.77	1.80
0.67	0.68	0.82	0.96	1.10	1.22	1.35	1.47	1.58	1.66	1.73	1.79	1.83	1.86
0.71	0.70	0.85	1.00	1.13	1.26	1.39	1.52	1.62	1.71	1.78	1.84	1.88	1.92
0.75	0.72	0.87	1.02	1.16	1.30	1.43	1.56	14.67	1.76	1.84	1.89	1.94	1.97
0.79	0.74	0.90	1.05	1.19	1.33	1.47	1.60	1.72	1.81	1.89	1.94	1.99	2.02
0.83	0.76	0.92	1.08	1.22	1.37	1.51	1.64	1.76	1.85	1.94	1.99	2.04	2.08
0.92	0.79	0.96	1.13	1.28	1.44	1.58	1.72	1.85	1.94	2.03	2.09	2.14	2.18
1.00	0.83	1.01	1.18	1.34	1.50	1.66	1.80	1.93	2.03	2.12	2.18	2.24	2.27
1.08	0.86	1.05	1.23	1.40	1.56	1.72	1.87	2.01	2.12	2.21	2.29	2.33	2.37
1.17	0.89	1.09	1.28	1.45	1.62	1.79	1.94	2.08	2.20	2.29	2.36	2.42	2.46
1.25	0.92	1.13	1.32	1.50	1.68	1.85	2.01	2.16	2.27	2.37	2.44	2.50	2.54
1.33	0.95	1.16	1.37	1.55	1.73	1.91	2.08	2.23	2.35	2.45	2.52	2.58	2.62
1.42	0.98	1.20	1.41	1.60	1.78	1.97	2.14	2.30	2.42	2.52	2.60	2.66	2.71
1.50	1.01	1.23	1.45	1.64	1.84	2.03	2.20	2.36	2.49	2.60	2.68	2.74	2.79

Armco-Type Meter Gate Tables - Preliminary

15-inch Round Gate

Head Difference (feet)	Discharge (cfs)																	
	0.17	0.21	0.25	0.29	0.33	0.38	0.42	0.46	0.50	0.58	0.67	0.75	0.83	0.92	1.00	1.08	1.17	1.25
0.08	0.46	0.57	0.66	0.75	0.83	0.91	0.98	1.07	1.14	1.30	1.43	1.58	1.71	1.84	1.94	2.04	2.13	2.18
0.10	0.51	0.62	0.73	0.83	0.92	1.02	1.09	1.19	1.27	1.44	1.59	1.75	1.90	2.05	2.17	2.29	2.38	2.43
0.13	0.55	0.67	0.79	0.91	1.00	1.11	1.19	1.30	1.38	1.57	1.74	1.91	2.08	2.24	2.38	2.51	2.62	2.67
0.15	0.59	0.72	0.85	0.98	1.08	1.19	1.28	1.39	1.49	1.68	1.87	2.06	2.24	2.41	2.57	2.72	2.83	2.90
0.17	0.63	0.77	0.90	1.04	1.15	1.27	1.37	1.48	1.59	1.79	1.99	2.20	2.39	2.58	2.75	2.90	3.03	3.09
0.19	0.67	0.81	0.95	1.10	1.22	1.34	1.45	1.57	1.68	1.89	2.11	2.33	2.54	2.73	2.91	3.07	3.22	3.28
0.21	0.70	0.85	1.00	1.15	1.28	1.41	1.53	1.65	1.76	1.99	2.22	2.45	2.68	2.87	3.07	3.24	3.40	3.46
0.23	0.73	0.89	1.05	1.20	1.33	1.48	1.60	1.73	1.84	2.09	2.33	2.57	2.81	3.01	3.21	3.40	3.57	3.64
0.25	0.76	0.93	1.09	1.25	1.38	1.54	1.67	1.80	1.92	2.18	2.43	2.69	2.93	3.14	3.35	3.54	3.73	3.81
0.27	0.79	0.97	1.13	1.29	1.43	1.60	1.73	1.87	2.00	2.27	2.53	2.80	3.05	3.27	3.49	3.68	3.88	3.97
0.29	0.82	1.00	1.17	1.33	1.48	1.65	1.79	1.94	2.08	2.36	2.63	2.90	3.17	3.39	3.62	3.82	4.01	4.11
0.31	0.85	1.03	1.21	1.37	1.53	1.70	1.85	2.01	2.15	2.44	2.72	3.00	3.28	3.51	3.75	3.96	4.14	4.25
0.33	0.88	1.06	1.25	1.41	1.58	1.75	1.91	2.07	2.22	2.52	2.81	3.10	3.39	3.63	3.87	4.09	4.27	4.39
0.35	0.91	1.09	1.29	1.45	1.63	1.80	1.97	2.13	2.29	2.60	2.90	3.20	3.49	3.74	3.99	4.21	4.40	4.53
0.38	0.93	1.12	1.32	1.49	1.68	1.85	2.03	2.19	2.36	2.68	2.98	3.29	3.59	3.85	4.10	4.33	4.53	4.67
0.40	0.95	1.15	1.35	1.53	1.73	1.90	2.09	2.25	2.42	2.75	3.06	3.38	3.69	3.96	4.21	4.45	4.65	4.80
0.42	0.97	1.18	1.38	1.57	1.77	1.95	2.14	2.31	2.48	2.82	3.14	3.47	3.79	4.06	4.32	4.57	4.77	4.92
0.46	1.01	1.23	1.44	1.64	1.85	2.05	2.24	2.43	2.60	2.96	3.30	3.63	3.97	4.26	4.54	4.79	5.00	5.14
0.50	1.05	1.28	1.50	1.71	1.93	2.14	2.34	2.54	2.72	3.09	3.44	3.79	4.15	4.44	4.74	5.00	5.22	5.36
0.54	1.09	1.33	1.56	1.78	2.01	2.23	2.44	2.64	2.83	3.22	3.58	3.95	4.32	4.62	4.93	5.20	5.43	5.58
0.58	1.13	1.38	1.62	1.85	2.09	2.31	2.53	2.74	2.93	3.34	3.72	4.10	4.48	4.79	5.11	5.40	5.64	5.79
0.63	1.17	1.42	1.68	1.92	2.16	2.39	2.62	2.84	3.03	3.46	3.85	4.25	4.64	4.96	5.29	5.59	5.84	5.99
0.67	1.21	1.46	1.73	1.98	2.23	2.47	2.71	2.93	3.13	3.57	3.98	4.39	4.79	5.13	5.47	5.78	6.03	6.19
0.71	1.24	1.50	1.78	2.04	2.30	2.55	2.79	3.02	3.23	3.68	4.10	4.52	4.93	5.29	5.64	5.95	6.22	6.38
0.75	1.27	1.54	1.83	2.10	2.37	2.62	2.87	3.11	3.33	3.79	4.22	4.65	5.07	5.44	5.80	6.12	6.40	6.56
0.79	1.30	1.58	1.88	2.16	2.43	2.69	2.95	3.19	3.42	3.89	4.34	4.78	5.21	5.59	5.96	6.29	6.58	6.74
0.83	1.33	1.62	1.93	2.22	2.49	2.76	3.03	3.27	3.51	3.99	4.45	4.91	5.35	5.73	6.11	6.46	6.75	6.92
0.92	1.39	1.70	2.03	2.32	2.61	2.90	3.17	3.43	3.68	4.18	4.66	5.14	5.61	6.01	6.41	6.77	7.07	7.26
1.00	1.45	1.78	2.12	2.42	2.73	3.03	3.31	3.59	3.84	4.37	4.87	5.37	5.86	6.29	6.70	7.07	7.39	7.59
1.08	1.50	1.85	2.21	2.52	2.84	3.15	3.45	3.73	4.00	4.55	5.07	5.59	6.10	6.54	6.97	7.36	7.69	7.89
1.17	1.55	1.92	2.29	2.62	2.95	3.27	3.58	3.87	4.15	4.72	5.26	5.80	6.34	6.79	7.24	7.64	7.98	8.19
1.25	1.60	1.99	2.37	2.71	3.05	3.38	3.70	4.01	4.30	4.88	5.44	6.00	6.56	7.03	7.49	7.91	8.26	8.47
1.33	1.65	2.05	2.45	2.80	3.15	3.49	3.82	4.14	4.44	5.04	5.62	6.20	6.77	7.26	7.73	8.17	8.53	8.75
1.42	1.70	2.11	2.52	2.89	3.25	3.60	3.94	4.27	4.57	5.20	5.80	6.39	6.98	7.48	7.97	8.42	8.80	9.02
1.50	1.75	2.17	2.59	2.97	3.34	3.70	4.05	4.39	4.70	5.35	5.96	6.58	7.18	7.69	8.20	8.66	9.05	9.28

16-inch Round Gate

Head Difference (feet)	Net Gate Opening (feet)																		
	0.17	0.21	0.25	0.29	0.33	0.38	0.42	0.46	0.50	0.58	0.67	0.75	0.83	0.92	1.00	1.08	1.17	1.25	1.33
0.08	0.49	0.59	0.70	0.79	0.89	0.97	1.05	1.14	1.22	1.37	1.53	1.68	1.83	1.96	2.10	2.24	2.35	2.43	2.47
0.10	0.55	0.66	0.77	0.88	0.98	1.08	1.16	1.27	1.36	1.52	1.70	1.87	2.04	2.19	2.34	2.50	2.63	2.72	2.78
0.13	0.59	0.72	0.84	0.96	1.07	1.18	1.27	1.39	1.48	1.66	1.86	2.04	2.22	2.39	2.56	2.74	2.89	2.98	3.05
0.15	0.63	0.77	0.90	1.03	1.15	1.27	1.37	1.49	1.59	1.79	1.99	2.20	2.40	2.57	2.76	2.96	3.12	3.23	3.31
0.17	0.67	0.82	0.96	1.10	1.23	1.35	1.46	1.59	1.69	1.91	2.12	2.34	2.56	2.74	2.94	3.16	3.31	3.46	3.54
0.19	0.71	0.86	1.02	1.16	1.30	1.43	1.54	1.68	1.79	2.02	2.25	2.48	2.71	2.91	3.11	3.33	3.50	3.66	3.75
0.21	0.75	0.90	1.07	1.21	1.36	1.50	1.62	1.76	1.89	2.13	2.37	2.61	2.85	3.08	3.28	3.50	3.68	3.85	3.95
0.23	0.78	0.94	1.12	1.26	1.42	1.57	1.69	1.84	1.98	2.23	2.49	2.74	2.99	3.23	3.44	3.67	3.86	4.03	4.14
0.25	0.81	0.98	1.16	1.31	1.48	1.64	1.76	1.92	2.06	2.33	2.60	2.86	3.12	3.37	3.59	3.83	4.04	4.20	4.33
0.27	0.84	1.02	1.20	1.36	1.54	1.70	1.84	2.00	2.14	2.43	2.71	2.98	3.25	3.51	3.74	3.99	4.20	4.37	4.51
0.29	0.87	1.06	1.24	1.41	1.59	1.75	1.91	2.08	2.22	2.52	2.81	3.09	3.37	3.64	3.88	4.14	4.36	4.53	4.69
0.31	0.90	1.09	1.28	1.46	1.64	1.81	1.98	2.15	2.30	2.61	2.91	3.20	3.49	3.77	4.02	4.28	4.52	4.69	4.86
0.33	0.93	1.12	1.32	1.51	1.69	1.87	2.05	2.22	2.38	2.69	3.00	3.31	3.61	3.89	4.15	4.42	4.67	4.85	5.02
0.35	0.96	1.15	1.36	1.56	1.74	1.93	2.11	2.29	2.45	2.77	3.09	3.41	3.72	4.01	4.28	4.56	4.81	5.00	5.18
0.38	0.99	1.18	1.40	1.61	1.79	1.99	2.17	2.36	2.52	2.85	3.18	3.51	3.83	4.13	4.40	4.69	4.95	5.15	5.32
0.40	1.02	1.21	1.44	1.65	1.84	2.04	2.23	2.42	2.59	2.93	3.27	3.61	3.94	4.24	4.52	4.82	5.09	5.29	5.46
0.42	1.04	1.24	1.48	1.69	1.89	2.09	2.29	2.48	2.66	3.01	3.36	3.70	4.04	4.35	4.64	4.95	5.22	5.43	5.59
0.46	1.08	1.30	1.55	1.76	1.98	2.19	2.40	2.60	2.79	3.16	3.52	3.88	4.24	4.56	4.87	5.19	5.47	5.69	5.85
0.50	1.12	1.36	1.61	1.83	2.07	2.28	2.50	2.71	2.91	3.30	3.68	4.05	4.42	4.76	5.08	5.42	5.71	5.94	6.10
0.54	1.16	1.41	1.67	1.90	2.15	2.37	2.60	2.82	3.03	3.43	3.83	4.21	4.60	4.96	5.29	5.64	5.95	6.18	6.35
0.58	1.20	1.46	1.73	1.97	2.23	2.46	2.70	2.93	3.14	3.56	3.97	4.37	4.77	5.15	5.49	5.85	6.18	6.41	6.59
0.63	1.24	1.51	1.79	2.04	2.31	2.55	2.80	3.04	3.25	3.69	4.11	4.53	4.94	5.33	5.68	6.06	6.39	6.64	6.82
0.67	1.28	1.56	1.85	2.11	2.39	2.63	2.89	3.14	3.36	3.81	4.25	4.68	5.11	5.50	5.87	6.26	6.60	6.86	7.05
0.71	1.31	1.60	1.90	2.18	2.46	2.71	2.98	3.24	3.46	3.93	4.38	4.82	5.26	5.67	6.05	6.45	6.81	7.07	7.27
0.75	1.34	1.64	1.95	2.24	2.53	2.79	3.07	3.33	3.56	4.04	4.51	4.96	5.41	5.83	6.23	6.64	7.01	7.27	7.48
0.79	1.37	1.68	2.00	2.30	2.60	2.87	3.15	3.42	3.66	4.15	4.63	5.10	5.56	5.99	6.40	6.83	7.20	7.47	7.68
0.83	1.40	1.72	2.05	2.36	2.67	2.95	3.23	3.51	3.75	4.26	4.75	5.23	5.71	6.15	6.56	7.00	7.39	7.67	7.88
0.92	1.46	1.80	2.15	2.48	2.80	3.09	3.39	3.68	3.93	4.46	4.98	5.48	5.98	6.45	6.88	7.34	7.74	8.04	8.26
1.00	1.52	1.88	2.25	2.59	2.92	3.23	3.54	3.84	4.11	4.66	5.20	5.73	6.25	6.74	7.19	7.66	8.09	8.40	8.63
1.08	1.58	1.96	2.34	2.69	3.04	3.36	3.68	4.00	4.28	4.85	5.41	5.96	6.50	7.01	7.48	7.98	8.41	8.74	8.98
1.17	1.64	2.04	2.43	2.79	3.15	3.49	3.82	4.15	4.44	5.03	5.61	6.19	6.75	7.28	7.76	8.27	8.73	9.07	9.32
1.25	1.70	2.11	2.51	2.89	3.26	3.61	3.96	4.29	4.60	5.21	5.81	6.40	6.99	7.54	8.04	8.56	9.04	9.39	9.65
1.33	1.76	2.18	2.59	2.99	3.37	3.73	4.09	4.43	4.75	5.38	6.00	6.61	7.22	7.79	8.30	8.85	9.34	9.70	9.96
1.42	1.81	2.25	2.67	3.08	3.48	3.84	4.22	4.57	4.90	5.55	6.19	6.82	7.44	8.03	8.56	9.13	9.63	10.00	10.27
1.50	1.86	2.31	2.75	3.16	3.58	3.95	4.34	4.70	5.04	5.71	6.37	7.01	7.65	8.25	8.80	9.39	9.90	10.28	10.56



Armco-type Meter Gate Tables - Preliminary

20-inch Round Gate

Head Difference (feet)	Net Gate Opening (feet)																						
	0.17	0.21	0.25	0.29	0.33	0.38	0.42	0.46	0.50	0.58	0.67	0.75	0.83	0.92	1.00	1.08	1.17	1.25	1.33	1.42	1.50	1.58	1.67
0.08	0.58	0.73	0.86	0.96	1.10	1.21	1.32	1.43	1.54	1.75	1.92	2.12	2.29	2.48	2.66	2.84	3.01	3.16	3.31	3.44	3.57	3.68	3.71
0.10	0.66	0.81	0.96	1.09	1.23	1.35	1.42	1.60	1.72	1.95	2.16	2.37	2.56	2.77	2.98	3.18	3.37	3.54	3.71	3.86	4.01	4.14	4.19
0.13	0.72	0.88	1.04	1.18	1.35	1.47	1.61	1.74	1.88	2.12	2.35	2.58	2.79	3.03	3.25	3.47	3.69	3.88	4.06	4.23	4.39	4.52	4.61
0.15	0.77	0.95	1.11	1.27	1.44	1.58	1.72	1.87	2.02	2.27	2.52	2.77	3.00	3.26	3.50	3.73	3.95	4.18	4.36	4.55	4.71	4.86	4.97
0.17	0.82	1.01	1.18	1.36	1.53	1.68	1.83	1.99	2.15	2.42	2.69	2.95	3.21	3.48	3.73	3.97	4.20	4.46	4.66	4.85	5.02	5.20	5.31
0.19	0.87	1.07	1.25	1.44	1.62	1.78	1.94	2.11	2.27	2.56	2.86	3.13	3.42	3.69	3.96	4.21	4.45	4.72	4.95	5.15	5.33	5.53	5.65
0.21	0.91	1.12	1.31	1.51	1.70	1.87	2.04	2.21	2.38	2.70	3.02	3.31	3.62	3.89	4.18	4.45	4.70	4.97	5.21	5.43	5.63	5.83	5.95
0.23	0.95	1.17	1.37	1.58	1.78	1.96	2.13	2.31	2.49	2.84	3.16	3.49	3.80	4.08	4.39	4.66	4.95	5.21	5.46	5.70	5.91	6.13	6.24
0.25	0.99	1.22	1.43	1.65	1.85	2.04	2.22	2.41	2.60	2.96	3.30	3.64	3.96	4.26	4.58	4.86	5.16	5.44	5.70	5.95	6.17	6.41	6.52
0.27	1.03	1.27	1.49	1.71	1.92	2.12	2.31	2.51	2.71	3.08	3.44	3.79	4.12	4.44	4.77	5.06	5.37	5.66	5.94	6.20	6.43	6.68	6.80
0.29	1.07	1.31	1.54	1.77	1.99	2.19	2.40	2.61	2.82	3.20	3.56	3.93	4.28	4.60	4.95	5.25	5.57	5.88	6.16	6.43	6.67	6.92	7.08
0.31	1.10	1.35	1.59	1.83	2.06	2.26	2.49	2.70	2.92	3.31	3.68	4.07	4.43	4.76	5.13	5.44	5.77	6.09	6.38	6.65	6.90	7.15	7.32
0.33	1.13	1.39	1.64	1.88	2.12	2.33	2.57	2.79	3.02	3.42	3.80	4.20	4.57	4.92	5.29	5.62	5.96	6.29	6.58	6.87	7.12	7.38	7.56
0.35	1.16	1.43	1.69	1.93	2.18	2.40	2.65	2.88	3.12	3.52	3.92	4.33	4.71	5.07	5.45	5.79	6.14	6.48	6.78	7.08	7.34	7.61	7.79
0.38	1.19	1.47	1.73	1.98	2.24	2.47	2.73	2.96	3.21	3.62	4.04	4.46	4.85	5.22	5.61	5.96	6.32	6.67	6.98	7.29	7.56	7.84	8.02
0.40	1.22	1.51	1.77	2.03	2.30	2.54	2.80	3.04	3.29	3.72	4.16	4.58	4.99	5.36	5.76	6.13	6.50	6.85	7.18	7.50	7.77	8.05	8.25
0.42	1.25	1.55	1.81	2.08	2.36	2.61	2.87	3.12	3.37	3.82	4.26	4.70	5.12	5.50	5.91	6.29	6.66	7.03	7.36	7.69	7.97	8.26	8.46
0.46	1.31	1.61	1.89	2.18	2.48	2.74	3.02	3.28	3.53	4.01	4.47	4.93	5.36	5.77	6.20	6.59	6.98	7.37	7.72	8.06	8.36	8.66	8.87
0.50	1.37	1.67	1.97	2.28	2.59	2.86	3.15	3.42	3.69	4.19	4.67	5.15	5.60	6.04	6.48	6.88	7.30	7.70	8.06	8.41	8.73	9.05	9.26
0.54	1.42	1.73	2.05	2.38	2.69	2.98	3.28	3.56	3.85	4.36	4.86	5.37	5.84	6.28	6.76	7.17	7.60	8.02	8.40	8.76	9.10	9.44	9.65
0.58	1.47	1.79	2.12	2.47	2.79	3.09	3.40	3.70	3.99	4.53	5.05	5.57	6.06	6.51	7.01	7.45	7.89	8.32	8.72	9.10	9.43	9.78	10.02
0.63	1.52	1.85	2.19	2.56	2.89	3.20	3.52	3.82	4.13	4.68	5.22	5.76	6.26	6.74	7.25	7.70	8.16	8.61	9.02	9.42	9.76	10.12	10.37
0.67	1.57	1.91	2.26	2.65	2.98	3.30	3.63	3.94	4.26	4.83	5.39	5.95	6.46	6.96	7.48	7.95	8.43	8.88	9.31	9.72	10.08	10.44	10.70
0.71	1.61	1.97	2.33	2.73	3.07	3.40	3.74	4.06	4.39	4.98	5.55	6.13	6.66	7.17	7.70	8.19	8.69	9.15	9.60	10.01	10.40	10.76	11.03
0.75	1.65	2.02	2.40	2.80	3.16	3.50	3.85	4.18	4.52	5.13	5.71	6.31	6.86	7.38	7.92	8.43	8.95	9.42	9.88	10.30	10.70	11.08	11.34
0.79	1.69	2.07	2.47	2.87	3.25	3.60	3.96	4.30	4.65	5.27	5.87	6.49	7.06	7.59	8.14	8.67	9.20	9.69	10.15	10.59	11.00	11.40	11.65
0.83	1.73	2.12	2.54	2.94	3.34	3.70	4.07	4.42	4.77	5.41	6.03	6.65	7.25	7.79	8.36	8.90	9.44	9.95	10.41	10.88	11.28	11.69	11.96
0.92	1.81	2.22	2.67	3.08	3.50	3.87	4.28	4.63	5.00	5.67	6.31	6.97	7.60	8.16	8.76	9.32	9.88	10.42	10.91	11.44	11.82	12.24	12.53
1.00	1.88	2.32	2.79	3.22	3.66	4.04	4.47	4.84	5.22	5.92	6.59	7.29	7.93	8.53	9.16	9.74	10.32	10.89	11.41	11.93	12.34	12.79	13.10
1.08	1.95	2.42	2.91	3.36	3.81	4.21	4.64	5.04	5.44	6.17	6.87	7.60	8.26	8.88	9.55	10.13	10.76	11.33	11.88	12.40	12.86	13.32	13.65
1.17	2.02	2.52	3.02	3.49	3.95	4.38	4.81	5.23	5.64	6.40	7.13	7.89	8.57	9.21	9.90	10.52	11.16	11.77	12.32	12.87	13.33	13.82	14.17
1.25	2.08	2.61	3.12	3.61	4.09	4.53	4.97	5.41	5.84	6.62	7.38	8.15	8.86	9.53	10.24	10.88	11.53	12.18	12.74	13.30	13.80	14.30	14.65
1.33	2.14	2.70	3.22	3.73	4.22	4.67	5.13	5.58	6.03	6.84	7.62	8.41	9.15	9.85	10.58	11.23	11.90	12.56	13.16	13.72	14.25	14.78	15.13
1.42	2.20	2.78	3.32	3.84	4.35	4.81	5.29	5.75	6.22	7.05	7.85	8.67	9.43	10.15	10.90	11.58	12.27	12.94	13.58	14.14	14.69	15.24	15.59
1.50	2.26	2.86	3.41	3.95	4.47	4.95	5.45	5.92	6.40	7.25	8.08	8.93	9.70	10.44	11.22	11.92	12.64	13.32	13.98	14.56	15.11	15.68	16.05





ITRC Water Measurement Tables for  
**RECTANGULAR Gates on**  
**Round Pipes**  
Discharge Values in CFS



ITRC Water Measurement Tables - 24" Rectangular Gate, Sillling Web located 12" dis of Back of Gate [ Blue center represents best accuracy range]

Table with columns for AH (feet), Discharge (cfs), and various flow measurement values. The Discharge (cfs) column is highlighted in blue. Values range from approximately 0.04 to 2.83 for AH and up to 9.96 for Discharge.

## FLOW MEASUREMENT OPTIONS FOR CANAL TURNOUTS

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### ABSTRACT

Volumetric record-keeping, billing, and allocations at irrigation district delivery points (turnouts) are the norm, rather than the exception for most California irrigation districts. However, many older districts are just beginning these efforts, and other districts are trying to improve existing hardware and procedures. Volumetric accounting with high accuracy and a reasonable price presents unique engineering challenges for irrigation districts because of the variety of existing structures and configurations at irrigation delivery points. Because it is likely that irrigation districts will attempt to utilize existing devices, or slightly modify them, there is a need for standardized installation and/or calibration. This paper discusses three efforts to adapt, improve, and/or calibrate existing technologies for flow rate and volumetric metering of canal turnouts.

### INTRODUCTION

In the most basic form, all irrigation turnouts, or delivery points, serve two purposes:

- Starting and stopping the flow of water
- Control of delivered flow rates – typically provided by a mechanism such as a valve or gate. In other cases, the turnout mechanism is adjusted wide open, and the turnout flow rate is determined by something such as the number of alfalfa valves or sprinklers open downstream.

Modern turnouts are also capable of:

- Flow measurement – an instantaneous quantification provided by various methods.
  - For some turnouts, a supplementary device measures the flow rate (with various levels of accuracy) and displays the result digitally or with an analog gauge.
  - More frequently, field measurements of the mechanism's opening, upstream and (sometimes) downstream water levels are applied to an equation or rating table. In these cases, the turnout structure itself is used as the flow measurement device, without auxiliary equipment.
- Volumetric totalizing – an accumulation of the flow measurement over time. The accumulation can be completed by either:
  - Automatically mechanical or electronic methods, or
  - Manually “averaging” multiple, discrete flow measurements over an irrigation event.

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Regulations now mandate that in the near future, many California agricultural irrigation turnouts must be configured to provide flow measurement and volumetric totalizing of delivered irrigation water. Furthermore, the measured quantities must also meet specific accuracy standards for new and existing flow measurement devices (CA SBX77 2009).

In most cases accurate flow measurement requires, among other things, satisfactory hydraulic conditions both upstream and downstream of the flow measurement location. For this reason, flumes are not recommended immediately downstream of a bend in the canal. Similarly, propeller meters are not recommended for installations immediately downstream of a partially closed butterfly valve. In these examples, it is unlikely that the instantaneous flow measurement would reflect the actual flow rate.

From an engineering perspective, achieving flow measurement and automatic volumetric totalizing within acceptable accuracy stipulations has become relatively straight-forward for most pipeline turnouts because:

- The hydraulic conditions upstream and downstream of the flow measurement device can be easily “standardized” with a length of straight pipe. The exact length of straight pipe required by each product is specified by the manufacturer. If there is too little room to fit straight pipe lengths or a skewed flow profile cannot be corrected with straight pipe, commercially available “straightening vanes” can be installed to correct poor upstream hydraulic conditions.
- The round pipe cross section provides a clean and an easily calculated flow area.
- There are numerous commercially available “flow meters” (utilizing various technologies) that provide flow measurement and *automatic* volumetric totalizing with more than acceptable accuracies. Many can also be delivered with factory calibration certificates traceable to the National Institute for Science and Technology (NIST).
- If the piping system is designed properly, the flow meter can be easily removed and re-calibrated by the manufacturer or other entities.
- Flow meters can be easily installed with standard, commercially available fittings.

For the reasons above, meeting flow measurement and volumetric totalizing regulations for new or existing pipeline turnouts has become more of an economic analysis than an engineering topic. A variety of irrigation districts simplify the challenge by requiring that farmers install accessible, properly installed magnetic or propeller meters downstream of their filter systems when the farmers install a drip/micro system.

Conversely, meeting flow measurement mandates for canal turnouts is more complex. Although there are good solutions for new canal turnouts, there are very few new canal turnouts being constructed and it is prohibitively expensive to replace each non-conforming structure at the district level. As such, the remainder of this paper will focus on the options for utilizing existing structures for flow measurement as well as options for retrofitting existing canal turnout structures to meet flow measurement regulatory obligations.



A major constraint for canal turnout flow measurement is access to existing physical configurations. In general, most canal turnout structures and accompanying gate/valve mechanisms are installed in the canal. The structure discharges into a buried pipe under a canal access road. The buried pipe may or may not daylight on the farm side of the access road with various arrangements. This physical configuration limits flow measurement options to one side of the buried pipe or the other, and many districts have limited (or no) jurisdiction to install devices on the farm side of the turnout.

The size and placement of a flow measurement device is also constrained by other factors. The device cannot obstruct normal canal maintenance operations, or be vulnerable to damage from access road traffic (Burt 2010). In addition to these factors, flow measurement devices are also susceptible to typical problems experienced in most open channel applications such as sedimentation, trash and biological debris, and vandalism. Despite these challenges, canal turnout flow measurement has been successful at various levels.

Most existing canal turnouts fit into one of the following categories:

- Simple canal gate that was never designed to provide a means of flow measurement or volume totalizing.
- A “rated” gate to which a prescribed formula or rating table is used in conjunction with field measurements such as the upstream and downstream water levels, and the gate opening. Examples include:
  - ARMCO metergate
  - IID jack gate
  - Constant head orifice
- A simple canal gate, combined with an auxiliary and dedicated flow measurement device including:
  - Open propeller meters
  - Portable or permanent Acoustic Doppler Velocity Meters (ADVMS) and similar electric devices
- Relatively new, complete gate and flow measurement packages (e.g., the Rubicon SlipMeter)
- Pumps, which for the purposes of this paper are considered pipeline turnouts

This paper discusses three specific efforts to work with existing structures to improve accuracy. The three examples are:

1. Verifications of ARMCO meter gate rating tables for standard and non-standard installations
2. A calibration system and procedure for IID jack gates
3. Pilot installations of an adjustable, flow measurement orifice for non-standard canal turnouts

## METERGATE CALIBRATIONS

### Overview

Metergates are the most common canal turnout structure in California irrigation districts (ITRC 2002), although many (if not most) do not have a proper downstream stilling well. Since the early 1900's metergates have been commercially available from various manufacturers as an integrated canal turnout package, functioning as both a flow control and flow measurement device. Metergates are standard round canal gates with a specific configuration, as shown in Figure 1, which serves to "standardize" the downstream hydraulic conditions for field measurements.

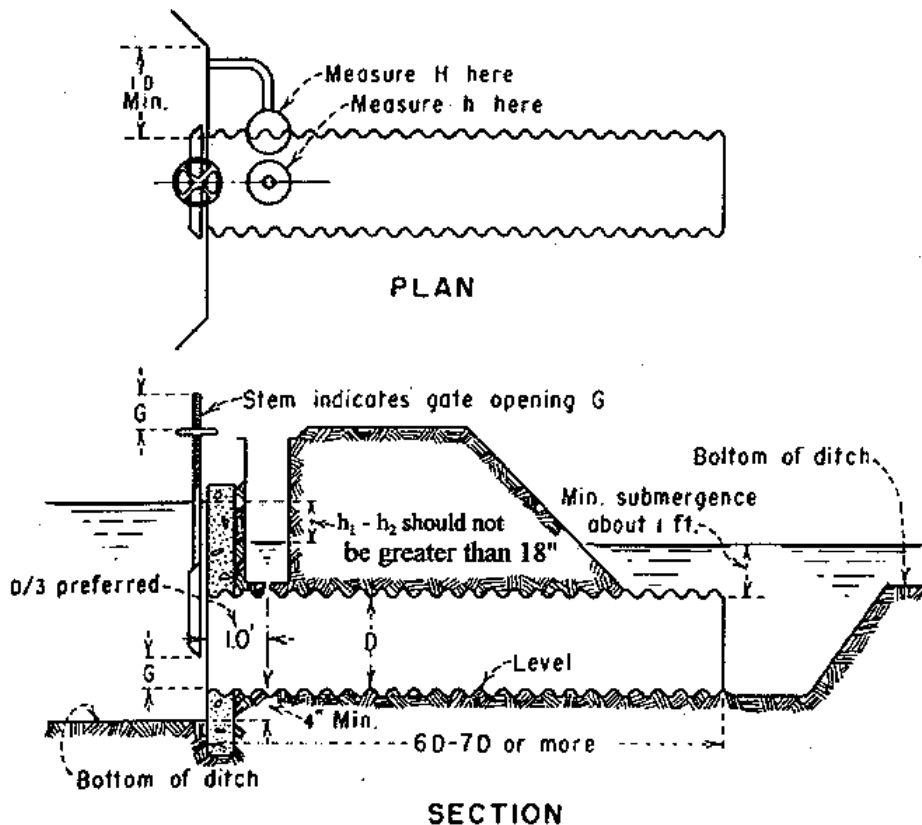


Figure 1. Metergate installation requirements (USBR 1997). Recommended modifications are noted in Burt and Howes (2014).

Flow Measurement. The difference in head pressure between the upstream and downstream sides of the gate mechanism and the gate opening are determined during an irrigation event and applied to manufacturer-provided rating tables (USBR 1997).

Volumetric Totalizing. The irrigation water volume delivered during an irrigation event can be calculated with the following equation:

$$V = \sum_{i=1}^n (Q_i \times t) \times \frac{3600}{43560} \quad \text{Equation (1)}$$

Where,  
V = volume delivered (Acre-feet)  
 $Q_i$  = instantaneous flow measurements ( $\text{ft}^3/\text{sec}$ )  
n = number of observations made  
t = times between measurements, (hours)  
3600/43560 = conversion factor

### **Calibration Evaluation**

Many existing metergate installations do not meet the prescribed installation requirements; for example, the downstream water level measurement connection is often not installed 12" downstream of the gate face. For these and other non-standard metergate installations, applying the standard rating tables provides an unknown flow measurement uncertainty.

ITRC evaluated standard metergate rating tables for both standard and non-standard installations (Howes and Fulton 2013). Round and square gates of various sizes were included in the evaluation.

Results. A summary of the results from the evaluation is provided below (Burt and Howes 2014):

1. A high level of flow measurement accuracy (+/-5%) was found if all of the following conditions are met:
  - a. The gate opening is between 20% and 75%
  - b. The top of the gate is submerged by a minimum of one-half the gate opening
  - c. The location of the downstream water level measurement is between 4" to 12" downstream of the face of the gate
2. A downstream water level measurement location between 4" and 12" downstream of the gate face does not have a significant effect on the flow rate obtained using the existing rating tables unless the gate is open more than 70-75% (percent of fully open).
3. Supply canal (tangential) water velocities did not seem to have a significant impact on the flow through the turnout gates. Supply channel velocities up to 1.9 feet per second (fps) were examined.
4. Higher flow measurement uncertainty (error) occurred at gate openings less than 20%.
5. Optimum range of operation for the highest accuracy was an opening between 20% and 75% under most conditions. Smaller gate openings seemed to be more problematic than larger gate openings.
6. Increased flow measurement uncertainty occurs if the upstream gate face is not submerged by at least one-half the gate height (or diameter). USBR recommends upstream gate submergence of at least a full gate height (or diameter).

During the evaluation, practical installation and operational recommendations were developed for metergates:

1. The buried pipe downstream of any metergate needs to remain full to enable downstream water level measurements.
2. Upstream submergence of at least one-half the gate height (or diameter) is required.
3. The true gate opening needs to be known. This is typically different than simply measuring the vertical gate movement from the seating position because of:
  - a. Tolerances between the gate stem and the gate face. There is almost always measurable “slop” (0.25” or more) in the stem-gate connection.
  - b. Overlap of the gate face to the actual opening. To fully seated (closed) position, most round and square canal gate faces must overlap the flow area opening.
4. The true gate zero should be marked by a grinder or other permanent means other than a marker or paint.
5. A stilling well should be installed on the downstream water level measurement location. The stilling well provides dampening of water level fluctuations due to turbulence. The stilling well should be:
  - a. At least 6”-8” in diameter with a small access hole to the buried pipe of approximately  $\frac{3}{4}$ ” diameter. Not only does this combination of sizes provide for adequate dampening, but also:
    - i. The larger diameter allows easier measurements. The operator can actually see the water level and use a standard tape to measure down.
    - ii. The larger diameter allows for cleaning the stilling well, such as removing sediment, trash, leaves, and other debris.
  - b. The top of the stilling well should be equal in elevation to the top of the gate frame. This ensures that a single reference plane is available to the operator to measure the upstream water level (down from the gate frame) and the downstream water level (down from the top of the stilling well).

Discussion. The results of the evaluation indicated that with the proper installation, preparation and operation techniques, metergates could achieve acceptable accuracies for both flow measurement and volumetric totalizing.

- The delivered flow rate can be measured within acceptable accuracies using rating tables as long as various key conditions are met. The ITRC rating tables also provide flow measurements with improved uncertainties for less-than-ideal gate openings (less than 25% or greater than 75%).
- Delivered volumes of water can meet required accuracy standards with sufficient periodic flow measurements. The minimum frequency of those periodic measurements must be determined by local conditions, such as the variability in the water level of the supply canal.

### **IID GATE CALIBRATION SYSTEM**

The typical canal turnout for Imperial Irrigation District (IID) is a jack gate. The name is derived from the lifting mechanism. A typical IID jack gate is shown in Figure 2.



Figure 2. A typical IID jack gate

For flow measurement, the difference in head pressure between the upstream and downstream sides of the gate mechanism and the gate opening are measured during an irrigation event and applied to gate discharge equations. It is difficult to determine the validity of the equation and its coefficients without verification. Furthermore, different equations and sets of measurements are required for submerged and free flow conditions.

Various theoretical and analytical methods have been proposed to determine the correct coefficients based on field-measured ratios such as the relative opening using momentum or energy conservation approaches (Belaud et al 2009); however, these are likely too complex for utilization in the field. Rather, it was proposed that the general submerged and free flow gate discharge equations could be used (or rating tables) to provide sufficiently accurate flow measurement if the discharge coefficient was determined empirically. The general gate discharge equation for a submerged flow condition is shown as (USBR 1997):

$$Q = CA\sqrt{2g\Delta H} \quad \text{Equation (2)}$$

Where,

C = discharge coefficient

A = open flow area (ft<sup>2</sup>)

g = acceleration of gravity, (ft/sec<sup>2</sup>)

$\Delta H$  = head differential across the gate (ft)

For gates that operate in free-flow conditions, the following general equation is used:

$$Q = CA\sqrt{2gH} \quad \text{Equation (3)}$$

Where,

C = discharge coefficient

A = open flow area (ft<sup>2</sup>)

G = acceleration of gravity, (ft/sec<sup>2</sup>)

$\Delta H$  = upstream head (ft)

Through in-situ field testing, the discharge coefficient could be determined. It was thought that such an approach would not only simplify the flow measurement process compared to other methods, but also provide verified field data as an improvement over theoretical equations.

### **Characterization Overview**

Transitioning flow conditions and the variety of (i) side contractions, (ii) bottom contractions, and (iii) hydraulic entrance conditions further complicate the use of theoretical equations and coefficients. Because it would also be economically infeasible to standardize all IID jack gates through replacement, it was determined that characterizing jack gates could be a possible solution to meet district-level flow measurement obligations.

In cooperation with Sawtelle and Rosprim, a Corcoran, CA fabrication firm, ITRC modified a “moon-buggy” pumping system that would be used to calibrate individual IID jack gates. The pumping system is shown in Figure 3.



Figure 3. Pumping system for IID jack gate characterization

Fundamentally, the pumping system can be used to characterize canal turnouts by delivering water through the gate, and pumping the water back to the supply canal while measuring the flow rate with redundant, certified flow meters.

More specifically, the characterization process was conducted as follows:

1. The supply canal would be configured to provide relatively good water level control via weir flow, and the water level was manually adjusted to be close to the high water mark. Therefore, slight fluctuations in the canal water level would be a smaller percent of the total submergence of the gate.
2. A removable dam was installed in the farm ditch approximately 60 feet downstream of the turnout gate.

3. The suction piping of the pumping system was set approximately 20-40 feet downstream of the turnout gate.
4. The discharge piping of the pumping system was set to return into the supply canal.
5. The true gate zero was determined.
6. The gate was slowly opened to deliver a historic maximum flow, and the pumping system flow rate was adjusted via hydraulic Vernier controls.
7. The pumping system flow rate was adjusted so that the farm ditch had little freeboard, but a consistent depth.
8. Once the farm ditch water level had stabilized at the maximum flow rate, multiple flow meter readings and gate water level measurements were recorded over a period of 10 minutes.
9. The gate position was adjusted to lower the flow rate, and the process was repeated.
10. The field data was recorded at a total of three flow rates: a historical maximum, a medium flow rate, and the historical minimum flow rate.

The field measurements were entered into a spreadsheet that was set up to automatically calculate a discharge coefficient at the particular flow rate and gate opening. Equation (4) is rearranged from Equation (2) for a submerged flow condition:

$$C_d = \frac{Q}{A\sqrt{2g\Delta H}} \quad \text{Equation (4)}$$

## Results

To train IID staff on the characterization operations, a full gate characterization was completed. A jack gate was characterized at three different flow rates. Using Equation (4) the results from the completed characterization are shown in Table 1.

Table 1. Results of completed jack gate characterization

	High Flow	Medium Flow	Low Flow
Submerged (Y/N)	Y	Y	Y
Measured Flow Rate (CFS)	11.09	7.25	3.96
$\Delta H$ (ft)	0.26	0.34	0.48
Flow Area (sq. ft)	3.81	1.90	0.78
Discharge Coefficient, Cd	<b>0.715</b>	<b>0.815</b>	<b>0.912</b>

The three discharge coefficients can then be plotted to develop an equation to solve for interpolated discharge coefficients for any expected flow rate. The plot is shown in Figure 4. A linear trendline was developed so that discharge coefficients can be interpolated with a reasonable level of accuracy ( $R^2 = 0.9988$ ), for flow rates typical of the specific canal turnout.

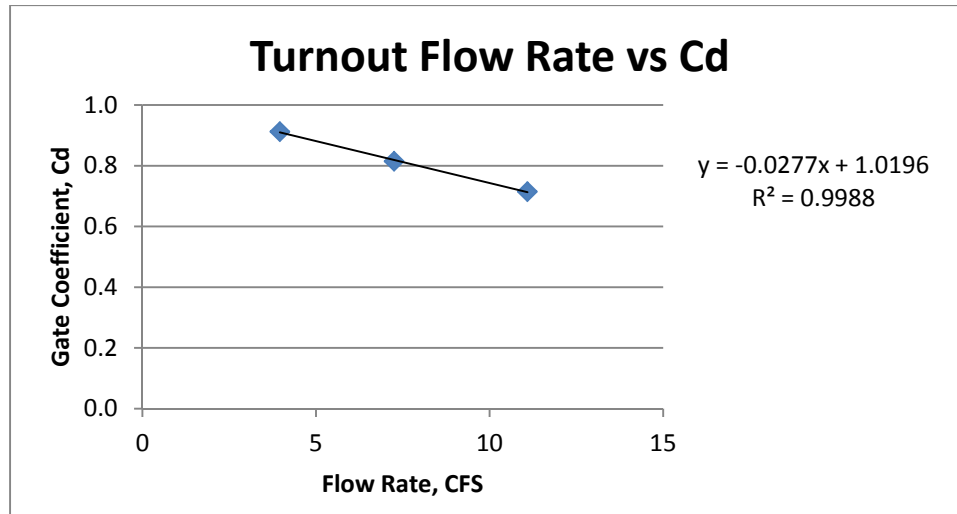


Figure 4. IID jack gate – flow rate versus discharge coefficient

### **Discussion**

Some jack gates transition between free flow and submerged flow conditions. The transition between flow conditions can occur between low and high flow rates, or be caused by fluctuating downstream conditions throughout irrigation events.

For these transitional flow condition turnouts, it can also be difficult to properly identify the flow condition, and can be confusing to operators. For these sites, it would be recommended that a hydraulic “bump” be installed downstream of the jack gate to raise the water level downstream of the gate for a short distance. This would ensure the gate operates under submerged flow conditions for typical delivered flow rates.

Flow Measurement. The pumping system was successful in developing individual discharge coefficients, which could be used in conjunction with the appropriate gate discharge equation and field measurements. It is expected this method would provide flow measurement within the stipulated accuracies for existing gates.

However, many of the same practical and operational recommendations developed by ITRC from the metergate evaluation also apply to the use of gate discharge equations for jack gate flow measurement, including:

1. Determining a true gate zero opening position
2. Permanently marking that position
3. Providing a single reference plane for water level measurements for submerged flow gates

In addition, ITRC recommended that jack gate turnouts could be categorized by similar hydraulic conditions such as:

- Submerged, free-flow, or transitioning conditions
- Suppression or contraction on the gate sides
- Suppression or contraction on the gate bottom



By categorizing gates, the total number of characterizations could be significantly decreased. A second gate characterization was started as part of the training, but was not completed with ITRC support.

Volumetric Totalizing. Similar to the metergate, operators must take one or more instantaneous flow measurements and apply those to Equation (1) to determine the delivered irrigation water volume per irrigation event.

Challenges. A complete turnout characterization took approximately 6 hours; however, much of that was focused on IID staff training on the transportation, operation and data analysis. It is likely that after a few iterations, two complete characterizations could be completed in less than 8 hours with a team of 2-3 operators/engineers, if the two turnouts were somewhat close together along the same channel.

Safe transportation along a canal access road was possible with a standard 1-ton truck; however, over-the-road transport required a semi-truck and trailer with “oversize” flags.

Cost. The complete pumping system, parts and accessories cost approximately \$110,000. Although the initial capital investment is relatively large, the cost per turnout is much lower in such a large district. Furthermore, the pumping system can be, and probably will be, used for other district operations such as dewatering canals.

### **ADJUSTABLE ORIFICE PLATE**

There are many existing California canal turnouts that were never designed to provide flow measurement, or never installed properly to meet certain conditions. For these installations, districts will need to either replace the structure or install an auxiliary device to provide accurate flow measurement and volumetric totalizing.

For these structures, ITRC examined the applicability of an adjustable orifice plate with a key feature – a datalogger with single pressure transducer that measured the differential head across the orifice. There is nothing new about using orifice plates upstream of a flow control gate – this application was designed for the case of frequently varying flow rates into a turnout that would not be properly measured by the district operators. The plate can be installed without replacing the existing structure, keeping everything on the irrigation district side of the access road. The orifice plates can be installed vertically or parallel with a canal’s side slope, upstream of an existing canal turnout, as shown in Figure 5.

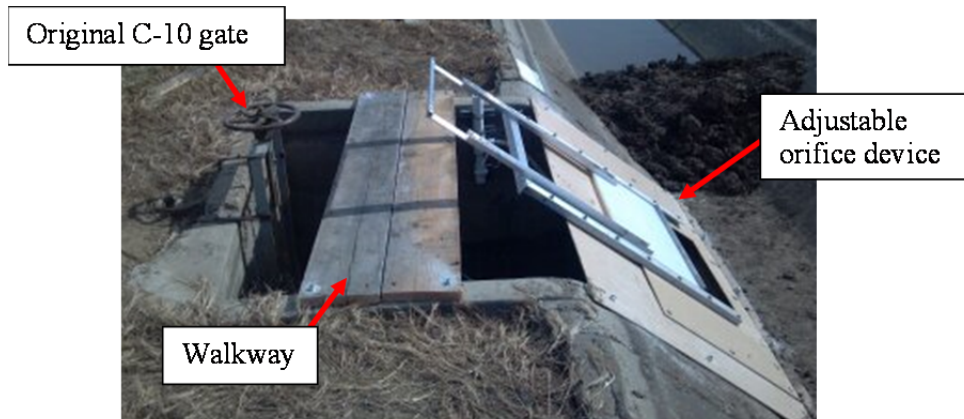


Figure 5. Orifice plate configuration with an existing non-standard metergate

### Orifice Plate Overview

The orifice plate approach combines a standard USBR submerged orifice discharge equation with the physical configuration of a constant head orifice (CHO). The discharge equation is the same as Equation (4), for a submerged flow gate, with the exception of the discharge coefficient. Provided the following conditions are met, a  $C_d$  of 0.61 can be used (USBR 1997):

- The upstream edges of the orifice should be straight, sharp, and smooth.
- The upstream face and the sides of the orifice opening need to be vertical.
- The top and bottom edges of the orifice opening need to be level.
- Any fasteners present on the upstream side of the orifice plate and the bulkhead must be countersunk.
- The face of the orifice plate must be clean of grease and oil.
- The thickness of the orifice plate perimeter should be between 0.03 and 0.08 inches. Thicker plates would need to have the downstream side edge chamfered at an angle of at least 45 degrees.
- Flow edges of the plate require machining or filing perpendicular to the upstream face to remove burrs or scratches and should not be smoothed off with abrasives.
- For submerged flow, the differential in head should be at least 0.2 feet.
- Using the dimensions depicted in Figure 6,  $P > 2Y$ ,  $Z > 2Y$ , and  $M > 2Y$ .

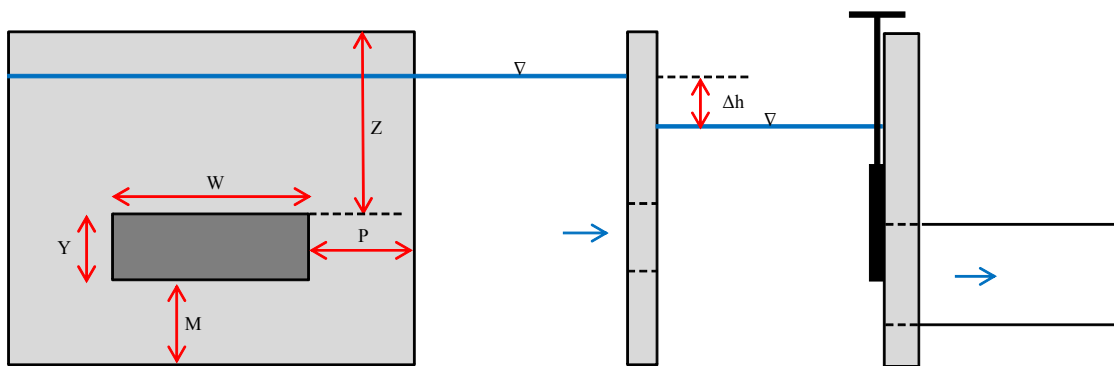


Figure 6. Submerged orifice dimensional requirements

It was proposed that the orifice area be made adjustable so that a range of flows could be delivered, while maintaining a measurable head differential across the orifice (0.2' minimum).

Operators could then use a rating table to choose an appropriate orifice opening to meet the irrigation demand, such as the one as shown in Table 2.

Table 2. Orifice plate rating table

Flow Rate, CFS	Width of Orifice Opening, ft											
	2.5											
	Height of Orifice Opening, ft											
	0.5	0.6	0.8	1.0	1.2	1.4	1.6	1.8	2.0	2.2	2.4	2.5
Change in Head, ft												
30.0											1.04	0.96
25.0									1.04	0.86	0.72	0.67
20.0							1.04	0.82	0.67	0.55	0.46	0.43
15.0					1.04	0.77	0.59	0.46	0.38	0.31	0.26	0.24
10.0			1.04	0.67	0.46	0.34	0.26	0.21	0.17	0.14	0.12	0.11
9.0			0.85	0.54	0.38	0.28	0.21	0.17	0.14	0.11		
8.0		1.19	0.67	0.43	0.30	0.22	0.17	0.13	0.11			
7.0		0.91	0.51	0.33	0.23	0.17	0.13	0.10				
6.0	0.96	0.67	0.38	0.24	0.17	0.12						
5.0	0.67	0.46	0.26	0.17	0.12							
4.5	0.54	0.38	0.21	0.14								
4.0	0.43	0.30	0.17	0.11								
3.5	0.33	0.23	0.13									
3.0	0.24	0.17										
2.5	0.17	0.12										
2.0	0.11											
1.5												
1.0												

The orifice can be adjusted and locked in place with pins at discrete orifice opening intervals (0.1'), as shown in Figure 7.

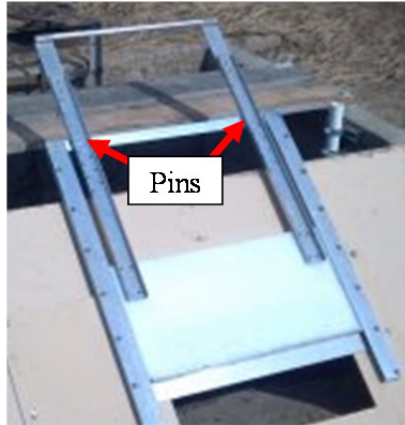


Figure 7. Adjustable orifice

Flow Measurement. The existing canal gate would then be used to start and adjust the delivered flow. The flow rate can be manually measured by using an incorporated stilling well, as shown in Figure 8.

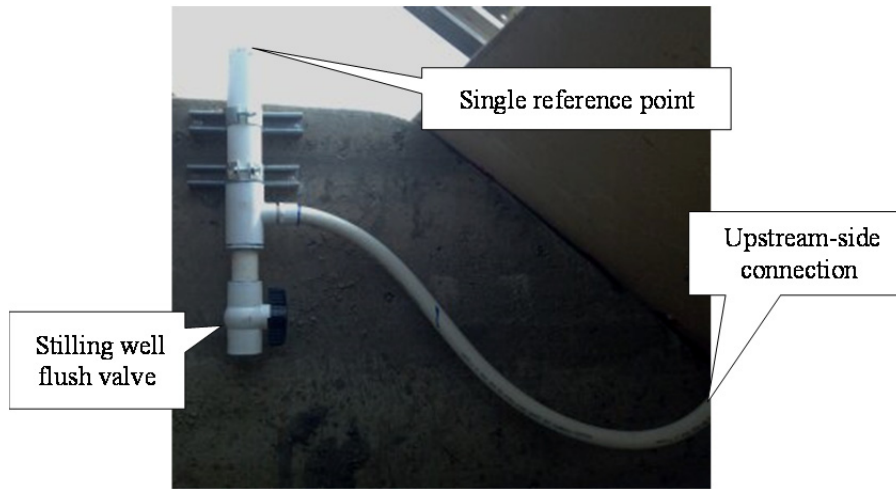


Figure 8. Stilling well configuration, installed downstream of the orifice plate

With the orifice width fixed and the orifice height known, the head differential is measured by two methods. Manual head differential measurements are taken at the top of the stilling well. The upstream water level is measured from the top of the stilling well to the water level inside. The downstream water level is also measured from the top of the stilling well to the surrounding water level. In addition, a differential pressure transducer and data logger is installed to record the head differential measurement over time.

Volume Totalizing. Manual flow measurements could be averaged and the volume totalized using Equation (1). The data logger provides a redundant record of instantaneous flow measurements at 2.5 minute intervals. The spreadsheet data can then be manipulated using a computer program such as Microsoft Excel® to calculate delivered volumes.

**Results**

Flow Measurement. ITRC installed two orifice plates with single pressure transducers: one at Patterson Irrigation District (PID) and a second in Merced Irrigation District (MID). During the first season, problems with the differential pressure transducer were found. However, the PID installation has continued to operate over two complete irrigation seasons. The PID data was retrieved and plotted. The PID flow measurement results using Equation (2) and a discharge coefficient of 0.61 are shown in Figure 9.

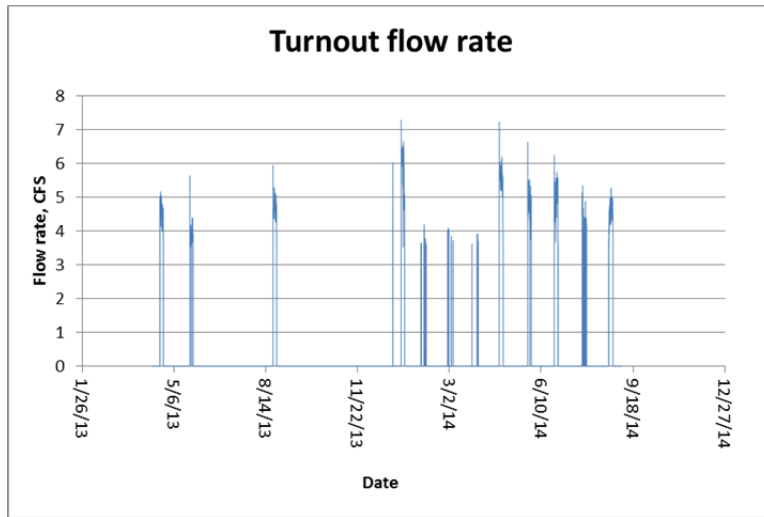


Figure 9. PID orifice plate flow measurement data

Volume totalizing. Using the same spreadsheet, the delivered volumes were calculated and accumulated over two irrigation seasons. The volumetric results are shown in Figure 10.

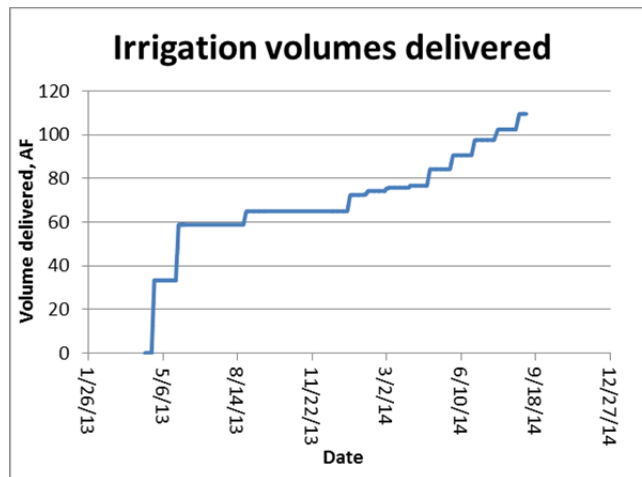


Figure 10. PID orifice plate volumetric data

The turnout delivered roughly 65 acre-feet during the 2013 irrigation season and 55 acre-feet during the 2014 irrigation season.

### **Discussion**

Although the orifice plates were not calibrated at a flow measurement facility, their configuration provided a method of applying standard discharge equations to non-standard canal turnouts. Further evaluation may be conducted in the future regarding the discharge coefficient in both the vertical orifice and slanted orifice orientations.

Challenges. The Telog data logger utilized for these and other trials has proven to be a rugged and dependable tool for research. However, data retrieval requires a field visit, as well as a proprietary cable and program installed on a laptop. Recent technological advances have become readily available for these applications such as wireless communication, cloud-based databases, and automated reporting. However, that advanced technology would do little to resolve most of the challenges experienced during this experiment.

The most challenging aspect to the expanded implementation of the orifice plate trials was finding adequate sensing products. There are very few manufacturers of submersible, differential pressure transducers of the type used in this experiment. Even fewer of these available products are sufficiently rugged for the application. One of the two GE Druck pressure transducers experienced significant drift over the first season. It has since been removed until another solution can be found. Future testing of orifice plates for flow measurement will likely include various other sensing technologies.

Cost. Each orifice plate cost roughly \$6,000 to fabricate and install in the field. The cost of construction could likely be decreased with less expensive materials and local fabrication shops.

## **CONCLUSION**

Various methods are available to irrigation districts that can provide canal turnout flow measurement and volumetric totalizing that conform to regulatory standards. However, the variety of existing canal turnout structures, their hydraulic conditions, and specific local considerations will likely result in an equally varied implementation of flow measurement and volumetric totalizing across California.

Regardless of the method used for flow measurement and volumetric totalizing, there will likely be further challenges in the future for irrigation districts to aggregate and organize the large amounts of volumetric data.

### ACKNOWLEDGEMENTS

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Rubicon Water  
San Luis Canal Company  
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# SBx7 Flow Rate Measurement Compliance for Agricultural Irrigation Districts



**SBx7 Compliance**

**Aug 26, 2012**



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Irrigation Training & Research Center

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## TABLE OF CONTENTS

<b>Grouped Deliveries</b> .....	<b>1</b>
Conclusions.....	2
<b>Flow Rate vs. Volumetric Accuracy</b> .....	<b>3</b>
<b>Impact of Canal Water Level Changes on Annual Volumetric Accuracy</b> .....	<b>4</b>
Background .....	4
Error Analysis .....	4
<i>Water Level Error Model</i> .....	4
<i>Sample Set</i> .....	4
<i>Results</i> .....	5
Conclusion.....	6
<b>Selection of a Representative Sample for Verification of Accuracy</b> .....	<b>7</b>
Background .....	7
<i>Representative Sample</i> .....	7
<i>Considerations for Availability</i> .....	7
Scenario 1: Acreage-Based Sampling Using Probability-Proportional-to-Size (PPS).....	8
<i>Background</i> .....	8
<i>Step 1: Assign Sequence Range Numbers to Each Turnout</i> .....	9
<i>Step 2: Use a Random Number Generator to Select Turnouts</i> .....	9
<i>Step 3: Evaluate Selected Turnouts and Record Data</i> .....	10
<i>Step 4: Determination of Compliance</i> .....	11
Scenario 2: Limited Availability of Turnouts and Opportunity Sampling.....	12
<i>Background</i> .....	12
<i>Step 1: Choose a Currently Available Turnout</i> .....	12
<i>Steps 2-4 : Follow the Previous Scenario Instructions</i> .....	12
<b>Flow Measurement Devices</b> .....	<b>13</b>
Background .....	13
Meter Gates .....	13
Orifice Plates.....	15
Trash Shedding Propeller Meters.....	19
Rubicon Transit Time Flow Meter.....	21

## LIST OF FIGURES

Figure 1. Sample distribution for hourly % error in water level vs. frequency .....	5
Figure 2. Means and standard deviations for each block .....	6
Figure 3. Side contractions rather than a traditional "Replogle Flume". Designed by USBR, Yuma. The rocks are not part of the design. ....	14
Figure 4. Flow through a submerged orifice plate.....	15
Figure 5. Installation of orifice.....	16
Figure 6. Rubicon Sonaray flow meter.....	21

## LIST OF TABLES

Table 1. Example of assigning sequence range numbers .....	9
Table 2. Example of randomly selected sample set.....	10
Table 3. Sample data collection for selected turnouts .....	10
Table 4. Device selection on two separate days .....	12
Table 5. Orifice size values .....	17

## GROUPED DELIVERIES

Senate Bill x7-7 (SBx7-7) requires documented volumetric accounting to individual turnouts for water deliveries. Section 597.3 of the bill lists two very different requirements for devices (**bold, underlined, italics** have been added for emphasis):

- Section 597.3(a) discusses measurement devices that must be used at points where there is a reasonable degree of flow rate control.
- Section 597.3(b) states that "An agricultural water supplier may measure water delivered at a location upstream of the delivery points or farm-gates of multiple customers using one of the measurement options described in §597.3(a) if the downstream individual customer's delivery points meet **either** of the following conditions:

A. The agricultural water supplier does not have legal access to the delivery points of individual customers or group of customers to install, measure, maintain, operate, and monitor a measurement device.

**Or,**

B. An engineer determines that due to small differentials in water level **or** large fluctuations in flow rate or velocity that occur during the delivery **season** at a single farm-gate, accuracy standards of the measurement options in §597.3(a) cannot be met by installing a measurement device or devices (manufactured or on site built or in-house built devices) with or without additional components (such as gauging rod, water level control structure at the farm-gate, etc.).

This last section (B) in essence defines the most downstream point of measurement to be located at the "hand-off point".

**The "hand-off point" can be defined as the location, moving downstream in the branching hydraulic network, below which the irrigation district no longer has good control over the flow rates that go to individual farm-gates.**

For example, one might consider using a ditch or pipeline with a rotation delivery schedule, with one "head" or delivery at a time. That single "head" or flow rate is rotated among users, one at a time. There is no control over flow rates at individual turnouts (along that ditch or pipeline); the flow rate is controlled at the head of the ditch or pipeline.

This is also true of ditches or pipelines with a rotation delivery schedule, with two or three "heads" or deliveries. These systems typically have little or no precise flow control downstream of the heading. In some districts, the delivery points are not even to a field; the distribution pipelines have alfalfa valves for each border strip that is irrigated. When there is an internal splitting of two "heads", it is done without the benefit of the structures that provide good water level or pressure control.

While it may be possible in many cases to install flow measurement devices within these pipelines or canals, the measurement would be of uncontrolled flows unless the pipelines or canals were substantially modified. In other words, "additional components" besides the flow measurement devices would be required.

Rice systems are a special category, as good water management of rice irrigation is premised on maintaining a target water level in the fields, rather than on delivering a specific volume to a specific field.

That said, with traditional rice laterals, or with traditional rotation laterals, it is entirely reasonable to require farmers with new pressurized systems on such ditches/pipelines to install magnetic meters or propeller meters on their systems. Such flow measurement installations are rather typical and do not represent technical or fiscal challenges for implementation.

## ***Conclusions***

1. The wording of SBx7 appears to clearly indicate that the proper, most downstream flow measurement location would be at the head of any "community ditches". "Community ditches" (sometimes called "improvement districts") are defined as privately owned distribution systems that receive water from the irrigation district. The distribution, partitioning, and scheduling of water deliveries within the "community ditch" is not done by irrigation district personnel.
2. Irrigation district ditches and pipelines that are operated on a rotation schedule need an accurate flow measurement device at the head of the ditch or pipeline, but not at individual delivery points within/along the ditch or pipeline that receives water on a rotation schedule. This pertains to ditches and pipelines that are owned either by improvement districts or by irrigation districts.
3. Individual delivery points with pressurized irrigation systems that receive water from an irrigation district ditch or pipeline that is primarily a "rotation" system must be individually metered.

*Note: The phrase "irrigation district" encompasses a wide range of district types including reclamation districts (e.g., RD108), water districts (e.g., Coachella WD), irrigation districts (e.g., Modesto ID), and Water Storage Districts (e.g., Buena Vista WSD).*

## FLOW RATE VS. VOLUMETRIC ACCURACY

SBx7 requires the verification of the accuracy of annual volumes provided at delivery points.

- For devices **with** totalizers, it can be assumed that:

$$\text{Flow rate accuracy} = \text{Volumetric accuracy}$$

- For devices such as meter gates and orifice plates that do **not** have totalizers, the flow rate accuracy may only be part of the total desired 12% volumetric accuracy. The annual volumetric accuracy of any such single turnout depends upon errors due to:
  - IFR – Instantaneous flow rate error
  - CWLF – Canal water level fluctuations, or pipeline pressure fluctuations over time. The impact of these fluctuations are mostly self-canceling over the course of an irrigation season. This is discussed later in this report.
  - CBP – Changes in "backpressure". Backpressure is the pressure on the downstream side of the flow measurement device.
  - ARD – Accuracy of the recording of durations. For example, if an actual delivery lasts for a total of 25 hours but it is recorded and billed as a 24-hour delivery, this would be an error of one hour, or 4.2%

These inaccuracies must be mathematically combined to determine the total volumetric accuracy.

$$\text{Volumetric accuracy} = 100 \times \left[ 1 - \sqrt{(\text{IFR})^2 + (\text{CWLF})^2 + (\text{CBP})^2 + (\text{ARD})^2} \right]$$

For example, assume the following errors expressed as decimals rather than as percentages. These are plus/minus errors ("within 5%" means "within +/- 5%"):

$$\text{IFR is within 5\% (IFR} = .05)$$

$$\text{CBP} = .03$$

$$\text{CWLF} = .02$$

$$\text{ARD} = .04$$

Then,

$$\begin{aligned} \text{Volumetric accuracy (VA)} &= 100 \times \left[ 1 - \sqrt{(.05)^2 + (.02)^2 + (.03)^2 + (.04)^2} \right] \\ \text{VA} &= 92.7 = 93\% \end{aligned}$$

The errors are independent of each other. Therefore, the total error does **not** equal the sum of the errors (14%), which would incorrectly indicate an 86% accuracy.

The maximum acceptable flow rate measurement error (expressed as a decimal) equals:

$$\text{Max. acceptable device flow rate error} = \sqrt{\left(1 - \frac{\text{VA}}{100}\right)^2 - \text{ARD}^2 - \text{CBP}^2 - \text{CWLF}^2}$$

For example, if the required volumetric accuracy (VA) = 88% (88) (i.e., within 12%) and:

$$\text{ARD} = .04 \quad \text{CBP} = .03 \quad \text{CWLF} = .02$$

Then, the maximum acceptable device flow rate accuracy error = 0.107 = 10.7%

That is, this specific device, when tested at a specific representative flow rate, must be within 89.3% accuracy.

# IMPACT OF CANAL WATER LEVEL CHANGES ON ANNUAL VOLUMETRIC ACCURACY

## ***Background***

The volume delivered through flow measurement devices without totalizers is computed as:

$$\text{Volume} = (\text{Flow Rate}) \times \text{Time}$$

The flow rate is typically checked once per day, and a new flow rate is either noted on the records, or the flow rate control device is re-adjusted to provide the target flow rate.

During any 24-hour period, the canal water levels will fluctuate, resulting in a delivery of more or less flow rate than was originally set.

The question addressed in this section is: Over the course of an irrigation season with ten, twenty, or thirty 24-hour irrigation events, do these minute-to-minute fluctuations cancel out? If they do, this will remove the "CWLF" (discussed in the previous section) from consideration.

To examine this, ITRC obtained water level data from multiple locations throughout San Luis Canal Company, over a time period from June 8 to July 11, 2012. Canal levels were recorded automatically on an hourly basis. The total change in water level across the turnout [(water surface in the canal) - (water surface in the downstream ditch)] was also recorded at the start of each datalogging session. The irrigation district has typical flashboard check structures to maintain water levels in the majority of its locations.

A series of 22 sites were analyzed for 48-72 hours. It is believed that these sites are representative of the range of conditions throughout the district. No special management of the check structures was involved; the canal operators were unaware that the levels were being recorded.

## ***Error Analysis***

### **Water Level Error Model**

In order to assess the error of volumetric flow rate measurement in the canal system, first the fluctuations in water level must be computed. A model was constructed to measure the percent error of the water level over a 24-hour period from a given starting point in the sample set.

The raw data was normalized so that canal fluctuations would be represented as a percentage of the head difference. In this way, all the data points could be accumulated to create a contiguous set of hourly fluctuations for the model data set. The resulting model contains a total of 5500 hourly data points.

### **Sample Set**

A sample set was generated from the model. The sample set contained three different blocks. Each block had 30 different seasons with varying numbers of irrigations events per season. Block 1 had 30 seasons of ten 24-hour irrigations, Block 2 had 30 seasons of twenty 24-hour irrigations, and block 3 had 30 seasons of thirty 24-hour irrigations.

The starting points for the irrigation events in each season were selected by a random number generator. The error was recorded for each hour from the starting point for a total 24 hours. Thus, each irrigation event consisted of 24 data points, resulting in a total of 21,600 data points sampled for all of the seasons in all 3 blocks.

## Results

If the present water level for a moment during an irrigation event in the model is equal to the starting water level for that event, then the percent error at that moment is zero. The percent error at each recorded time during an irrigation is calculated by the following equation:

$$\% \text{ Error at a moment} = \frac{\text{Present Water Level} - \text{Initial Water Level}}{\text{Initial Change in Head}} \times 100$$

Where "Initial Water Level" is the water level when the 24-hour irrigation began.

The characteristics of the population of "errors" in water level are shown in the figure below.

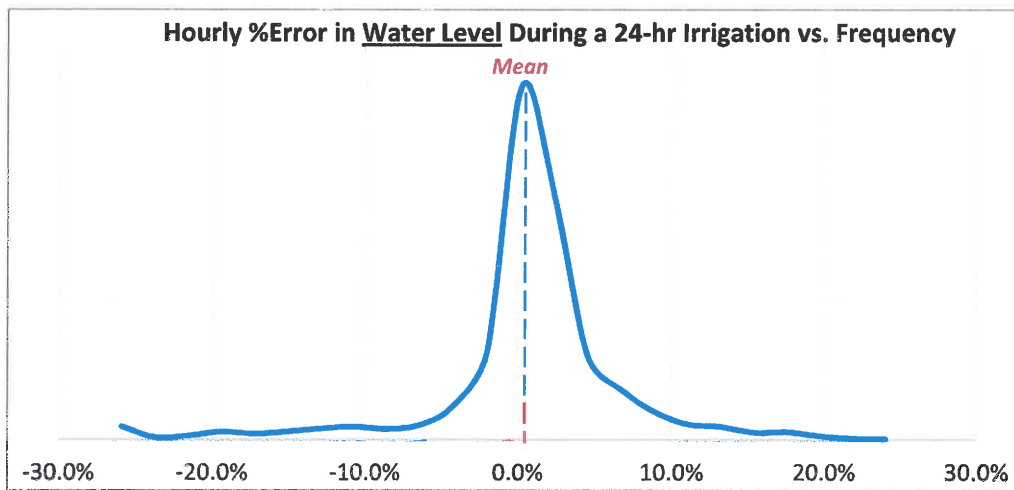


Figure 1. Sample distribution for hourly % error in water level vs. frequency

The variation in relative water levels over time is interesting, but of more interest is the impact on turnout flow rates. There are two possible situations, described below:

1. The flow measurement device is operated under "free flow". That is, the water jets out from it, and the flow rate through the orifice device is not affected by changing downstream water levels. The variation in flow rate over time can be computed, based solely on the upstream water level change. In this case, the sensitivity of the turnout flows to canal water levels is computed as:

$$\text{Free Flow Error} = (1 + \text{Level Error})^{0.5} - 1$$

2. The flow measurement device operates under a "submerged" condition. In this case, what happens is that if the canal water level changes, the flow through the measurement device increases. But that also results in a rise in the downstream water level. This provides a "pressure compensating" effect. The total head change is less than the change in the canal water level. ITRC has examined a number of possible downstream channel conditions, and uses the following equation to estimate the effect of a change in canal water level:

$$\text{Submerged Flow Error} = (1 + \text{Level Error})^{0.38} - 1$$



For each block (group of 30 randomly selected seasonal irrigation cycles), the mean and standard deviation of the error were computed. **Figure 2** shows the results of the analysis. The mean error is plotted for each block along with the standard deviations. The red bars are 1 standard deviation above the mean, and the green bars are 1 standard deviation below the mean.

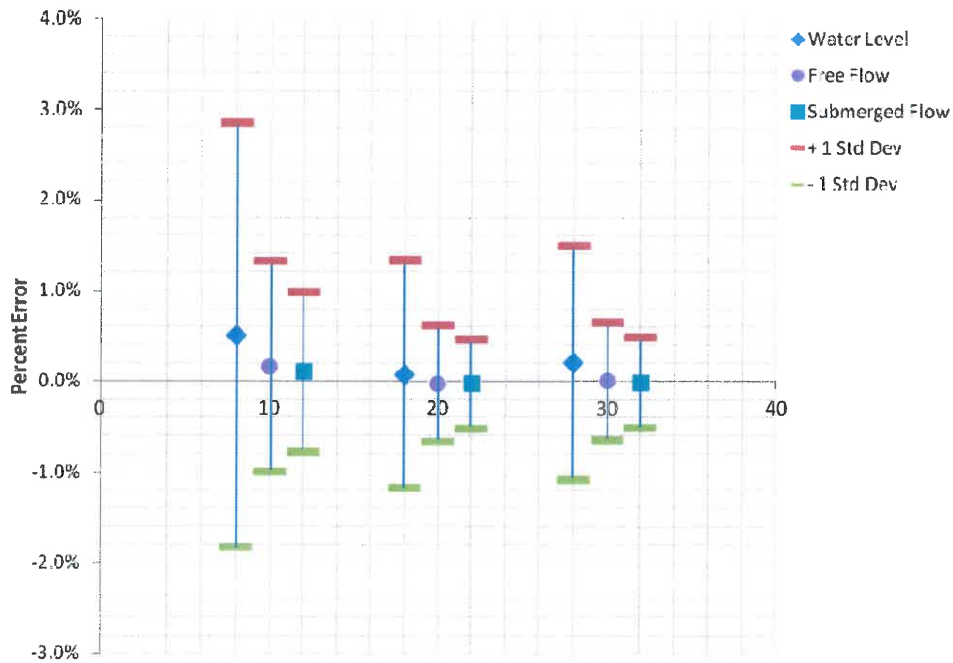


Figure 2. Means and standard deviations for each block

### Conclusion

For the condition of 10 irrigations per season, the seasonal flow rate error due to fluctuating canal water levels averages less than 0.2%, regardless of whether the turnout is free flow or submerged flow. The average seasonal error for 20-30 irrigations per season is almost 0.0%.

Because most irrigation districts deliver more than 10 irrigations per season, it appears that a reasonable estimate of the annual volumetric error due to a fluctuating canal water level is about +/- 0.5%, when one considers one standard deviation from the mean.

While this data originated in a single district, ITRC believes that the conditions are representative of "typical" canal districts, based on experiences in about 150 irrigation districts in the western U.S. The exception would be the few irrigation districts that have a very extensive distribution of long-crested weirs or ITRC flap gates throughout the canals. An extreme example would be Modesto ID, in which case almost every check structure is a long-crested weir. In that case, the seasonal impact of fluctuating canal water levels is likely 0.0%, for all practical purposes.

## SELECTION OF A REPRESENTATIVE SAMPLE FOR VERIFICATION OF ACCURACY

California Legislature SBx7 requires flow measurement devices to be within a required level of accuracy. For existing flow measurement devices, the acceptable error for volumetric flow measurement is  $\pm 12\%$  as stated in §597.3(a)(1). Initial certification of existing devices requires a random and statistically representative sample set or an accepted statistical methodology as described in §597.4(a)(1) and §597.4(b)(1). This document defines a statistical methodology that can be used to provide good information that meets both the intent of SBx7 and the needs of the irrigation districts.

### *Background*

#### Representative Sample

Irrigation districts have turnouts with flow measurement devices that supply water to areas with correspondingly varying annual delivered volumes. The selection process defined below is intended to define how to select a representative sample set of flow measurement devices for verification of volumetric measurement quality in the district as whole.

In an irrigation district with a wide range of acreages downstream of flow measurement devices, a simple random selection of measurement devices would statistically over-emphasize the importance of small delivery points. The sampling may only represent a very small percentage of all the water delivered in the district. The volume delivered through a turnout is related to the size of the area irrigated. Therefore, it is better to weigh the importance of each measurement device according to the area it services, rather than weighing all turnouts equally. Thus, the sample of flow measurement devices to be tested will be constructed using a *probability-proportional-to-size (PPS)* sampling method so that the likelihood of inspection for a given flow measurement device will be proportional to the acreage served by that device.

#### Considerations for Availability

Ideally, all the devices would be randomly selected by the PPS sampling process mentioned above, and then the selected devices would be evaluated for accuracy. However, only some percentage of the turnouts will be operating at a given time. Therefore, if a turnout is selected in a purely random manner, the customer served by that turnout may not be ready to irrigate, prohibiting evaluation of the flow measurement device at that turnout. It is also clear that even if farmers are scheduled to receive water from a turnout on a specific date/time, they do not always irrigate on that schedule; this makes advance and careful scheduling of field evaluations problematic.

A solution to this is to use *opportunity sampling* in combination with *sampling quotas*. An opportunity sample is composed of samples taken as they are available or convenient. Since device availability will be an issue, devices should be inspected when they are available.

- Point #1:** To ensure that the data set is representative of the district's overall volumetric flow measurement, a minimum of 10% of the district's service area (or volume) should be represented by the combined service acreage for the turnouts in the sample set.
- Point #2:** To meet the SBx7 requirements, the minimum sample size of 5 and maximum of 100 for a particular device type should be evaluated.
- Point #3:** Two scenarios for sampling are described in this document:
- Advance Probability-Proportional-To-Size (PPS) Sampling
  - Opportunity Sampling with a consideration of PPS

## ***Scenario 1: Acreage-Based Sampling Using Probability-Proportional-to-Size (PPS)***

Scenario 1 is the ideal situation, where at any given time all turnouts will be available for inspection.

### **Background**

#### ***Representative Sample Selection***

Flow measurement devices in a district will be assigned a number *range* based on the acreage (or known annual volume) that the devices serve (e.g., a turnout servicing 10 acres may be assigned 10 numbers such as 61-70). This numbering will have a logical sequencing that is appropriate for the given district. A random number generator will then be used to select a device from the developed sequence. In this way each device will be weighted in selection by the acreage it serves. Specifically, the sample will be skewed favoring devices that measure greater volumes of water. This will ensure that the random sample will be statistically representative of the overall accuracy of flow measurement within the district.

#### ***Random Selection Process***

A random number generator will be used to select a device to be tested. If the number produced by the random number generator is within the range assigned to a device, then that device will be tested. Once a device has been tested, its range will no longer be considered in the selection process, and numbers randomly generated in its range will be ignored. This procedure will be improved from the example given in §597.4(b)(1), in that devices providing at least 10% of the district volume or acreage (rather 10% of the devices) will be tested, with a minimum of 5 devices, and not to exceed 100 individual devices of a certain type.

#### ***Device Types***

It is important to take note of device types for this legislation. If 25% of existing devices (as estimated from the properly selected sample) of a particular type are not in compliance with  $\pm 12\%$  accuracy requirements, the district must develop a plan to test another sample of measurement devices of this type as stated in §597.4(b)(2). This document interprets the intent of the legislation as applying to 25% of water delivered, rather than 25% of existing devices. For illustration, in the extreme case of a district with the following:

- 100 garden plots of 0.25 acres each, each with a measurement device (25 acres total)
- 50 larger fields of 80 acres each, each with a measurement device (4000 acres total)

Certainly, careful irrigation water management would not focus on the large number of very small plots that represent less than 1% of the total acreage. This document therefore assumes that the proper interpretation is to focus on reasonable measurement of at least 25% of sample water volume, rather than 25% of the sample devices.

**Step 1: Assign Sequence Range Numbers to Each Turnout**

Table 1 describes a sample scenario and shows a sequence range of number assignments for each turnout. The district in the sample scenario has one lateral with 10 turnouts serving a varying array of acreage.

**Table 1. Example of assigning sequence range numbers**

Turnout #	Acreage Served	Sequence Range	
		From	To
1	10	1	10
2	10	11	20
3	15	21	35
4	15	36	50
5	2	51	52
6	2	53	54
7	5	55	59
8	5	60	64
9	50	65	114
10	50	115	164
<b>Total</b>	<b>164</b>		

*Note that the final sequence number should be equal to the total acreage*

Each turnout is assigned sequence range numbers based on their acreage. Turnout 1 is assigned the sequence range from 1 to 10 because it has 10 acres, and Turnout 2 is similarly assigned 11 to 20. Turnout 3 is assigned a longer sequence range, from 21 to 35, because it has 15 acres. Turnouts are continued to be assigned sequence range numbers in this fashion. As a result of this sequence range numbering, each turnout will represent a portion of the total 164 acres.

**Step 2: Use a Random Number Generator to Select Turnouts**

Use a random number generator to choose a number between 1 and the total acreage of the district. A random number generator can be a software program or simply pulling numbers out of a hat. In the example above the random number generator would pick a number between 1 and 164. If the number produced by the random number generator is between the sequence range numbers assigned to a device, then that device will be tested.

Repeat this process until devices representing 10% of the acreage served (or volume delivered) have been selected with a minimum of 5 and a maximum of 100 per device type.

Continuing with the example data set above, assume that the first numbers selected by the random number generator were: 17, 24, 157, 156, 53, 42, 41, 36, 2, 12, and 52.

Eliminate duplicate turnouts, starting from the first random number.

With this random selection of numbers, the following turnouts are selected:

- 2 (selected by number 17; 12 is a duplicate)
- 3 (selected by number 24)
- 10 (selected by number 157; 156 is a duplicate)
- 6 (selected by number 53)
- 4 (selected by number 41; 41 and 36 are duplicates)

This provides the minimum number of 5 turnouts. Now, the acreage must be checked to verify that the selection represents more than 10% of the acreage (or volume).

**Table 2. Example of randomly selected sample set**

*Green rows indicate the selected devices for the sample set*

Turnout #	Acreage Served		Sequence Range	
	Acres	% of Total	From	To
1	10	6%	1	10
2	10	6%	11	20
3	15	9%	21	35
4	15	9%	36	50
5	2	1%	51	52
6	2	1%	53	54
7	5	3%	55	59
8	5	3%	60	64
9	50	30%	65	114
10	50	30%	115	164
<b>Total</b>	<b>164</b>	<b>100%</b>		

The five turnout samples represent 55% of the total acreage.

Therefore, this sample set meets the criteria of:

- greater than or equal to 10% of the acreage, and
- a minimum of 5 turnouts of a particular type - assuming all are the same device.

Note: If there is more than one device, this process would be repeated *by device*. The final criteria to be met are:

- Including all device sample sets, at least 10% of the district acreage (or volume) must be accounted for.
- A minimum of 5 turnouts of a particular device, for each device.
- No more than 100 of any particular device.

**Step 3: Evaluate Selected Turnouts and Record Data**

Once the turnouts have been selected, evaluate each flow measurement device for accuracy. Record gate type, total acreage serviced by the device, and measured accuracy. This data will need to be retained for ten years or two Agricultural Water Management Plan Cycles as per 597.4(c).

To continue the example, **Table 3** shows how data should be recorded for the example district. For simplicity, it is assumed that all devices are meter gates.

**Table 3. Sample data collection for selected turnouts**

*Red rows indicate devices that do not meet the required standard*

Turnout #	Device Type	Acreage Served	Flow Accuracy Error, %
2	Meter Gate	10	15%
3	Meter Gate	15	9%
4	Meter Gate	15	6%
6	Meter Gate	2	8%
10	Meter Gate	50	4%
<i>Total acreage sampled:</i>		<b>92</b>	

**Step 4: Determination of Compliance**

SBx7 requires an annual volumetric accuracy of within 12% on existing devices. Table 3 addresses flow rate accuracy, not volumetric accuracy.

If 25% or more of the sampled area for a particular device type exceeds the 12% annual volumetric allowable error, then a second round of testing must be conducted. This second round of testing should be conducted in the same manner as the first, but only for the device type(s) that did not meet the required accuracy standard.

*Compliance of this particular example.* Table 3 is repeated below for illustration.

**Table 3. Sample data collection for selected turnouts**

*Red rows indicate devices that do not meet the required standard*

Turnout #	Device Type	Acreage Served	Flow Accuracy error, %
2	Meter Gate	10	15%
3	Meter Gate	15	9%
4	Meter Gate	15	6%
6	Meter Gate	2	8%
10	Meter Gate	50	4%
<i>Total acreage sampled:</i>		<b>92</b>	

Assuming that the minimum required flow rate accuracy is 10.7% (using the example), then only one turnout measurement device does not meet the requirement. No re-testing is needed, because:

1. Ninety-two acres were tested out of the total 164 acres. This is much greater than the 10% sample size required.
2. Five devices were sampled, which meets the minimum because all devices are of the same basic design.
3. The one device with greater than 10.7% error only represents 10 acres, which is 11% of the acreage sampled. This is below the allowable 25%.

## Scenario 2: Limited Availability of Turnouts and Opportunity Sampling

Turnouts may not be available for inspection due to fluctuations in irrigation scheduling. Therefore, opportunity sample can be used to select devices to be evaluated. As opposed to the PPS random sample set, this sample will be based on availability and service size rather than a weighted random sampling.

### Background

#### *Representative Sample Selection*

To ensure the sample is representative of the district as a whole, evaluators need to ensure that the area serviced by the devices evaluated is at least 10% of the district’s entire area. Furthermore, when given a choice between devices of equal convenience, devices servicing a larger acreage should be given priority for inspection. Additionally, a minimum of 5 devices must be inspected. In this way each device will be weighted in selection by the acreage it serves. Specifically, the sample will be skewed favoring devices that measure greater volumes of water. This will ensure that the opportunity sample will be statistically representative of the overall accuracy of flow measurement within the district.

#### *Selection Process*

Devices will be selected as they are available to be tested. Priority for evaluation will be given to devices that service greater acreage. Once a device has been tested, it will no longer be considered in the selection process. A minimum of 5 devices will be tested, and all evaluated devices (summation of all types) will service a combined 10% of the district’s total area (or delivered volume), not to exceed 100 individual devices of a certain type.

#### Step 1: Choose a Currently Available Turnout

Select a turnout that is available for testing based on the size of the turnout, giving priority to turnouts that serve greater acreage. Do not test the same device more than once. **Table 4** shows an example of the selection process for two days. On the first day Turnout 10 serves the largest acreage out of the available turnouts. On day two, Turnout 5 is chosen because it serves the largest area and has not yet been tested. The district in this example has one canal lateral with 10 turnouts, and the turnouts have limited availability for testing.

**Table 4. Device selection on two separate days**

*Green rows indicate the selected turnout. Grey rows indicate a turnout that has been tested.*

Day 1			Day 2		
Turnout #	Currently Available	Acreage Served	Turnout #	Currently Available	Acreage Served
1	yes	10	1	no	10
2	yes	10	2	yes	10
3	no	9	3	no	9
4	yes	7	4	yes	7
5	no	30	5	yes	30
6	no	1	6	no	1
7	yes	1	7	yes	1
8	yes	2	8	yes	2
9	no	50	9	no	50
10	yes	50	10	yes	50

**Continue testing devices until the following criteria have been met:**

- At least 10% of the total district acreage is serviced by the devices tested
- At least 5 devices have been tested
- Test no more than 100 devices of a particular type

#### Steps 2-4 : Follow the Previous Scenario Instructions

## FLOW MEASUREMENT DEVICES

### *Background*

This section is intended to provide useful information on several common flow measurement devices that might be considered for traditional, non-pressurized turnouts. Often, the problems with some of the devices (meter gates, orifice plates, and propeller meters) are largely associated with improper measurement, or improper installation or maintenance. If properly designed and maintained, all three of these measurement devices will generally fall well within required SBx7 requirements.

### *Meter Gates*

Meter gates are one of the most common devices used in California irrigation districts to both measure and control flow rates. There is no doubt that many of these devices provide accurate results. However, as with all devices, certain rules must be followed. Typical physical inaccuracies associated with meter gates include:

1. *Incorrect "zero" measurement of gate opening*, as determined by the vertical movement of the threaded shaft.
  - a. There are four primary reasons operators might measure the opening from an incorrect "zero" mark on the threaded shaft:
    - i. The zero point is affected by "slop" in the connection between the shaft and the gate plate.
    - ii. Wedges are used to force the plate against the gate frame during gate closure. These wedges are often adjusted in the field, so there is no standard stopping distance (vertically) for the plate.
    - iii. When the plate begins to move, it may overlap the opening (by 0.5 - 2"). Although water may begin to leak as the plate moves out of the wedge constraint, the true zero is the opening at which the bottom of the plate is exactly at the bottom of the frame opening.
    - iv. The "zero" point should always be determined while the gate is being raised.
  - b. Once the zero point is known, a notch should be scribed into the shaft to note the location of the zero mark. Then the gate opening should always be measured as the gate is being opened, rather than being closed.
2. *Incorrect downstream water level measurement*.
  - a. The stilling well must be placed over a full pipe, at a specific distance downstream of the meter gate.
  - b. Many existing stilling wells were actually designed to be air vents, and have such a small diameter that there is constant surging. A large diameter stilling well, fed by a relatively small access hole at its bottom (about 1/6th the diameter of the stilling well), is needed to "still" the water surface so it can be measured downstream of the gate. The problem with a small access hole is that it can plug up easily. A good combination is a 2" access hole (connecting the stilling well to the top of the pipe) and a 12" stilling well.
  - c. The pipe must be full at all flow rates. This may require the placement of a small obstruction downstream, in the pipe, similar to what is done with well pump discharges to keep propeller meters full. Various entities, including ITRC, have successfully designed side contractions in pipes to create "Replogle flumes" that have very little loss, and that pass bottom loads of silt. Something similar could be used downstream of the meter gates.





**Figure 3. Side contractions rather than a traditional "Replogle Flume". Designed by USBR, Yuma. The rocks are not part of the design.**

Another technique used in some districts to maintain a submerged condition on a gate is to install "bumps" in the bottom of a canal or ditch downstream of the turnout. These should be permanent "bumps" which, at low flows, will keep the water level high. The rule for building these "bumps" is:

Build up the restriction from the bottom of the ditch/canal so that at high flow rates, the upstream water surface (relative to the bump) is only raised by about 0.1' or less. In other words, its presence will hardly be noticeable.

If farmers move downstream in their canal, setting siphons at a different place, this "bump" will keep the backpressure on the meter gate almost constant, and minimize the flow rate change that would normally occur.

3. *Incorrect gate opening geometry.* Since the plate has a larger outside diameter than the inside diameter of the pipe, the ratio of the open area between the two openings must be taken into account. Almost everyone uses tables that were developed decades ago. ITRC is not certain if the gate dimensions have changed since then, or if different manufacturers use different gate dimensions. ITRC is planning to verify this in the future.
4. *Non-standard entrance and exit conditions.* The flow rate is associated with a measured opening and head loss. The head loss will be different (at the same flow rate) with different entrance conditions. Various manuals, such as the USBR Flow Measurement Manual, provide recommended dimensions.

## Orifice Plates

The following is an explanation of the characteristics of a submerged (on both sides) rectangular orifice plate.

According to the U.S. Bureau of Reclamation *Water Measurement Manual*, conditions for achieving accurate flow measurement of  $\pm 2\%$  for a fully contracted submerged rectangular orifice are:

- The upstream edges of the orifice should be straight, sharp, and smooth.
- The upstream face and the sides of the orifice opening need to be vertical.
- The top and bottom edges of the orifice opening need to be level.
- Any fasteners present on the upstream side of the orifice plate and the bulkhead must be countersunk.
- The face of the orifice plate must be clean of grease and oil.
- The thickness of the orifice plate perimeter should be between 0.03 and 0.08 inches. Thicker plates would need to have the downstream side edge chamfered at an angle of at least 45 degrees.
- Flow edges of the plate require machining or filing perpendicular to the upstream face to remove burrs or scratches and should not be smoothed off with abrasives.
- For submerged flow, the differential in head should be at least 0.2 feet.
- Using the dimensions depicted in **Figure 4** below,  $P > 2Y$ ,  $Z > 2Y$ , and  $M > 2Y$

The equation for determining the flow through a submerged orifice plate is:

$$Q = C_d A \sqrt{2g\Delta h}$$

Where:

Q = Flow Rate, CFS

$C_d$  = Coefficient of Discharge, 0.61

A = Area of the orifice,  $\text{ft}^2$

A = W x Y

W = Orifice opening width, ft

Y = Orifice opening height, ft

g = Acceleration due to gravity,  $32.2 \text{ ft/s}^2$

$\Delta h$  = Change in head, ft

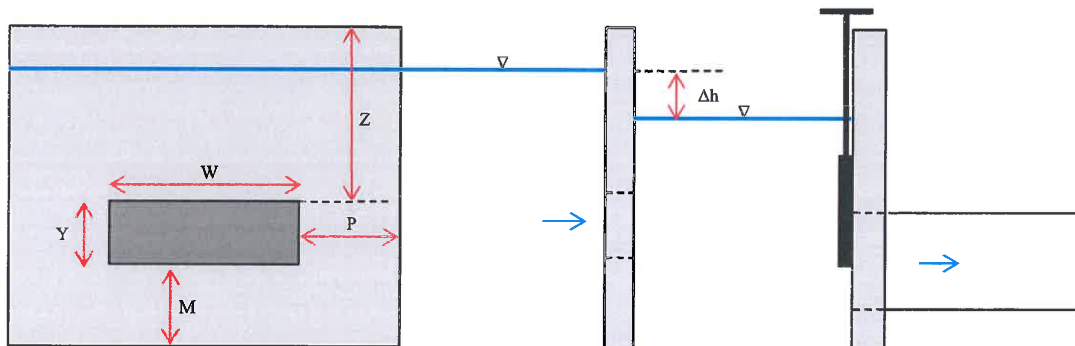


Figure 4. Flow through a submerged orifice plate

For a sharp-edged rectangular orifice where full contraction occurs from every side of the orifice, the coefficient of discharge is 0.61.

It is recommended that “Y” be smaller than “W”, so that a good depth “Z” can be maintained. This helps keep the orifice entrance submerged all the time regardless of upstream water level fluctuations, and also provides for the proper entrance conditions.

It is assumed that the flow control gate will be located downstream of the orifice plate. The particular dimensions of that gate would rarely influence the performance of an orifice plate.

Typical problems include:

1. Inaccurate measurement of the difference in head.

*Solution:*

- a. Careful relative calibration of pressure transducers, if used. They do not need to read a correct "elevation", but at zero flow rate must read the same "elevation".
- b. Install a horizontal reference steel plate on a bulkhead wall, so operators use the same reference elevation for both measurements if they manually measure the head difference.

2. The distances P, Z, or M are not greater than 2 times the smallest opening dimension (usually “Y”). In reality, it is rare that this "2 times" criteria is met in irrigation districts, except with very small flows.

*Solution:*

- a. If only one side is suppressed (typically the bottom entrance, which might have no convergence), adjust the discharge coefficient,  $C_d$  as follows:

W/Y	1	2	4
$C_d$	0.63	0.64	0.65

- b. We do not know exactly how much to adjust the  $C_d$  if the distances P, Z, or M are less than two times the smallest opening dimension. Therefore, it is recommended that the orifice be installed in a plate that is wide enough and tall enough to approximately meet those required distances – even if the plate must be extended beyond the inlet to the turnout. See the figure below.

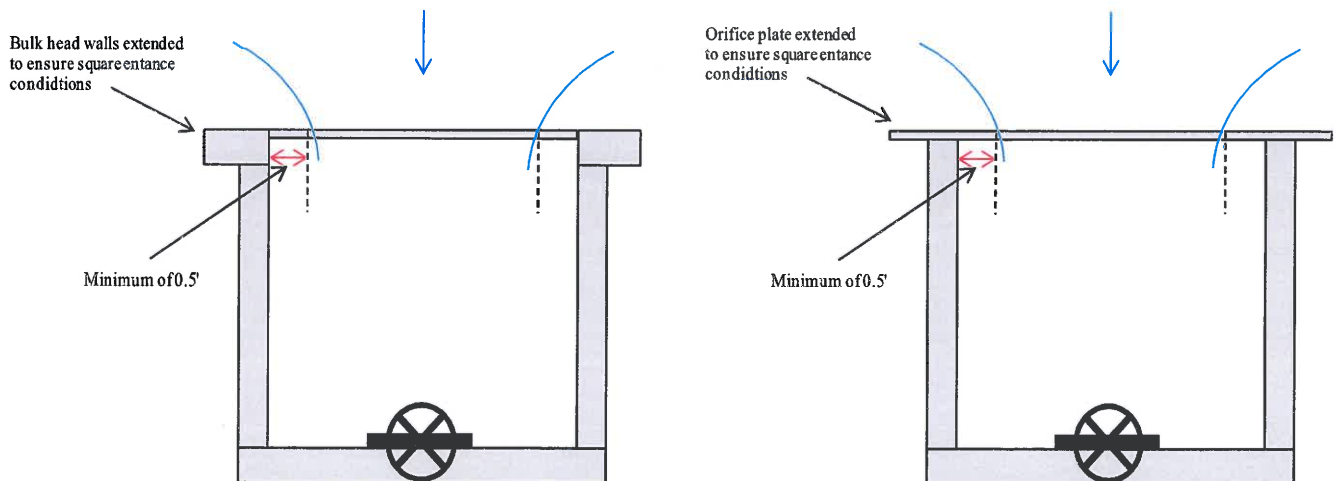


Figure 5. Installation of orifice

3. A single orifice size has a limited flow rate range. This is illustrated in the tables below. At too low a flow rate, the measured head difference is very small, often resulting in major errors in head difference. At too high a flow rate, the measured head difference is excessive, and may well exceed the available head. For this reason, *it is common to have a moveable plate that can be adjusted up and down*, varying the "Y" dimension.

The addition of the moveable plate (often a rectangular sluice gate) creates the commonly known "CHO" or "constant head orifice". The device certainly does not create a "constant head", but it does provide an adjustable orifice. It provides the flexibility needed for a turnout to supply different flows at different times, with reasonably accurate head measurements. The opening should be adjusted so that the minimum head difference is greater than 0.2'. A 1' head loss across the orifice plate is more than what is attainable in many California irrigation district turnouts.

Table 5. Orifice size values

Flow Rate, CFS	Width of Orifice Opening, ft							
	1.0							
	Height of Orifice Opening, ft							
	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
	Change in Head, ft							
5.0								1.0
4.5							1.0	0.8
4.0						1.0	0.8	0.7
3.5					1.0	0.8	0.6	0.5
3.0				1.0	0.8	0.6	0.5	0.4
2.5			1.0	0.7	0.5	0.4	0.3	0.3
2.0		1.0	0.7	0.5	0.3	0.3	0.2	0.2
1.5	1.0	0.6	0.4	0.3	0.2	0.1	0.1	
1.0	0.5	0.3	0.2	0.1				

Flow Rate, CFS	Width of Orifice Opening, ft						
	1.5						
	Height of Orifice Opening, ft						
	0.5	0.6	0.8	1.0	1.2	1.4	1.5
	Change in Head, ft						
11.0						1.1	1.0
10.0						0.9	0.8
9.0					1.0	0.8	0.7
8.0				1.2	0.8	0.6	0.5
7.0				0.9	0.6	0.5	0.4
6.0			1.0	0.7	0.5	0.3	0.3
5.0			0.7	0.5	0.3	0.2	0.2
4.5		1.0	0.6	0.4	0.3	0.2	0.2
4.0	1.2	0.8	0.5	0.3	0.2	0.2	0.1
3.5	0.9	0.6	0.4	0.2	0.2	0.1	0.1
3.0	0.7	0.5	0.3	0.2	0.1		
2.5	0.5	0.3	0.2	0.1			
2.0	0.3	0.2	0.1				
1.5	0.2	0.1					

Table 5 (continued). Orifice size values

Flow Rate, CFS	Width of Orifice Opening, ft								
	2.0								
	Height of Orifice Opening, ft								
	0.5	0.6	0.8	1.0	1.2	1.4	1.6	1.8	2.0
	Change in Head, ft								
20.0									1.0
19.0								1.2	0.9
16.0							1.0	0.8	0.7
13.0						0.9	0.7	0.5	0.4
10.0				1.0	0.7	0.5	0.4	0.3	0.3
9.0				0.8	0.6	0.4	0.3	0.3	0.2
8.0			1.0	0.7	0.5	0.3	0.3	0.2	0.2
7.0			0.8	0.5	0.4	0.3	0.2	0.2	0.1
6.0		1.0	0.6	0.4	0.3	0.2	0.1	0.1	
5.0	1.0	0.7	0.4	0.3	0.2	0.1	0.1		
4.5	0.8	0.6	0.3	0.2	0.1	0.1			
4.0	0.7	0.5	0.3	0.2	0.1				
3.5	0.5	0.4	0.2	0.1					
3.0	0.4	0.3	0.1						
2.5	0.3	0.2	0.1						
2.0	0.2	0.1							

Flow Rate, CFS	Width of Orifice Opening, ft										
	2.5										
	Height of Orifice Opening, ft										
	0.5	0.6	0.8	1.0	1.2	1.4	1.6	1.8	2.0	2.2	2.4
	Change in Head, ft										
30.0										1.0	1.0
25.0								1.0	0.9	0.7	0.7
20.0						1.0	0.8	0.7	0.6	0.5	0.4
15.0				1.0	0.8	0.6	0.5	0.4	0.3	0.3	0.2
10.0			1.0	0.7	0.5	0.3	0.3	0.2	0.2	0.1	0.1
9.0			0.8	0.5	0.4	0.3	0.2	0.2	0.1	0.1	
8.0		1.2	0.7	0.4	0.3	0.2	0.2	0.1	0.1		
7.0		0.9	0.5	0.3	0.2	0.2	0.1	0.1			
6.0	1.0	0.7	0.4	0.2	0.2	0.1					
5.0	0.7	0.5	0.3	0.2	0.1						
4.5	0.5	0.4	0.2	0.1							
4.0	0.4	0.3	0.2	0.1							
3.5	0.3	0.2	0.1								
3.0	0.2	0.2									

Flow Rate, CFS	Width of Orifice Opening, ft												
	3.0												
	Height of Orifice Opening, ft												
	0.5	0.6	0.8	1.0	1.2	1.4	1.6	1.8	2.0	2.2	2.4	2.6	2.8
	Change in Head, ft												
45.0												1.2	1.0
40.0											1.1	0.9	0.8
35.0									1.2	1.0	0.8	0.7	0.6
30.0								1.0	0.9	0.7	0.6	0.5	0.5
25.0						1.1	0.9	0.7	0.6	0.5	0.4	0.4	0.3
20.0					0.9	0.7	0.6	0.5	0.4	0.3	0.3	0.2	0.2
15.0			1.0	0.7	0.5	0.4	0.3	0.3	0.2	0.2	0.2	0.1	0.1
10.0			0.7	0.5	0.3	0.2	0.2	0.1	0.1				
5.0	0.5	0.3	0.2	0.1									

If steel theft is a concern, a marine plywood frame could be used to support a steel orifice opening frame. Fasteners used to connect the steel orifice to the plywood frame would need to be countersunk to minimize debris getting caught on them.

## Trash Shedding Propeller Meters

For several decades there has been interest in "trash shedding propeller meters". ITRC examined the "cloggability" of an early design about 20 years ago. Boat propellers are sold with "weed shedding" features, which include specially designed propellers as well as fixed vanes upstream of the propeller that are intended to pass the weeds below or to the side of the boat propeller. McCrometer sells a saddle meter with the trash shedding options.



### MODEL M0300SW

#### DESCRIPTION

The M0300SW is a bolt-on reverse-helix\* propeller meter designed to shed debris often associated with surface water applications. The M0300SW is designed with the meter body turned 180 degrees from normal, a propeller installed nose-first on the bearing shaft, and a reverse flow style bearing assembly. This configuration allows the ell to curve with the flow, allowing grass or other debris to shed off with ease. The assembly design also reduces the ability of sand and silt to accumulate in the bearing.

The M0300SW features a fabricated stainless steel saddle with McCrometer's unique drive and register design. The stainless steel saddle eliminates the fatigue-related breakage common to cast iron and aluminum saddles and provides unsurpassed corrosion protection. Fabricated stainless steel construction offers the additional advantage of being flexible enough to conform to out-of-true pipe. The Model M0300SW is manufactured to comply with applicable provisions of American Water Works Association Standard No. C704-02 for propeller-type flowmeters. As with all McCrometer propeller flowmeters, standard features include a magnetically coupled drive, instantaneous flowrate indicator and straight reading, six-digit totalizer.

The impellers are manufactured of high-impact plastic, capable of retaining their shape and accuracy over the life of the meter. Each impeller is individually calibrated

### CONFIGURATION SHEET REVERSE BOLT-ON SADDLE SURFACE WATER FLOWMETER


at the factory to accommodate the use of any standard McCrometer register, and since no change gears are used, the M0300SW can be field-serviced without the need for factory recalibration. Factory lubricated, stainless steel bearings are used to support the impeller shaft. The shielded bearing design limits the entry of materials and fluids into the bearing chamber providing maximum bearing protection.

The instantaneous flowrate indicator is standard and available in gallons per minute, cubic feet per second, liters per second and other units. The register is driven by a flexible steel cable encased within a protective vinyl liner. The register housing protects both the register and cable drive system from moisture while allowing clear reading of the flowrate indicator and totalizer.

#### INSTALLATION


Standard installation is horizontal mount. If the meter is to be mounted in the vertical position, please advise the factory. A straight run of full pipe the length of eight pipe diameters upstream and five diameters downstream of the meter is recommended for meters without straightening vanes. Meters with optional straightening vanes require at least three pipe diameters upstream and two diameters downstream of the meter.

\* 4" meters use a forward helix propeller with a reverse register.



Typical face plate

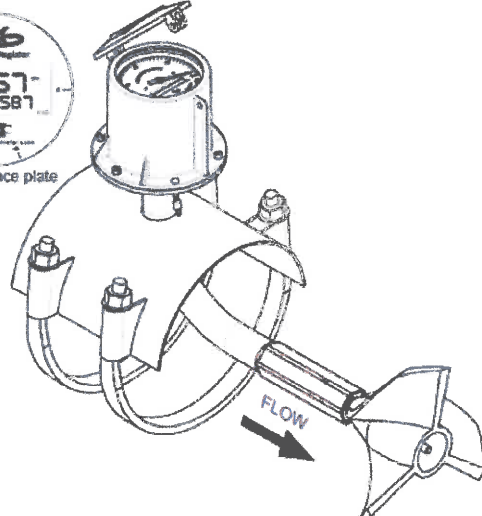
The McCrometer Propeller flowmeter comes with a standard instantaneous flowrate indicator and straight-reading totalizer. An optional FlowCom register is also available.



Typical face plate

#### APPLICATIONS

- Surface Water
- Water Containing Trash
- Sand Producing Wells
- Irrigation District Turnouts



McCrometer will also mount a reverse-facing propeller on a standard open flow meter, which can be mounted on stands above low pressure pipelines.



## CONFIGURATION SHEET OPEN FLOWMETER

### MODEL M1700

#### DESCRIPTION

Model M1700 Open Flowmeters are designed to measure the flow in canal outlets, discharge and inlet pipes, irrigation turnouts and other similar installations. The M1700 series meets or exceeds the American Water Works Association Standard C704-02. Constructed of stainless steel, the meter incorporates bronze mounting brackets that permit simple installation and removal. As with all McCrometer propeller flowmeters, standard features include a magnetically coupled drive, instantaneous flowrate indicator and straight reading, six-digit totalizer.

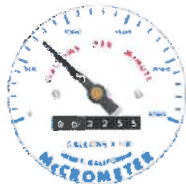
Impellers are manufactured of high-impact plastic, designed to retain both shape and accuracy over the life of the meter. Each impeller is individually calibrated at the factory to accommodate the use of standard McCrometer registers, and since no change gears are necessary, the M1700 can be field-serviced without the need for factory recalibration. Factory lubricated, stainless steel bearings are used to support the impeller shaft. The sealed bearing design limits the entry of

materials and fluids into the bearing chamber providing maximum bearing protection.

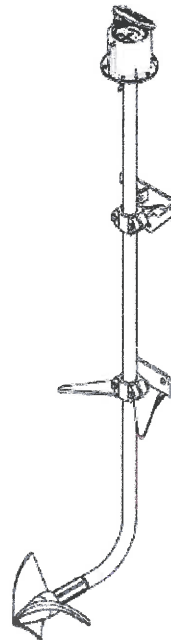
An instantaneous flowrate indicator is standard and available in gallons per minute, cubic feet per second, liters per second and other units. The register is driven by a flexible steel cable encased within a protective, self-lubricating vinyl liner. The die-cast aluminum register housing protects both the register and cable drive system from moisture while allowing clear reading of the flowrate indicator and totalizer.

#### INSTALLATION

The M1700 must be mounted on a headwall, standpipe or other suitable structure so that the propeller is located in the center of the discharge or inlet pipe. A straight run of full pipe the length of ten pipe diameters upstream and two diameters downstream of the meter is recommended for meters without straightening vanes. Meters with optional straightening vanes require at least five pipe diameters upstream of the meter. Please specify the inside diameter of the pipe when ordering.



The McCrometer Propeller flowmeter comes with a standard instantaneous flowrate indicator and straight-reading totalizer. An optional FlowCom register is also available. Typical face plates.



#### APPLICATIONS

The McCrometer propeller meter is the most widely used flowmeter for municipal water and wastewater applications as well as agricultural and turf irrigation measurements.

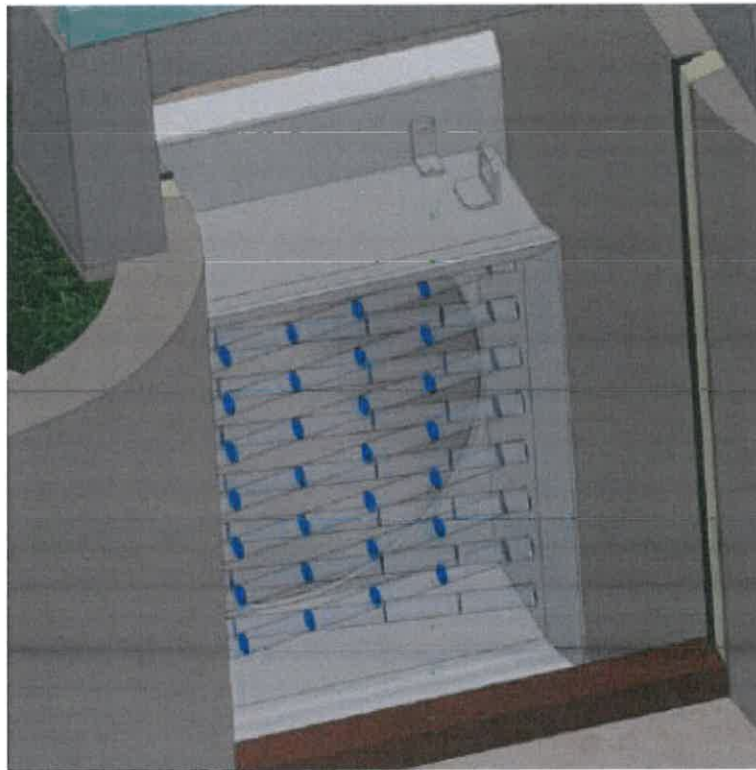
Typical applications include:

- Water and wastewater management
- Canal laterals
- Gravity turnouts from underground pipelines
- Sprinkler irrigation systems
- Golf course and park water management

A commercially available package that includes a reverse propeller meter and trash-shedding fixed vane, plus flow straighteners, is available from RSA.

### ***Rubicon Transit Time Flow Meter***

The Rubicon Sonaray flow meter is an interesting addition for larger turnouts with a canal supply, in that it also has a totalizer. The Rubicon literature cites a flow test in California, but it is unclear if the magmeter used for flow rate verification was recently calibrated. ITRC has found that new magmeters with guaranteed accuracies can be off by several percentage points. The device appears to be new, without substantial field testing in the USA.



**Figure 6. Rubicon Sonaray flow meter**



**MODESTO IRRIGATION DISTRICT**

**2015 AGRICULTURAL WATER MANAGEMENT PLAN**

**APPENDIX I**

**COMMENTS ON DRAFT AGRICULTURAL  
WATER MANAGEMENT PLAN**

To: John B. Davids, P.E. Civil Engineering Manager for MID  
From: Ross Campbell  
Subject: Comments on 2015 Agricultural Water Management Plan Draft

There have been two significant actions taken in the last two years, one by the MID in September 2013 and one by the state of California in September 2014. These actions will impact on the how the MID is operated and manages for the foreseeable future.

The Modesto Irrigation District has been in operation for over 125 years. During that period of time they have not been regulated by the State of California. The District has a five-member Board of Directors who manage the District's three businesses; irrigation, electric and domestic water. On September 10<sup>th</sup> 2013 the district adopted a set of Governance Policies in order to formalize the policies and relationships between the MID Board and staff as well as the owners and customers.

The State of California adopted the Sustainable Groundwater Management Act (SGMA) on September 16, 2014. The legislation went into effect January 1, 2015

Key dates for the implementation of SGMA are:

- January 1, 2015-legislation is effective
- January 1, 2016-state regulations finalized for basin boundary adjustments
- June 1, 2016-state regulations finalized for evaluation of groundwater sustainability plans (GSPs)
- June 30, 2017-local groundwater sustainability agencies (GSP)
- January 31, 2020 groundwater sustainability plans (GSPs) must be completed for basins in a critical condition of overdraft
- January 31, 2022-GSP must be completed in all other high-and medium-priority basins
- 20 years after adoption of the GSP (2040 and 2042)-all high-and medium priority groundwater basins must achieve sustainability.

As you can see changes are in place which will have a major impact on how it MID faces the future. The current draft under review represents the past. I believe there is a need for some rewriting to be done so that the plan anticipates the future and does not depend on the past.

I would suggest the following;

Add a chapter on State Water Law, Water Rights and word definitions.

I've talked to many people in the County about water law and it's amazing how many interpretations of the law exist. Definitions of words used in discussing water such as beneficial use, reasonable use, safe yield, water budget, cost of service, etc. mean different things to different people. You should also include in this chapter a listing of your water rights and the quantity of water involved in acre-feet. Knowing the amount of surface water you have a right to is essential in building a Water Budget for the district.

Add a chapter on conjunctive use of surface and groundwater.

The term conjunctive use is scattered throughout the AWMP but it is never defined as a specific program of the MID. In years of adequate surface water, the excess water is delivered to the farmers at little cost and claimed as groundwater recharge by the MID. That water is surplus water and subject to appropriation by others in need of water. There is plenty of need for surplus water in Stanislaus County. The sooner the MID has contracts for the use of the surplus water the safer that water will be for your use in the County.

The city of Modesto has the only conjunctive use plan for surface and groundwater. It is costing the citizens of Modesto millions of dollars a year for this project and it is worth every penny to meet existing and future water needs.

MID has a program of grants and loans in place to help fund and install pressure irrigation systems so that they can use surface water for irrigation in times of surplus water and groundwater in times when there is a drought. It will be costly but needed to ensure the future of farming as we know it in the County. Conjunctive use of surface and groundwater is a far better program for use by farmers than groundwater recharge.

On page 7 the item, Water Allocation and Pricing, may be facing a major rewrite as the MID converts to actual cost of service for setting service rates. The October 2015 publication of Currents, which the MID distributes with the electric bills, also states that the MID uses cost of service for setting rates for the three businesses the MID manages.

On page 12 the report begins a listing of capital projects. For each project listed it would be useful to see added, the date it was completed, the cost of the project and how it was funded.

On page 15 it is noted that the MID was formed on July 9, 1887 and that during early years, the MID acquired numerous water rights. The water rights need to be listed and the volume of water shown in acre-feet is also needed. This information is needed for the development of a water budget by the MID.

On page 19 on about the middle of the page it says the Districts have agreed to allow CCSF to have a water bank of 570,000 acre-feet in new Don Pedro reservoir. The fact is CCSF paid \$47 million for that privilege.

On page 21 in the third paragraph from the top it notes that permanent crop irrigation is being converted to low-volume irrigation systems. The current rate of conversion to low-volume micro irrigation systems is estimated to be about 130 acres per year. It would be nice to know how many acres of low-volume micro irrigations system are now in place and the impact that has on water use and cost of service.

On page 22 begins a short chapter on Climate. The CCSF, TID and MID all depend on the Tuolumne River for their surface water (CCSF 85%, TID 100%, MID 100%). It strikes me that these three agencies should be working together to suggest what the future holds and what we need to do to be prepared for the future.

On page 27 Table 11 - Water Allocation Policy The table indicates that in a normal water year you a lot 42 inches of water to your irrigation customers. This is 6 inches of water more than the average crop demand which is in the order of 36 inches. The 6 inches of extra water is actually surplus water. The value of surplus water is in the range of \$200 - \$700 an acre foot. The discussion lower on this page should include an evaluation of paying \$160 an acre foot to deliver this water for groundwater recharge or the transfer of use of this water to others in need for a fee that represent income for the district. This income could go a long way for funding MID capital projects.

On page 30 there is a discussion of Water Rates and Schedules and Billing. This discussion should be revised to reflect that service rates are based on cost of service as required in the Governance Policies and restated in the October 2015 Currents publication of the MID. It also notes that the raw water sent to the city of Modesto is billed at the same rate as agricultural water users. The raw water is not a cost of service and is used to subsidize the losses in the irrigation service

On page 35 special drought programs are described. It appears to me that each of these programs is illegal under state water law. You can't sell water that you don't own and you can't transfer water use unless it is surplus water. Drought programs do not involve surplus water.

On page 37 irrigation water delivered to a property or groundwater pumped on that property must stay on that property under state water law. How do you justify these programs?.

On page 45 you assume that the overall irrigation efficiency of 63%. You then note that the remaining 37% of applied water is assumed to be destined to groundwater recharge. It is my understanding that the 37% allows for reasonable and unavoidable losses which are due to soil variability, wind, evaporation, changes in system flow rates, irrigation system constraints, etc.

A Water Budget is required as part of the development of the SGMA plan. What you have in the updated AWMP is an Over All Water Balance. The Water Budget below is based on a report prepared by JMLord, agricultural scientists consulting engineers, for the MID entitled Assessment Of Reasonable Water Requirements dated October 1998 Such a Water Budget will be needed to meet SGMA requirements.

#### WATER BUDGET

Crop demand	184,700 acre-feet
Operations system loss	68,389 acre-feet - 37% of total water delivered
Irrigation demand	253,089 acre-feet
Surface water delivered	328,502 acre-feet
Groundwater pumped	127,200 acre-feet
Total water supply	455,702 acre-feet
Operational spills	40,000 acre-feet
Surplus water	87,200 acre-feet

The figures used in the above water budget came from table 48, overall water balance for 2012, on page 64 of the AWMP. The numbers I've placed in the water budget need checking

I appreciate the opportunity to comment on the draft report and look forward to comments about the suggestions.

H.D. Peter  
Drekmeier

# Tuolumne River Trust

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December 15, 2015

President Larry Byrd and Board Members  
Modesto Irrigation District  
1231 Eleventh Street  
Modesto, CA 95354

## RE: MID's 2015 Agricultural Water Management Plan

Dear President Byrd and Board Members:

Thank you for the opportunity to comment on MID's updated Agricultural Water Management Plan. In general, we believe this document is comprehensive and well written. It's assuring to see progress being made on water delivery measurements, water use efficiency, and better groundwater management.

We are pleased to see that MID believes the measurement methodology in some cases may be improved to increase agricultural water use efficiency, and we encourage the continued evaluation of alternatives. We also appreciate that MID is moving forward on a Comprehensive Water Resources Management Plan to guide future water management decisions.

We believe the Plan could be improved by incorporating more details on potential water conservation measures. For example, in 2012 when MID was considering a water transfer with the City and County of San Francisco, between 25,000 and 40,000 acre-feet of operational discharges were identified as having the potential to be retained through infrastructure improvements such as renovating the Dry Creek Flume, improving the main lateral and headings, improving flow control structures, instituting outflow interception, installing canal interceptor pipelines, and constructing regulating reservoirs.

The above projects (and there likely are others) would help MID adapt to reduced water availability resulting from climate change and likely changes to the instream flow schedule imposed by the Federal Energy Regulatory Commission through relicensing of Don Pedro Dam and the State Water Resources Control Board through the Bay Delta Water Quality Control Plan.

We also encourage MID to consider higher volumetric pricing of water through the new tiered-rate structure. Currently, the vast majority of cost is incorporated into the fixed charge (\$40 per acre). For the first two acre-feet of water purchased, customers pay only \$1/AF. For the third acre-foot the cost is just \$2, and for the next six inches users pay only \$3/AF. Anything above 42 inches costs just \$10/AF.

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We believe the tiered-rate structure could be improved to encourage greater water use efficiency. By increasing volumetric costs, MID would send a strong price signal that efficiency is encouraged while generating revenue to implement the Efficient Water Management Practices identified in the Plan, as well as other measures not yet considered.

Finally, TRT has been following MID's progress working with the Stanislaus and Tuolumne Rivers Groundwater Basin Association to develop a Groundwater Sustainability Plan, and we appreciate the effort going into this process. We hope the Plan will thoroughly study the hydrological connectivity between the Tuolumne River and the groundwater basin.

Thank you for the opportunity to share our thoughts.

Sincerely,

A handwritten signature in purple ink that reads "Peter Drekmeier". The signature is written in a cursive style with a large initial "P".

Peter Drekmeier  
Policy Director