

Loleta Community Services
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FAX TRANSMITTAL FORM

To NCRWQCB

From Markus Drumm
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- Urgent
- For Review
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① Submission in support of request

O&M manual Vroman Engineering
re: plant capacity

Loleta Community Services District
 Submission in support of request for amendments to
 Discharge Requirements WDID No. 1B800810HUM

Markus Drumm
 General Manager
 1/25/2008
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I. FACILITY INFORMATION

Design Flows:

Operations and Maintenance Manual, Loleta Sanitary District, *Wastewater Treatment Facility Operation and Maintenance Manual*.

p. 1 "In 1978, a new clarifier, sludge wasting tank, and disinfection facilities were added and the capacity increased to 100,000 gpd."

Figure I-2 *Load Data and Design Criteria*

	1978 (current)	1998 Projection
Average Dry Weather Flows	81,000	95,000
Peak Dry Weather Flows	188,000	211,000
Maximum Monthly Flows	143,000	158,000
Peak Wet Weather Flows	761,300	816,000

Thus the design flow should read 100,000 gpd (average dry weather).

It should also be greater than the 1998 projections, but I have not found exact numbers in the engineer's report.

(see attached)

IV. EFFLUENT LIMITATIONS

pH:

The District of Loleta would benefit from a relaxation of the pH 6.5 Basin plan standard to the USEPA pH 6.0 standard.

The District has been successful in decreasing the effects of Inflow and Infiltration and plant loading, although we have not yet achieved the 225 gpd per person USEPA guideline. During winter rains it is not uncommon for influent to enter the plant at or below pH 6.5. Our permit requires chlorination and dechlorination, both of which contribute to depress the pH below permissible levels. The rainwater itself is the cause of the pH problem.

As described in the permit, we discharge to a wetland, which is tributary to an unnamed slough, and the Eel River. The slough is eutrophic, and I am sure that the testing required under this permit will show that our discharge is incapable of dropping the pH of the slough below 6.5.

REPORTING REQUIREMENTS

River Flow Reporting Requirements:

I could not find the report requirement for receiving water flow under the present permit in the new permit. If they will be required, the District requests that only the Eel River Daily Flow be required. The ratio of flows from the WWTP and the Eel River are consistently in the range of 1:10,000, far in

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excess of the 1:100 preferred. The Van Duzen dries up long before the Eel. The data for the Van Duzen is reported in minute by minute cubic feet per second estimates, so either massive calculations or good faith estimates of averages are required to report Van Duzen flow.

PROHIBITION ON DISCHARGES TO THE TRIBUTARY MAY 15 THROUGH SEPTEMBER 30TH

The tributary to which the Loleta WWTP discharges is seasonal and controlled at this point by the water table in the Eel River basin. I assume that the purpose behind the prohibition is to prevent discharges while the Eel is running low and impacts to beneficial uses would be magnified. Since the volume of flow the Eel and the flow through the tributary are controlled identically by the water table, the enforcement of exact dates does not serve the purpose for which they exist. Since the dates serve no purpose as it relates to the Loleta discharge, the District requests a waiver from a specific date requirement in the permit.

LOLETA SANITARY DISTRICT
WASTEWATER TREATMENT FACILITY
OPERATION AND MAINTENANCE MANUAL

September 1978

prepared by ...

VROMAN ENGINEERING AND CONSTRUCTION

627 "H" Street, Eureka, California

I. INTRODUCTION

* A. Purpose of the Project: This facility was originally constructed as a packaged extended aeration treatment facility in 1958 with a capacity of 73,600 GPD. In 1978, a new clarifier, sludge wasting tank and disinfection facilities were added and the capacity increased to 100,000 GPD.

The method of treatment employed by the Loleta Sanitary District Wastewater Treatment Facility is a variation of the Activated Sludge process which is commonly called an Extended Aeration Treatment Process. This method of treatment involves culturing and maintaining an aerobic bacteria population sufficient to reduce wastewater pollutants to more inert and more easily removable forms. The process involves long and repeated exposure of the aerobic organisms to incoming wastewater while maintaining a sufficient life sustaining oxygen level in the aeration basin. This is effected through the continuous removal and recirculation of activated sludge from the final clarifier to the aeration basin. Excess sludge is wasted to the sludge holding tank as necessary to maintain an efficient level of Mixed Liquor Volatile Suspended Solids, an indication of the biotic population, in the aeration basin.

This facility is designed to meet the following effluent limitations:

FIGURE 1-2

LOADING DATA AND DESIGN CRITERIA

EXTENDED AERATION TREATMENT PLANT

Projected

	YEAR	YEAR
	1978	1998
BASIC DATA:		
Population	612	780
Per Capita Res. & Com. Flows (GPCD)	67	70
Industrial Flows (GPD)	40,000	40,000
Average Dry Weather Flows (GPD)	81,000	95,000
Peak Dry Weather Flows (GPD)	188,000	211,000
Maximum Monthly Flows (GPD)	143,000	158,000
Peak Wet Weather Flows (GPD)	761,300	816,000
Average BOD (mg/l)	350	350
Suspended Solids (mg/l)	150	150

Wet weather design flow

GRIT REMOVAL:

Pump Sludge Holding Tank	All	All
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SCREENING:

Comminutor Capacity (GPD)	250,000	250,000
Comminutor Overflow Capacity (GPD)	750,000	750,000
Total Inlet Capacity (GPD)	1,000,000	1,000,000

PRIMARY SEDIMENTATION:

None	None	None
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